Council Chambers
62 Valentine Plains Road
Valentine Plains
Biloela Qld 4715

All Correspondence to
Chief Executive Officer
PO Box 412
Biloela Qld 4715

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Your Reference:
Our Reference: CW: mw: 23-10 (FID92235, MCU011-22/23, 15247-00000-000, ID1767441, ID1812710, ID1825305, OM005812)
Contact: enquiries@banana.qld.gov.au

27 October 2023

Edify Energy
C/- RPS AAP Consulting Pty Ltd
PO Box 1559
FORTITUDE VALLEY QLD 4006

Dear Sir/Madam

NEGOTIATED Decision Notice – Approval
(Given under section 63 of the Planning Act 2016)

Application Number:	MCU011-22/23
Description:	Public Facility Other (Solar Farm) & Animal Husbandry
Level of Assessment:	Impact Assessable
Site Address:	641 BILOELA CALLIDE ROAD, MOUNT MURCHISON 551 BILOELA CALLIDE ROAD, MOUNT MURCHISON
Lot & Plan Details:	Lot 3 on RP608599, Lot 28 on RN519, Lot 2 on RP619032, Lot 154 on SP126053

On 25 October 2023, at Council's Ordinary Meeting (OM005812), your request for a Negotiated Decision Notice, received by Council on 08 September 2023, was approved to the extent detailed in this Notice. This Negotiated Decision Notice replaces the Decision Notice previously issued and dated 25 July 2023, approved 19 July 2023 at Council's ordinary meeting (OM005715).

The nature of the changes are listed below and clearly shown in the Negotiated Decision Notice and Attachment 1 (as strikethrough bold text):-

- Condition 1 - Amended
- Condition 7 - Retained
- Condition 8 - Amended
- Condition 9 - Amended
- Condition 15 - Amended
- Condition 19 - Amended
- Condition 20 - Amended
- Condition 23 - Amended
- Condition 64 - Amended
- Condition 75 - Retained

1. Details of Approval

The following approval is given:

	Planning Regulation 2017 reference	Development Permit	Preliminary Approval
Making a Material Change of Use assessable under the planning scheme	s20	☑	☐

2. Approved Plans

The approved plans and documents for this development approval are listed in the following table:

Document number	Plan/Document name	Date
PR151484-1	Preliminary Site Layout prepared by RPS	Received by Council on ~~20 December 2022~~ **08 September 2023**
MJ2370-TIA (Revision C)	Traffic Impact Statement prepared by Northern Consulting Engineers	30/11/2022
EDF-002 (Revision A)	Surface Water and Flood Impact Assessment Report prepared by Civil IQ	9 November 2022
Version 2	Landscape Character and Visual Impact Assessment prepared by Accent Environmental	Received by Council on 17 March 2023

3. Further Development Permits

Please be advised that the following development permits are required to be obtained before the development can be carried out:

- Operational Works
- Building Works
- Plumbing & Drainage

4. Conflict with relevant instrument and reasons for the decision despite the conflict.

The assessment manager does not consider that the assessment manager's decision conflicts with a relevant instrument.

5. Submissions

There were properly made submissions about the application.

The name and address of the principal submitter for each properly made submission are as follows:

Name of Principal Submitter/s	Address
Raymond Wilkie	PO Box 269 BILOELA QLD 4715
Phillip & Narelle Wilkie	PO Box 269 BILOELA QLD 4715
Climate & Energy Realists Queensland	PO Box 259 SOUTHPORT BC GOLD COAST QLD 4215
Cedric Creed	beefy@activ8.net.au
Young Country Enterprises Pty Ltd	214 Jambin Goovigen Road GOOVIGEN QLD 4702
Terry Wilkie	PO Box 652 BILOELA QLD 4715
Christine Roebug	48 Pratton Street DALBY QLD 4405
Peter DeGit	ptdegit7@gmail.com
Emily Brosnan	emily_brosnan@hotmail.com
Gwen Jensen	gwenandbevan@bigpond.com
Tony & Bridgette Bongers	PO Box 6 JAMBIN QLD 4702
Hazel Reid	383 Shorts Road MOUNT MURCHISON QLD 4715
Mia Jensen	the4jensens@bigpond.com
Grazing BestPrac Mick Alexander	PO Box 8103 ALLENSTOWN QLD 4700
Batchfire	PO Box 144 BILOELA QLD 4715
Darren Jensen	11179 Dawson Highway MOUNT MURCHISON QLD 4715 sossj@me.com
Kamryn Johnston	The4jensens@bigpond.com
Ben Jensen	sossj@me.com
Tanya Jensen	11179 Dawson Highway BILOELA QLD 4715
Symonn Leighton	5 Callide Street BILOELA QLD 4715
Lynette LaBlack	lynettelablack@live.com
Carolyn Emms	carolynemms2@gmail.com

6. Referral Agencies

The referral agencies for this application were:

Name of referral agency	Advice agency or concurrence agency	Referral Basis	Address
Chief Executive - Queensland Treasury - State Assessment Referral Agency (SARA)	Concurrence	Schedule 10, Part 9, Division 4, Subdivision 2, Tbale 4.	RockhamptonSARA@dsdmip.qld.gov.au Fitzroy & Central Region PO Box 113 ROCKHAMPTON QLD 4701
The Chief Executive Officer of the entity	Advice	Schedule 10, Part 9, Division 2, Table 2.	Powerlink PO Box 1193 VIRGINIA QLD 4014

7. Currency Period for the Approval

This development approval will lapse at the end of the period set out in section 85 of the *Planning Act 2016.*

8. Statement of Reasons

Description of the development	The development is for a Material Change of Use for an Integrated Animal Husbandry and Public Facility – Other (Solar PV & Battery Energy Storage System)
Assessment Benchmarks	The proposed development was assessed against the following assessment benchmarks: - Rural Zone Code - Animals Code - Development Standards Code - Natural Features & Conservation Area Overlay Code (Catchment Overlay) - Economic Resources Overlay Code (Agricultural Land - Class Overlay) - Major Utilities Overlay Code (Electricity Transmission Line Overlay) - Natural Disaster Overlay Code

Reasons for Decision	**Desired Environmental Outcomes (DEO)** The DEO relevant to the site and proposed development relate to the protection and sustainable use of productive agricultural land. The subject land is identified as suitable for grazing and cropping purposes and the proposed use allows grazing to continue during and cropping to be recommenced at the end of life of this project. The proposed development and the conditions attached to the approval ensure that the achievement of the DEO is not compromised. **Assessment Benchmarks** Rural Zone Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO1, PO3, PO5 and PO7. Natural Features and Conservation Area Overlays Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO1, PO2, PO3 and PO4. Economic Resources Overlays Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO1. Major Utilities Overlays Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO3. Natural Disaster Overlays Code The proposed development complies with the relevant Performance Outcomes. Animals Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO1. Development Standards Code The proposed development complies with the relevant Performance Outcomes or conditions have been imposed to ensure compliance, specifically for PO1, PO2, PO3, PO5, PO6, PO8, PO11, Po12, PO13 and PO17

9. Appeal rights

The rights of an applicant to appeal to a tribunal or the Planning and Environment Court against a decision about a development application are set out in chapter 6, part 1 of the Planning Act 2016. For particular applications, there may also be a right to make an application for a declaration by a tribunal (see chapter 6, part 2 of the Planning Act 2016).

Appeal by an applicant

An applicant for a development application may appeal to the Planning and Environment Court against the following:

- the refusal of all or part of the development application
- a provision of the development approval
- the decision to give a preliminary approval when a development permit was applied for
- a deemed refusal of the development application.

An applicant may also have a right to appeal to the Development tribunal. For more information, see schedule 1 of the Planning Act 2016.

Appeal by a submitter

A submitter for a development application may appeal to the Planning and Environment Court against:

- any part of the development application for the development approval that required impact assessment
- a variation request.

The timeframes for starting an appeal in the Planning and Environment Court are set out in section 229 of the Planning Act 2016.

Attachment 2 is an extract from the Planning Act 2016 that sets down the applicant's appeal rights and the appeal rights of a submitter.

The Planning and Environment Court appeals database lists all the appeals lodged in the Planning and Environment Court since 15 March 2008, which the department has been notified of. It contains information about the appeal, including the appeal number, site address, local government area, and a copy of the appeal notice, including grounds for the appeal. The appeal database is an easy way for anyone to obtain information about an appeal or check if an appeal has been lodged for a specific development application or approval.

The appeal database is available at https://planning.dsdmip.qld.gov.au/planning/our-planning-system/dispute-resolution.

Should you require further assistance in relation to this matter, please do not hesitate to contact Council's Development Services section on (07) 4992 9500, quoting you application number of MCU011-22/23.

Yours Sincerely

Chris Welch
DIRECTOR COUNCIL SERVICES

CC All Referral Agencies (both advice and concurrence)
State Assessment and Referral Agency (SARA)
rockhamptonSARA@dilgp.qld.gov.au

Powerlink
PO Box 1193
VIRGINIA QLD 4014

Enc Attachment 1 – Part A Conditions imposed by the Assessment Manager
Attachment 1 – Part B Assessment Manager Notes
Attachment 1 – Part C Conditions imposed by SARA
Attachment 1 – Part D PowerLink Advice
Attachment 2 – Appeal Rights
Attachment 3 – Approved Drawings
Attachment 4 – Infrastructure Charges

MCU011-22/23 Attachment 1

Part A - Conditions imposed by the Assessment Manager

General

1 **(Amended 25 October 2023)** The development is to be completed and carried out generally in accordance with the following approved plans and reports, except where modified by the conditions of this Development Approval:

Plan/Document number	Plan/Document name	Date
PR151484-1	Preliminary Site Layout prepared by RPS	Received by Council on ~~20 December 2022~~ **08 September 2023**
MJ2370-TIA (Revision C)	Traffic Impact Statement prepared by Northern Consulting Engineers	30/11/2022
EDF-002 (Revision A)	Surface Water and Flood Impact Assessment Report prepared by Civil IQ	9 November 2022
Version 2	Landscape Character and Visual Impact Assessment prepared by Accent Environmental	Received by Council on 17 March 2023

2 Comply with all of the conditions of this Development Approval prior to the commencement of the use, unless otherwise stated within this Decision Notice, and maintain compliance for the duration of the approved use.

3 Exercise the approval and complete all associated works, including any relocation or installation of services, at no cost to Council.

4 Alterations to public utilities, mains and services made necessary in connection with any of the works arising from this approval including works to restore and reinstate all roads are to be completed at no cost to Council.

Amended Plans

5 Final detailed layout plans of the solar farm facility are to be submitted to Council for approval prior to the commencement of the use. The plans at a minimum must show:
a. all building and structure locations;

b. substation locations;
c. inverter locations;
d. above and below ground cabling;
e. internal access roads;
f. boundary setbacks;
g. solar panel system type;
h. solar plant configuration; and
i. fencing associated with the use;

Approved Use

6 The approved use of the premises is for Integrated Animal Husbandry and Public Facility – Other (Solar PV & Battery Energy Storage System.

7 **(Retained 25 October 2023)** The approved use may operate for a maximum of 35 years from the date the facility, or part thereof, becomes operational after which the site must be returned to agricultural use.

8 **(Amended 25 October 2023)** The approved Public Facility – Other must operate in conjunction with Animal Husbandry ~~**activities**~~ **use** at all times**, where practicable,** following the reestablishment of grasses and groundcover after construction of the solar panel arrays **so that groundcover would adequately support Animal Husbandry activities. Where the Animal Husbandry use is inhinited by unforeseen circumstances relating to animal welfare, the applicant will notify Council providing detailed information to support suspension of the Animal Husbandry use and anticipated schedule for the resumption of activities**.

9 **(Amended 25 October 2023)** Operating mechanisms for the movement of solar arrays must ~~**be a minimum height to avoid**~~ **minimise** any conflict with grazing animals under the arrays**, as identified in a risk assessment to be prepared and submitted to Council, prior to the operation of the Animal Husbandry use**.

Building and other works

10 The applicant shall obtain a development permit prior to commencement of any works defined as building work under the Building Act 1975.

11 All habitable buildings must be located a minimum of 30 metres from any electricity transmission line.

12 The maximum height of any building must not exceed 15 meters above natural ground level. This does not include any aerials, chimneys, flagpoles, load-bearing antenna or transmission line.

13 Buildings, structures or other housings enclosing higher risk facilities such as battery storage are fitted with systems that manage fire and heat risks to avoid placing undue pressure on existing emergency services.

14 Plumbing and Drainage work permit is to be obtained for any sanitary facilities or plumbing fixtures regulated under the *Plumbing and Drainage Act 2018.*

15 **(Amended 25 October 2023)** Proposed earthworks are limited to the establishment of building pads, hardstand areas, internal roads, vehicle parking areas and minor re-profiling of land beneath the solar arrays. ~~**Buildings, roads, driveways and other works are sited and constructed without the need for cut and fill earthworks exceeding 2,000m3 of material. A development permit is required for all Operational Works**~~ **Earthworks will be be sited and constructed in accordance with an approved Operational Works permit to ensure that development is integrated with the landform and landscape of the site and surrounding area.**

Setbacks and screening

16 Prior to commencement of construction of the development, the applicant must negotiate with the landowner (at the date of the approval) of land located as Lot 1 RP612153 (described as R4 in the approved Landscape Character and Visual Impact Assessment) on an agreed and reasonable solution to the visual impact of the development on this land. Evidence of such agreement is to be provided to Council prior to the commencement of construction.

In the instance that an agreement cannot be reached with the landowner prior to the commencement of construction, the Applicant must negotiate a reasonable alternative solution with Council prior to operations commencing.

17 Except where in conflict with other conditions of this approval, the location of landscape buffers is in accordance with the approved plan and Landscape Character and Visual Impact Assessment.

18 Project infrastructure is setback a minimum of 10 metres from all site boundaries except where greater distances are required for landscaping by Condition 20 of this approval.

19 **(Amended 25 October 2023)** Project infrastructure is setback ~~**100**~~ **50** metres from the centreline of watercourses and 4 metres from the centreline of any drainage features (unless a greater distance is specified in the Approved Surface Water and Flood Impact Assessment Report or an Erosion and Sediment Control Plan).

20 **(Amended 25 October 2023)** Prior to installation of development infrastructure, excluding site fencing, screen landscaping in accordance with Condition 64 below is planted ~~**for a distance of 20 metres from the southern edge of the development footprint as identified on the approved plan and 10 metres from the site boundaries with Shorts Road**~~ **in the location illustrated on approved site plan (ref: 151484-1) and in accordance with the approved Landscape Character and Visual Impact Assessment (dated 28 February 2023). The landscaping along Shorts Road is to be 5 metres in width, with the landscape vegetation height in line with Condition 64 of this approval. The applicant is to submit an amended Preliminary Site Layour Plan for Council Approval, identifying the extent of the landscape buffer in accordance with the figure below, excluding site access areas.**

21 No development occurs within the easement associated with the electricity transmission lines traversing the site, unless permitted in the advice provided by Powerlink (as attached).

22 No animal enclosures (e.g., stables, barns, holding yards) are permitted within 100 metres of a residence not associated with the use.

Road work and access

23 **(Amended 25 October 2023)** Prior to commencement of development works, Shorts Road must be upgraded to address the safety deficiencies identified in the approved Traffic Impact Statement. The works must be

undertaken in accordance with an Operational Works approval and as per the following:

a. Shorts Road is to be upgraded to a minimum 2-coat seal in accordance with the Capricorn Municipal Design Guidelines, **subject to a minimum width of 6.5 metres**. Erosion and sediment controls are to be implemented during this construction process.
b. Signage warning of the isolated humps within the roadway section are to be installed to improve awareness for unfamiliar drivers.

24 Prior to undertaking any road upgrade works identified in the conditions of this development permit, provide a bank guarantee for an amount equivalent to 10% of the value of the road upgrade works.

25 Shorts Road is to be maintained by the applicant during the construction process. This road reserve is to remain of a standard consistent of the pre-development or better, in addition to the works detailed in Condition 23. The maintenance is to be undertaken in accordance with an Operational Works approval until the development is deemed practically complete.

26 Any damage to the existing road surface, services or furniture as a result of the development is to be repaired to the pre-existing or better condition within a reasonable period at no cost to Council.

27 Vehicle access is to be provided in accordance with the approved plan and be the subject of an Operational Works approval. Each access shall be constructed as per the proposed plan and in accordance with the requirements of the Capricorn Municipal Development Guidelines (Standard Drawing CMDG-R-040) for a rural access. Please note that the dimensions listed on this standard drawing are considered the minimum required for compliance.

Vehicle Parking and Manoeuvring Areas

28 The proposed internal access roads are to be constructed so as to be a minimum of 4 metres wide and be constructed of a suitably designed surface in accordance with an Operational Works approval.

29 Provide sufficient parking and manoeuvring, loading/unloading space on-site for all vehicles at the operational phase. All car parking spaces for employees of the proposed development must be:

I. clearly line marked and/or delineated to the satisfaction of the Assessment Manager; and;
II. located on the site and be fully contained within the title boundaries.

30 No vehicle storage or parking is permitted on the adjoining road reserve.

31 All vehicles accessing the site must be able to enter and exit in a forward gear.

Water and Sewerage Infrastructure

32 At the time of lodging a building application, documentation is required to be submitted to Council that demonstrates that a reasonable water supply for potable (including adequate storage for a minimum 45,000 litres capacity volume) is available for the development.

33 A separate storage system that permanently holds a minimum 5,000 litres is located within 50m of new buildings, exclusively for firefighting purposes. The storage system must allow for immediate access by emergency services plant.

34 The minimum standard of wastewater treatment to be considered is secondary treatment incorporating disinfection. Appropriate reserve disposal areas are to be provided and maintained on the site.

35 Prior to the commencement of use, an effluent disposal/storage system, appropriate for the proposed development, is to be installed in accordance with a detailed report prepared by a suitably qualified person. All relevant approvals for this system, in accordance with the requirements of the *Plumbing and Drainage Act*, are to be obtained before installation.

36 The proposed effluent disposal/storage system is to be maintained so that all effluent is wholly contained within the confines of the development site and does not pond or enter any gully, watercourse, stormwater system or adjoining properties.

Stormwater

37 The development does not adversely interfere with the existing hydrological regime of adjoining properties or catchments

38 Stormwater Management is to be undertaken in accordance with the approved Surface Water and Flood Impact Assessment Report.

39 All stormwater infrastructure must be designed and constructed, prior to the commencement of use, as per the requirements of the Approved Surface Water and Flood Impact Assessment Report.

40 The stormwater drainage system must be designed so that the development will not make material changes to the pre-development location, duration, frequency or concentration of overland stormwater flow at the point of discharge to all downstream properties, including road reserves. In the event that a material change to the pre-development stormwater flows cannot be avoided, provide written evidence to Council's satisfaction of a legal right to discharge stormwater over the downstream land in the proposed method.

41 Ponding of stormwater resulting from the development must not occur on adjacent properties.

42 Stormwater formerly flowing onto the site must not be diverted onto other properties.

43 All stormwater being discharged from the site is to meet the requirements of the Capricorn Municipal Development Guidelines and the Queensland Water Quality Guidelines 2009.

44 Contaminated water must not be directly or indirectly released from the premises onto the ground or into the groundwater at the premises.

45 Releases to stormwater must not cause any visible oil slick or other visible evidence of oil or grease, nor contain visible grease, scum, litter or floating oil.

Erosion and Sediment Control

46 A detailed Erosion and Sediment Control Plan, and associated engineered drawings, prepared by a Registered Professional Engineer of Queensland in accordance with the *Capricorn Municipal Design Guidelines*, is to be provided to Council as part of the Operation Works application.

47 During construction, the Developer is to undertake sedimentation and erosion control management in accordance with the approved Erosion and Sediment Management Plan.

48 Erosion and sediment control measures are to be maintained post-construction until disturbed areas are permanently stabilised through vegetation and/or landscaping.

Construction Environmental Management Plan

49 A Construction Environmental Management Plan (CEMP), prepared and certified by a suitably qualified person, is to be provided to Council as part of the Operation Works application. The CEMP is to ensure all potential impacts of the development are adequately controlled and provide detailed practical and achievable prevention, minimisation and mitigation strategies (including design standards) for controlling environmental impacts of the development.

50 The applicant must implement the recommendations of the approved CEMP including any recommended works, installation of monitoring equipment and management measures at all times during construction of the development.

Operational Environmental Management Plan

51 The Applicant must prepare and submit a detailed Operational Environmental Management Plan (OEMP), prepared and certified by a suitably qualified person, identifying environmental management measures to be implemented during operation of the development to Council for approval at least 40 working days prior to operations commencing. The plan must be approved by Council before operations commence.

52 The applicant must implement the recommendations of the approved OEMP including any recommended works, installation of monitoring equipment and management measures at all times during operation of the development.

Vegetation management

53 Vegetation clearing undertaken as a consequence of development occurs in compliance with the *Vegetation Management Act 1999* and *Nature Conservation Act 1992.*

54 Maintain development buffers to watercourses and/or drainage features in accordance with the approved plan and Condition 19 to minimise the removal of vegetation.

Biosecurity

55 Imported soil/fill used as part of works must be certified as weed free. A Weed Hygiene Declaration for the soil is to be provided to Council on request.

56 Invasive biosecurity matters must be managed to prevent or minimise the harmful effects a biosecurity risk could have on adjacent agricultural land uses and environmental qualities. A biosecurity plan is to be prepared, approved by Council and implemented which identifies pertinent biosecurity risks and details reasonable and practical measures to prevent or minimise the biosecurity risks.

Amenity

57 The photovoltaic panels, any visible support structures, framing, cabling, or other equipment and infrastructure shall have a non-reflective or matte finish.

58 In the event that panels become 'out-of-sync' (i.e., not tracking the sun such that the panels are perpendicular to the sun), the affected panels are to be repaired as soon as reasonably practicable; or removed; or adjusted to remain in a fixed stowed position (so that potential for reflection is minimised for any sensitive receptors) until the repair is completed.

59 The applicant must ensure that when undertaking any on-site or external works, including any filling and excavation, appropriate dust control measures are implemented in accordance with the *Environmental Protection Act 1994* and complies with the relevant air quality objectives defined in the Environmental Protection (Air) Policy 2019.

60 Operations on the site must have no significant impact on the amenity of adjoining premises or the surrounding area due to the emission of light, noise, dust or odour.

61 When requested by Council, nuisance monitoring must be undertaken and recorded within 3 months, to investigate any genuine complaint of nuisance caused by noise, light, odour or dust. An analysis of the monitoring data and a report, including nuisance mitigation measures, must be provided to Council within 14 days of the completion of the investigation.

62 Any lighting devices associated with the development, such as sensory lighting, must be positioned on the development site and shielded so as not to cause glare or other nuisance to nearby residents and motorists. Night lighting must be designed, constructed and operated in accordance with Australian Standard *AS4282 Control of the obtrusive effects of outdoor lighting*.

Landscaping

63 Prior to the commencement of the installation of any infrastructure associated with the use, the applicant is to submit for approval to Council, a landscaping plan in accordance with Section 6 of the approved Landscape Character and Visual Impact Assessment showing the vegetated buffers identified in Condition 20. The landscaping plan must include:

a. Identification of any existing vegetation to be retained as part of site landscaping;
b. A list of plantings, the species to be used, containing predominantly species that are endemic to Central Queensland;
c. The location of plantings, spaced to achieve a dense screen;
d. Sections through each area of landscaping showing the mature heights of the planted native vegetation
e. A watering and maintenance plan during the establishment phase;
f. An ongoing maintenance and replanting program.

64 **(Amended 25 October 2023)** Landscaping provided to the perimeter of the site, including road frontages traversing the site, has predominantly low to mid storey planting to screen view of the solar infrastructure more effectively from adjacent areas. Landscaping of internal areas (e.g., watercourses, drainage lines, etc.) ~~**focuses on taller species to screen views from areas further away**~~ **will be provided where the opportunity arise, that would not be in conflict with the stormwater drainage system's compatibility across the site and the approved activities**.

65 The landscaping is to be maintained in a tidy manner by the developer (i.e., watering, fertilising, mulching, weeding, and the like) at all times to the satisfaction of the Assessment Manager.

66 Any significant trees to be retained as part of proposed landscaping are to be protected during construction.

Fencing and signage

67 The applicant must install safety / security fencing a minimum of 1.8 metres in height along all property boundaries to prevent unauthorised or accidental public entry. The fencing must not obscure sight lines at corners or intersections. Where fronting the Biloela-Callide Road stock route, fencing must be stock resistant to prevent travelling stock entering the site.

68 The applicant must install industry standard warning signage on all boundaries of the site, at regular intervals, warning of the safety hazards associated with the approved use.

69 Erect and maintain a single sign with a minimum area of six square metres adjacent to each access for the approved use. The sign must display as a minimum:
the name of the business operating on the premises;
the maximum onsite speed limit of 20km/h;
contact details for complaints and the site office.

70 All fencing must be completed prior to the commencement of use.

Waste

71 The applicant is required to prepare a Waste Management Plan for the proposed development. The plan should include, but is not limited to, the following:

a. A description of the development activities that may generate waste
b. The types and amount of waste that might be generated by the activities
c. how the waste will be dealt with, including a description of the types and amounts of waste that will be dealt with under each waste management practices under the waste hierarchy
d. procedures for identifying and implementing opportunities to minimise the amount of waste generated, promote efficiency in the use of resources, and otherwise improve the waste management practices employed
e. procedures for dealing with accidents, spills and other incidents that may impact waste management
f. how often the waste management practices will be assessed

72 Recycling and waste must use appropriately licensed facilities.

73 Waste must not be burned at the premises.

74 No disposal of any solar panels or associated material is permitted to municipal landfill facilities.

Site rehabilitation

75 **(Retained 25 October 2023)** Bank guarantees are to be provided to Council at the commencement of construction of the development to be held against the cost of rehabilitating the site post-operation. The amount of the bank guarantee is to be agreed between the developer and Council and is to represent a reasonable estimation of rehabilitation costs less the estimated value of the renewable energy equipment situated on the site. The value of the bond is to be reviewed annually at the request of Council or the developer.

76 A minimum of 12 months prior to operations associated with the approved use ceasing on the premises the applicant must provide a Site Rehabilitation Plan (SRP) to Council detailing proposed site rehabilitation works.

77 Within 12 months of operations associated with the approved use ceasing on the premises and decommissioning of the facility, land that has been disturbed must be rehabilitated in a manner such that:

permits the rural use of the site to be re-established (unless otherwise agreed with the landholder or relevant authorities); and

the final landform is stable, potential erosion is minimised and protects public safety.

78 Decommissioning activities to be undertaken as part of site rehabilitation (cessation of approved use) must include:

a. disconnection of the connection point at the substation
b. disconnection and removal of the solar panels for recycling
c. removal of all buildings and equipment, with materials recycled wherever possible
d. removal of steel framework/supports and cabling for recycling
e. removal of underground infrastructure
f. removal of fencing (unless otherwise agreed by the landholder or relevant authorities)

79 Within 6 months of the site rehabilitation works being completed the applicant must submit a Site Conditions Report detailing the condition of the site following the recommended works stipulated in the SRP.

MCU011-22/23 Attachment 1

Part B – Assessment Manager Notes

A The approved development must also comply with Council's current Local Laws under the *Local Government Act 2009*.

B The applicant and or owner/s of the land and the person/s responsible for the management of the premise is/are to ensure ongoing compliance with conditions of this Development Permit including Conditions relating to the ongoing use of the premise, and the design and layout of the development.

C Pursuant to section 75 of the *Local Government Act 2009*, Council's written approval is required to carry out works on a road, or interfere with a road or its operation. This requirement applies to all Council-controlled roads within its local government area. The process for obtaining approval is set out in Council's *Local Law No. 1 (Administration) 2011.* Approval must be obtained prior to the commencement of the works.

D Please note the statements dated 21 February 2023 from Powerlink as an advice agency and attached to this Decision Notice.

E Where further development is proposed it is the applicant's / developer's responsibility to ensure further approvals are sought as required by the Banana Shire Planning Scheme.

Engineering

F. Prior to commencing any of the following construction activities the applicant/developer will be required to obtain a development permit for operational work:

i. Internal and external roadworks;
ii. earthworks;
iii. stormwater drainage ;
iv. erosion and sediment control;
v. internal and external lighting; and
vi. landscaping.

G. All works required by this approval shall be undertaken and completed in accordance with Council's Standards - Capricorn Municipal Development Guidelines (www.cmdg.com.au) at the Applicant's expense.

H. Any works on roads shall be conducted in accordance with the Queensland Department of Transport and Main Roads, "Manual of Uniform Traffic Control Devices – Part 3".

Cultural Heritage

I It is advised that under section 23 of the *Aboriginal Cultural Heritage Act 2003*, a person who carries out an activity must take all reasonable and practicable measures to ensure the activity does not harm Aboriginal cultural heritage (the "cultural heritage duty of care"). Maximum penalties for breaching the duty of care are listed in the Aboriginal cultural heritage legislation. The information on Aboriginal cultural heritage is available on the Department of Aboriginal and Torres Strait Islander and Partnerships website: www.datsip.qld.gov.au

Biosecurity

J Section 23 of the *Biosecurity Act 2014* outlines the General Biosecurity Obligation. All landowners have a General Biosecurity Obligation (GBO) for managing biosecurity risks that are under their control and that they know about or should reasonably be expected to know about. All individuals and organisations whose activities pose or is likely to pose a biosecurity risk must:

- take all reasonable and practical measures to prevent or minimise the biosecurity risk
- minimise the likelihood of causing a biosecurity event and limit the consequences if such an event occurs
- prevent or minimise the harmful effects a biosecurity risk could have
- not do anything that might make any harmful effects of a biosecurity risk worse

A biosecurity risk exists when you deal with any pest, disease, weed or contaminant. This includes moving an animal, plant, turf, soil, machinery and/or equipment that could carry a pest, disease, weed or contaminant.

Environmental Nuisance

K It is an offence under section 440 of the *Environmental Protection Act 1994* to cause environmental nuisance to adjacent premises or other property during construction work. Environmental nuisance includes unreasonable interference caused by noise, dust, fumes, odour, smoke, aerosols, particles or light.

L It is the developer's responsibility to ensure compliance with the *Environmental Protection Act 1994*, which prohibits any construction, building and earthworks activities likely to cause nuisance noise (including the entry and departure of heavy vehicles) between the hours of 6.30 pm and 6.30 am from Monday to Saturday and at all times on Sundays or Public Holidays.

General Environmental Duty

M The *Environmental Protection Act 1994* lists obligations and duties to prevent environmental harm, nuisances and contamination. The two primary duties that apply to everyone in Queensland are:

general environmental duty – which means a person must not carry out any activity that causes or is likely to cause environmental harm, unless measures to prevent or minimise the harm have been taken; and
duty to notify of environmental harm – to inform the administering authority and landowner or occupier when an incident has occurred that may have caused or threatens serious or material environmental harm.

N In carrying out the activity or works associated with the development, all reasonable and practical measures are to be taken to minimise releases and the likelihood of releases of contaminants to the environment, except as otherwise provided by the conditions of this development approval.

Mosquito breeding

O The site is required to be appropriately drained so that water is not allowed to accumulate or pond in a manner that may allow mosquito breeding, as required under the *Public Health Regulation 2005.*

Nature Conservation (Plants and Animals)

P It is an offence under section 335 of the Nature Conservation (Animals) Regulation 2020 to remove, or tamper with, an animal breeding place that is being used by a protected animal to incubate or rear the animal's offspring.

Animal breeding places include obvious structures such as bird nests and tree hollows, as well as more cryptic places such as amphibian or reptile habitat where breeding takes place. Where activities are likely to impact on an animal breeding place, the applicant should contact the Queensland Department of Environment and Science to discuss if additional actions are required to be undertaken to meet obligations under the *Nature Conservation Act 1992.*

Vegetation Management

Q The *Vegetation Management Act 1999* regulates the clearing of vegetation in Queensland. No interference or clearing of vegetation is to be undertaken (unless the clearing is exempt, a development approval authorising the clearing has been obtained or the clearing is authorised in accordance with a code). Contact the Queensland Department of Resources should you require any further information on these matters.

Water & Sewerage

R Subsequent applications will be required for Operational Works, Building and Plumbing/Drainage Works. Building works are to comply with the *Building Act 1975*, the Building Code of Australia and other relevant authorities.

S All new taps and plumbing fixtures on the site are to be installed and maintained with approved water saving devices in accordance with current legislative and Council requirements (AAA rating or better). The installation shall include but not be limited to approved water efficient shower heads, flow restrictors/aerators on internal taps, dual flush toilets, etc. In addition approved water efficient washing machines, dishwashers and other appliances shall be the only appliances installed on the site. Pre and post installation inspections shall be arranged with Council's Plumbing Inspector.

T Hydraulic Services plans will be required to be submitted to Council for Plumbing and Drainage approval. These plans must show all drinking, non-drinking, heated, rainwater, sanitary plumbing, sanitary drainage and trade waste services.

Amenity

U Air and light emissions must be appropriately managed to prevent environmental nuisance beyond the boundaries of the property during all stages of the development including earthworks and construction.

V Suitable dust suppression should be used, where required during excavation and building works, to reduce the emission of dust or other such emissions from the site.

W Artificial illumination should not cause a nuisance to occupants of nearby premises and any passing traffic. Security and flood lighting is to be directed away from adjacent premises to minimise the protrusion of light outside the site.

Water & Stormwater

X It is an offence under the *Environmental Protection Act 1994* to discharge sand, silt, mud and other such contaminants to a stormwater drain, roadside gutter or a watercourse.

Y It is an offence under the *Environmental Protection Act 1994* to discharge or permit a prescribed water contaminant to enter a stormwater drain, roadside gutter or a watercourse. Prescribed contaminants include a wide variety of contaminants listed in Schedule 9 of the *Environmental Protection Act 1994*.

Waste Management

Z It is an offence under the *Waste Reduction and Recycling Act 2011* to leave litter behind or allow litter to blow from site. All waste must be appropriately contained on site prior to removal.

AA Trap Gully Landfill is the only approved waste facility within the Banana Shire for the disposal of commercial waste. No commercial waste is to be deposited at other Banana Shire landfills or transfer stations without prior written approval from Council.

MCU011-22/23 Attachment 1

Part C - Conditions imposed by the SARA

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Queensland Government

SARA reference: 2301-32909 SRA
Council reference: MCU011-22/23

28 February 2023

Chief Executive Officer
Banana Shire Council
PO Box 412
Biloela QLD 4715
enquiries@banana.qld.gov.au

Attention: Rentia Robertson

Dear Sir/Madam

SARA referral agency response— 551 and 641 Biloela Callide Road and Shorts Road, Mount Murchison

(Referral agency response given under section 56 of the *Planning Act 2016*)

The development application described below was confirmed as properly referred by the State Assessment and Referral Agency (SARA) on 17 January 2023.

Response

Outcome:	Referral agency response – with conditions
Date of response:	28 February 2023
Conditions:	The conditions in **Attachment 1** must be attached to any development approval
Advice:	Advice to the applicant is in **Attachment 2**
Reasons:	The reasons for the referral agency response are in **Attachment 3**

Development details

Description:	Development Permit	Material Change of Use - Solar Farm and Animal Husbandry
SARA role:	Referral agency	
SARA trigger:	Schedule 10, Part 9, Division 4, Subdivision 2, Table 4, Item 1 (Planning Regulation 2017)	

Fitzroy/Central regional office
Level 2, 209 Bolsover Street, Rockhampton
PO Box 113, Rockhampton QLD 4700

	Material change of use of premises within 25m of a state-controlled road and railway corridor
SARA reference:	2301-32909 SRA
Assessment manager:	Banana Shire Council
Street address:	551 and 641 Biloela Callide Road and Shorts Road, Mount Murchison
Real property description:	Lot 154 on SP126053; Lot 28 on RN519; Lot 2 on RP619032; Lot 3 on RP608599
Applicant name:	Edify Energy C/- RPS AAP Consulting Pty Ltd
Applicant contact details:	PO BOX 1559 Fortitude Valley QLD 4006 harry.connolly@rpsgroup.com.au
State-controlled road access permit:	This referral included an application for a changed road access under section 62A(2) of *Transport Infrastructure Act 1994*. Below are the details of the decision: • Approved • Reference: TMR23-038441 • Date: 17 February 2023 If you are seeking further information on the road access permit, please contact the Department of Transport and Main Roads at Central.Queensland.IDAS@tmr.qld.gov.au
Human Rights Act 2019 considerations:	A consideration of the 23 fundamental human rights protected under the *Human Rights Act 2019* has been undertaken as part of this decision. It has been determined that this decision does not limit human rights.

Representations

An applicant may make representations to a concurrence agency, at any time before the application is decided, about changing a matter in the referral agency response (s.30 Development Assessment Rules). Copies of the relevant provisions are in **Attachment 4**.

A copy of this response has been sent to the applicant for their information.

For further information please contact Leanne Simpson, Principal Planning Officer, on 5352 9707 or via email RockhamptonSARA@dsdilgp.qld.gov.au who will be pleased to assist.

Yours sincerely

Steve Conner
Executive Director

cc Edify Energy, harry.connolly@rpsgroup.com.au

enc Attachment 1 - Referral agency conditions
Attachment 2 - Advice to the applicant
Attachment 3 - Reasons for referral agency response
Attachment 4 - Representations about a referral agency response provisions

Attachment 1—Referral agency conditions

(Under section 56(1)(b)(i) of the *Planning Act 2016* the following conditions must be attached to any development approval relating to this application)

No.	Conditions	Condition timing
Material Change of Use – Solar Farm and Animal Husbandry		
10.9.4.2.4.1 – Material change of use near a state transport corridor—The chief executive administering the *Planning Act 2016* nominates the Director-General of Department of Transport and Main Roads to be the enforcement authority for the development to which this development approval relates for the administration and enforcement of any matter relating to the following condition(s):		
1.	(a) Provide road works comprising basic left turn (BAL) at Biloela Callide Road / Shorts Road intersection. (b) Design and construct the road works in accordance with the TMR Road Planning and Design Manual 2nd Edition to cater for a 19m semi-trailer.	Prior to commencement of use
2.	(a) Permanently close and remove the existing vehicular property access located at Lot 2 on RP619032 and Biloela Callide Road. (b) Reinstate the table drain between the pavement edge and the property boundary in accordance with the TMR Road Planning and Design Manual at no cost to the Department of Transport and Main Roads.	Prior to commencement of use
3.	(a) Stormwater management resulting from the development must ensure no worsening or actionable nuisance to the state-controlled road. (b) Any works on the land must not: (i) create any new discharge points for stormwater runoff onto the state-controlled road (ii) interfere with and/or cause damage to the existing stormwater drainage on the state-controlled road (iii) surcharge any existing culvert or drain on the state-controlled road (iv) reduce the quality of stormwater discharge onto the state-controlled road.	At all times

Attachment 2—Advice to the applicant

General advice	
1.	Terms and phrases used in this document are defined in the *Planning Act 2016*, its regulation or the State Development Assessment Provisions (SDAP) (version 3.0). If a word remains undefined it has its ordinary meaning.
2.	Under section 33 of the *Transport Infrastructure Act 1994,* written approval is required from the Department of Transport and Main Roads to carry out road works on a state-controlled road. Please contact the Department of Transport and Main Roads on 4931 1500 to make an application for road works approval. This approval must be obtained prior to commencing any works in the state-controlled road reserve. The approval process will require the approval of engineering designs of the proposed works, certified by a Registered Professional Engineer of Queensland (RPEQ).

Attachment 3—Reasons for referral agency response

(Given under section 56(7) of the *Planning Act 2016*)

The reasons for the SARA's decision are:

- The development application is for a Material Change of Use – Solar Farm and Animal Husbandry at 551 and 641 Biloela Callide Road and Shorts Road, Mount Murchison
- The site is within 25m of a state-controlled road, being Biloela Callide Road, and within 25m of a railway corridor
- The development proposes a changed access to the state-controlled road, being closure of a redundant vehicular access, requiring an access decision under s62A of the *Transport Infrastructure Act 1994*
- The application requires assessment by SARA against the State Development Assessment Provisions (SDAP), version 3.0, State code 1: Development in a state-controlled road environment and State code 2: Development in a railway environment
- SARA has assessed the development against State codes 1 and 2 and found the development complies with the relevant performance outcomes, subject to conditions to:
    - upgrade the Biloela Callide Road / Shorts Road intersection to provide a basic left turn (BAL) suitable to accommodate a 19m semi-trailer
    - close the existing vehicular property access at Lot 2 on RP619032 and reinstatement of the table drain
    - ensure that the impacts of stormwater events associated with development are minimised and managed to avoid creating any adverse impacts on the state-controlled road corridor.

Material used in the assessment of the application:

- the development application material and submitted plans
- *Planning Act 2016*
- Planning Regulation 2017
- the SDAP (version 3.0), as published by SARA
- the Development Assessment Rules
- SARA DA Mapping system
- section 58 of the *Human Rights Act 2019*

Attachment 4—Representations about a referral agency response provisions

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Development Assessment Rules—Representations about a referral agency response

The following provisions are those set out in sections 28 and 30 of the Development Assessment Rules[1] regarding **representations about a referral agency response**

Part 6: Changes to the application and referral agency responses

28 Concurrence agency changes its response or gives a late response

28.1. Despite part 2, a concurrence agency may, after its referral agency assessment period and any further period agreed ends, change its referral agency response or give a late referral agency response before the application is decided, subject to section 28.2 and 28.3.

28.2. A concurrence agency may change its referral agency response at any time before the application is decided if—

(a) the change is in response to a change which the assessment manager is satisfied is a change under section 26.1; or

(b) the Minister has given the concurrence agency a direction under section 99 of the Act; or

(c) the applicant has given written agreement to the change to the referral agency response.[2]

28.3. A concurrence agency may give a late referral agency response before the application is decided, if the applicant has given written agreement to the late referral agency response.

28.4. If a concurrence agency proposes to change its referral agency response under section 28.2(a), the concurrence agency must—

(a) give notice of its intention to change its referral agency response to the assessment manager and a copy to the applicant within 5 days of receiving notice of the change under section 25.1; and

(b) the concurrence agency has 10 days from the day of giving notice under paragraph (a), or a further period agreed between the applicant and the concurrence agency, to give an amended referral agency response to the assessment manager and a copy to the applicant.

---

1 Pursuant to Section 68 of the *Planning Act 2016*

2 In the instance an applicant has made representations to the concurrence agency under section 30, and the concurrence agency agrees to make the change included in the representations, section 28.2(c) is taken to have been satisfied.

Part 7: Miscellaneous

30 Representations about a referral agency response

30.1. An applicant may make representations to a concurrence agency at any time before the application is decided, about changing a matter in the referral agency response.[3]

[3] An applicant may elect, under section 32, to stop the assessment manager's decision period in which to take this action. If a concurrence agency wishes to amend their response in relation to representations made under this section, they must do so in accordance with section 28.

Our ref TMR23-038441
Your ref
Enquiries Jason Giddy

Queensland Government

Department of
Transport and Main Roads

17 February 2023

Decision Notice – Permitted Road Access Location
(s62(1) *Transport Infrastructure Act 1994*)

This is not an authorisation to commence work on a state-controlled road[1]

Development application reference number MCU011-22/12, lodged with Banana Shire Council involves constructing or changing a vehicular access between Lot 154SP126053 and Lot 2RP619032, the land the subject of the application, and 472 Biloela – Callide Road (a state-controlled road).

In accordance with section 62A(2) of the *Transport Infrastructure Act 1994* (TIA), this development application is also taken to be an application for a decision under section 62(1) of TIA.

Applicant Details

Name and address	Edify Energy c/- RPS AAP Consulting Pty Ltd PO Box 1559 Fortitude Valley QLD 4006

Application Details

Address of Property	551 Biloela Callide Road, Mount Murchison QLD 4715
Real Property Description	Lot 154SP126053 and Lot 2RP619032

Decision (given under section 67 of TIA)

It has been decided to approve the application, subject to the following conditions:

No.	Conditions of Approval	Condition Timing
1	The permitted road access location to Lot 154 on SP126053, is to be located at approximate chainage 5.557km (lat: -24.366656; long: 150.587398) on 472 Biloela – Callide Road.	At all times.
2	Direct access to the state-controlled road from Lot 2 on RP619032 is prohibited.	At all times.
3	The existing property access to Lot 2 on RP619032, located at approximate chainage 6.483km (lat: -24.363967; long: 150.596032) must be removed and the table drain reinstated.	Prior to commencement of use.

[1] Please refer to the further approvals required under the heading 'Further approvals'

Program Delivery and Operations
Fitzroy District
31 Knight Street North Rockhampton QLD 4701
PO Box 5096 Red Hill Rockhampton QLD 4701

Telephone +61 7 (07) 4931 1686
Website www.tmr.qld.gov.au
Email Central.Queensland.IDAS@tmr.qld.gov.au
ABN: 39 407 690 291

No.	Conditions of Approval	Condition Timing
4	The road access is to be constructed and maintained at no cost to the department in accordance with section 64(a) & (b) of the *Transport Infrastructure Act 1994*.	At all times.
5	The applicant shall be responsible for all maintenance works for the access in accordance with Module 9 of the Local Government Association of Queensland document 'TMR/Local Government Cost Sharing Arrangement', dated October 2017.	At all times.
6	All vehicles entering or exiting the property via the permitted access must travel in a forward direction only.	At all times.
7	Reasonable steps are taken to ensure that the permitted road access is used by others in accordance with these conditions.	At all times.

Reasons for the decision

The reasons for this decision are as follows:

a) To remove a redundant property access no longer required due to the material change of use removing the existing dwelling and therefore the need for access.
b) The use is intended to obtain access via Shorts Road.
c) To approve an existing property access that remains required to provide access to Lot 154SP126053.

Please refer to **Attachment A** for the findings on material questions of fact and the evidence or other material on which those findings were based.

Information about the Decision required to be given under section 67(2) of TIA

1. There is no guarantee of the continuation of road access arrangements, as this depends on future traffic safety and efficiency circumstances.

2. In accordance with section 70 of the TIA, the applicant for the planning application is bound by this decision. A copy of section 70 is attached as **Attachment B**, as required, for information.

Further information about the decision

1. In accordance with section 67(7) of TIA, this decision notice:
   a) starts to have effect when the development approval has effect; and
   b) stops having effect if the development approval lapses or is cancelled; and
   c) replaces any earlier decision made under section 62(1) in relation to the land.

2. In accordance with section 485 of the TIA and section 31 of the *Transport Planning and Coordination Act 1994* (TPCA), a person whose interests are affected by this decision may apply for a review of this decision only within 28 days after notice of the decision was given under the TIA. A copy of the review provisions under TIA and TPCA are attached in **Attachment C** for information.

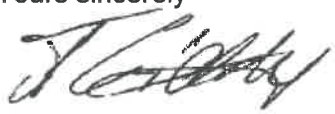
3. In accordance with section 485B of the TIA and section 35 of TPCA a person may appeal against a reviewed decision. The person must have applied to have the decision reviewed before an appeal about the decision can be lodged in the Planning and Environment Court. A copy of the Appeal Provisions under TIA and TPCA is attached in **Attachment C** for information.

Further approvals
The Department of Transport and Main Roads also provides the following information in relation to this approval:

1. Road Access Works Approval Required – Written approval is required from the department to carry out road works that are road access works (including driveways) on a state-controlled road in accordance with section 33 of the TIA. This approval must be obtained prior to commencing any works on the state-controlled road. The approval process may require the approval of engineering designs of the proposed works, certified by a Registered Professional Engineer of Queensland (RPEQ). Please contact the department to make an application.

If further information about this approval or any other related query is required, Mr Jason Giddy, Senior Town Planner should be contacted by email at CorridorManagement@tmr.qld.gov.au or on (07) 4931 1686.

Yours sincerely



Jason Giddy
Senior Town Planner

Attachments: Attachment A – Decision evidence and findings
Attachment B - Section 70 of TIA
Attachment C - Appeal Provisions

Attachment A

Decision Evidence and Findings

Findings on material questions of fact:

- Planning application (council ref. DA066-2022; SARA ref. 2301-32811 SRA) was referred to the Department of Transport and Main Roads (TMR) in accordance with Schedule 10 of the *Planning Regulation 2017*.
- The application is a Material Change of Use for a Solar Farm, Battery Storage and Animal Husbandry. The planning application is taken to be an application for a permitted access location in accordance with section 62(A) of the *Transport Infrastructure Act 1994* (TIA).
- The department assessed the application in accordance with the State Development Assessment Provisions v3.0, the provisions of the *Transport Infrastructure Act 1994* and the TMR Vehicular Access Policy (VAP).
- The use will obtain access via Shorts Road for all construction and operation activities. This results in the property access into Lot 2 RP619032 becoming redundant. The existing property access to Lot 154 SP126053 will remain as the use does not extend to the dwelling on this parcel. The access locations have been refused and approved respectively.
- As the property access into Lot 2 RP619032 is redundant, it must be removed to ensure access is obtained via the local road network in accordance with Principle 2, Strategy 2 of the TMR Vehicular Access Policy.

Evidence or other material on which findings were based:

- *Transport Infrastructure Act 1994*
- *Planning Act 2016*
- *Planning Regulation 2017*
- TMR Vehicular Access Policy

Attachment B

Section 70 of TIA

Transport Infrastructure Act 1994
Chapter 6 Road transport infrastructure
Part 5 Management of State-controlled roads

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70 Offences about road access locations and road access works, relating to decisions under s 62(1)

(1) This section applies to a person who has been given notice under section 67 or 68 of a decision under section 62(1) about access between a State-controlled road and adjacent land.

(2) A person to whom this section applies must not—

(a) obtain access between the land and the State-controlled road other than at a location at which access is permitted under the decision; or

(b) obtain access using road access works to which the decision applies, if the works do not comply with the decision and the noncompliance was within the person's control; or

(c) obtain any other access between the land and the road contrary to the decision; or

(d) use a road access location or road access works contrary to the decision; or

(e) contravene a condition stated in the decision; or

(f) permit another person to do a thing mentioned in paragraphs (a) to (e); or

(g) fail to remove road access works in accordance with the decision.

Maximum penalty—200 penalty units.

(3) However, subsection (2)(g) does not apply to a person who is bound by the decision because of section 68.

Attachment C

Appeal Provisions

Transport Infrastructure Act 1994
Chapter 16 General provisions

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485 Internal review of decisions

(1) A person whose interests are affected by a decision described in schedule 3 (the ***original decision***) may ask the chief executive to review the decision.

(2) The person is entitled to receive a statement of reasons for the original decision whether or not the provision under which the decision is made requires that the person be given a statement of reasons for the decision.

(3) The *Transport Planning and Coordination Act 1994*, part 5, division 2—

(a) applies to the review; and

(b) provides—

(i) for the procedure for applying for the review and the way it is to be carried out; and

(ii) that the person may apply to QCAT to have the original decision stayed.

485B Appeals against decisions

(1) This section applies in relation to an original decision if a court (the appeal court) is stated in schedule 3 for the decision.

(2) If the reviewed decision is not the decision sought by the applicant for the review, the applicant may appeal against the reviewed decision to the appeal court.

(3) The Transport Planning and Coordination Act 1994, part 5, division 3—

(a) applies to the appeal; and

(b) provides—

(i) for the procedure for the appeal and the way it is to be disposed of; and

(ii) that the person may apply to the appeal court to have the original decision stayed.

(4) Subsection (5) applies if—

(a) a person appeals to the Planning and Environment Court against a decision under section 62(1) on a planning application that is taken, under section 62A(2), to also be an application for a decision under section 62(1); and

(b) a person appeals to the Planning and Environment Court against a decision under the Planning Act on the planning application.

(5) The court may order—

(a) the appeals to be heard together or 1 immediately after the other; or

(b) 1 appeal to be stayed until the other is decided.

(6) Subsection (5) applies even if all or any of the parties to the appeals are not the same.

(7) In this section—

original decision means a decision described in schedule 3.

reviewed decision means the chief executive's decision on a review under section 485.

31 Applying for review

(1) A person may apply for a review of an original decision only within 28 days after notice of the original decision was given to the person under the transport Act.

(2) However, if—

(a) the notice did not state the reasons for the original decision; and

(b) the person asked for a statement of the reasons within the 28 days mentioned in subsection (1)

the person may apply within 28 days after the person is given the statement of the reasons.

(3) In addition, the chief executive may extend the period for applying.

(4) An application must be written and state in detail the grounds on which the person wants the original decision to be reviewed.

32 Stay of operation of original decision

(1) If a person applies for review of an original decision, the person may immediately apply for a stay of the decision to the relevant entity.

(2) The relevant entity may stay the original decision to secure the effectiveness of the review and any later appeal to or review by the relevant entity.

(3) In setting the time for hearing the application, the relevant entity must allow at least 3 business days between the day the application is filed with it and the hearing day.

(4) The chief executive is a party to the application.

(5) The person must serve a copy of the application showing the time and place of the hearing and any document filed in the relevant entity with it on the chief executive at least 2 business days before the hearing.

(6) The stay—

(a) may be given on conditions the relevant entity considers appropriate; and

(b) operates for the period specified by the relevant entity; and

(c) may be revoked or amended by the relevant entity.

(7) The period of a stay under this section must not extend past the time when the chief executive reviews the original decision and any later period the relevant entity allows the applicant to enable the applicant to appeal against the decision or apply for a review of the decision as provided under the QCAT Act.

(8) The making of an application does not affect the original decision, or the carrying out of the original decision, unless it is stayed.

(9) In this section—

relevant entity means—

(a) if the reviewed decision may be reviewed by QCAT—QCAT; or

(b) if the reviewed decision may be appealed to the appeal court—the appeal court.

35 Time for making appeals

(1) A person may appeal against a reviewed decision only within—

(a) if a decision notice is given to the person—28 days after the notice was given to the person; or

(b) if the chief executive is taken to have confirmed the decision under section 34(5)—56 days after the application was made.

(2) However, if—

(a) the decision notice did not state the reasons for the decision; and

(b) the person asked for a statement of the reasons within the 28 days mentioned in subsection (1)(a);

the person may apply within 28 days after the person is given a statement of the reasons.

(3) Also, the appeal court may extend the period for appealing.

MCU011-22/23 Attachment 1

Part D - PowerLink Advice

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20 February 2023

Our Ref: DA5119 (MSLink14269, 14264, 14266)

Banana Shire Council
PO Box 412
BILOELA QLD 4715

Attention: Rentia Robertson
Email: enquiries@banana.qld.gov.au
Application: MCU011-22/23

Edify Energy
C/- RPS AAP consulting Pty Ltd
PO BOX 1559
FORTITUDE VALLEY QLD 4006

Attention: Harry Connolly
Email: Harry.Connolly@rpsgroup.com.au

Dear Harry,

Referral Agency Response (Advice)

(Given under section 9.2 of the Development Assessment Rules)

Transmission Infrastructure Impacted	
Transmission Corridor	Callide A Moura (132kV) Transmission Line Corridor
Easement ID	Easement A on RN1176 (Dealing No. 601399533) Easement A on RP615786 (Dealing No. 601288121) Easement A on RN1178 (Dealing No. 601399528)
Location Details	
Street address	551 & 641 Biloela Callide Road, Mount Murchison
Real property description	Lot 154 on SP126053 Lot 2 on RP619032 Lot 28 on RN519 Lot 3 on RP608599
Local government area	Banana Shire Council
Application Details	
Proposed development:	Material change of use
Approval sought	Development Permit

We refer to the above referenced development application which has been referred to Powerlink Queensland in accordance with Section 54 of the *Planning Act 2016*.

In accordance with its jurisdiction under Schedule 10 Part 9 Division 2 of the *Planning Regulation 2017*, Powerlink Queensland is a Referral Agency (Advice) for the above development application.

Specifically, the application has been triggered for assessment by Powerlink Queensland because:

1. For **material change of use** – all or part of the premises are subject to a transmission entity easement which is part of the transmission supply network (Table 2 1b)

33 Harold Street, Virginia
PO Box 1193, Virginia, Queensland 4014, Australia
Telephone: (07) 3860 2111 Facsimile: (07) 3860 2100
www.powerlink.com.au

PLANS AND REPORTS ASSESSED

The following plans and reports have been reviewed by Powerlink Queensland and form the basis of our assessment. Any variation to these plans and reports may require amendment of our advice.

Table 1: Plans and Reports upon which the assessment is based

Drawing / Report Title	Prepared by	Dated	Reference No.	Version / Issue
Preliminary Site Layout	RPS	20/12/2022	PR151484-1	

Powerlink Queensland, acting as a Referral Agency (Advice) under the Planning Regulation 2017 provides its response to the application as attached **(Attachment 1)**.

Please treat this response as a properly made submission for the purposes of Powerlink being an eligible advice agency in accordance with the *Planning Act 2016*.

For further information please contact our Property Services Team via email property@powerlink.com.au who will be pleased to assist.

Yours sincerely

for: Narelle Titman
MANAGER PROPERTY

ATTACHMENT 1 – REFERRAL AGENCY (ADVICE) RESPONSE

Powerlink Queensland **supports** this application subject to the inclusion of the following conditions in the Assessment Manager's Decision Notice.

No.	Condition	Timing	Reason
1	The development must be carried out generally in accordance with the reviewed plans detailed in Table 1.	At all times.	To ensure that the development is carried out generally in accordance with the plans of development submitted with the application.
2	The statutory clearances set out in the *Electrical Safety Regulation 2013* must be maintained during construction and operation. No encroachment within the statutory clearances is permitted.	At all times.	To ensure that the purpose of the *Electrical Safety Act 2002* is achieved and electrical safety requirements are met.
3	Compliance with the terms and conditions of the easement dealing no. shown in the heading of this letter.	At all times.	To ensure that the existing rights contained in the registered easement dealings are maintained.
4	Compliance with the generic requirements in respect to proposed works in the vicinity of Powerlink Queensland infrastructure as detailed in the enclosed Annexure "A".	At all times.	To ensure that the purpose of *the Electrical Safety Act 2002* is achieved and electrical safety requirements are met. To ensure the integrity of the easement is maintained.

Advice to Council and the Applicant

1. Powerlink and Edify are currently negotiating network connection of the solar farm to the transmission grid. This correspondence does not constitute approval for connection which remains the subject of ongoing technical assessment and commercial negotiations. The exact location of connecting infrastructure is also part of ongoing negotiations. As a result we wish to advise council that the location of any infrastructure is likely to change, and as such its location should not form part of the approval.

2. Powerlink has previously met with members of the Callide Solar Power Station (CSPS) team (meeting October 2022), and discussed how Powerlink's corridor selection investigations for the Banana Range Wind Farm connection project apply to lot 2/RP619032 and lot 28/RN519. At that meeting, Edify representatives noted that all three corridor options potentially traverse north to south of the eastern edge of CSPS project land (where the project land shares a boundary with the Callide Power Station, CS Energy site), and this may result in a loss of approximately 3-5MW PV capacity if the easement sits within the CSPS project land. Further, it was noted that out of the 3 corridors proposed, the northern 1 corridor would have no further impact on CSPS project lands as it would not traverse over any further CSPS land (east to west). By way of an update, Powerlink can advise that the northern 1 corridor was recommended by our draft corridor selection report released in November. The latest information regarding the status of this project is available via our project page www.powerlink.com.au/projects/Banana-range-wind-farm-connection-project

3. Should any doubt exist in maintaining the prescribed clearance to electrical infrastructure the applicant is obliged under the *Electrical Safety Act 2002* to seek advice from Powerlink.

4. This response **does not constitute an approval to commence operational works within the easement.** Prior written approval is required from Powerlink Queensland before any work is undertaken within the easement area. All works on easement (including but not limited to earthworks, drainage and detention basins; road construction; underground and overhead service installation) require detailed submissions, assessments and consent (or otherwise) by Powerlink. Further, Powerlink may require that such drawings be provided in electronic format (3D DXF or equivalent of final design RL's AHD and MGA GDA94 in applicable zone)

5. In order for Powerlink to maintain and operate a safe and reliable supply of electricity, we require unrestricted 24-hour access to our corridors and infrastructure.

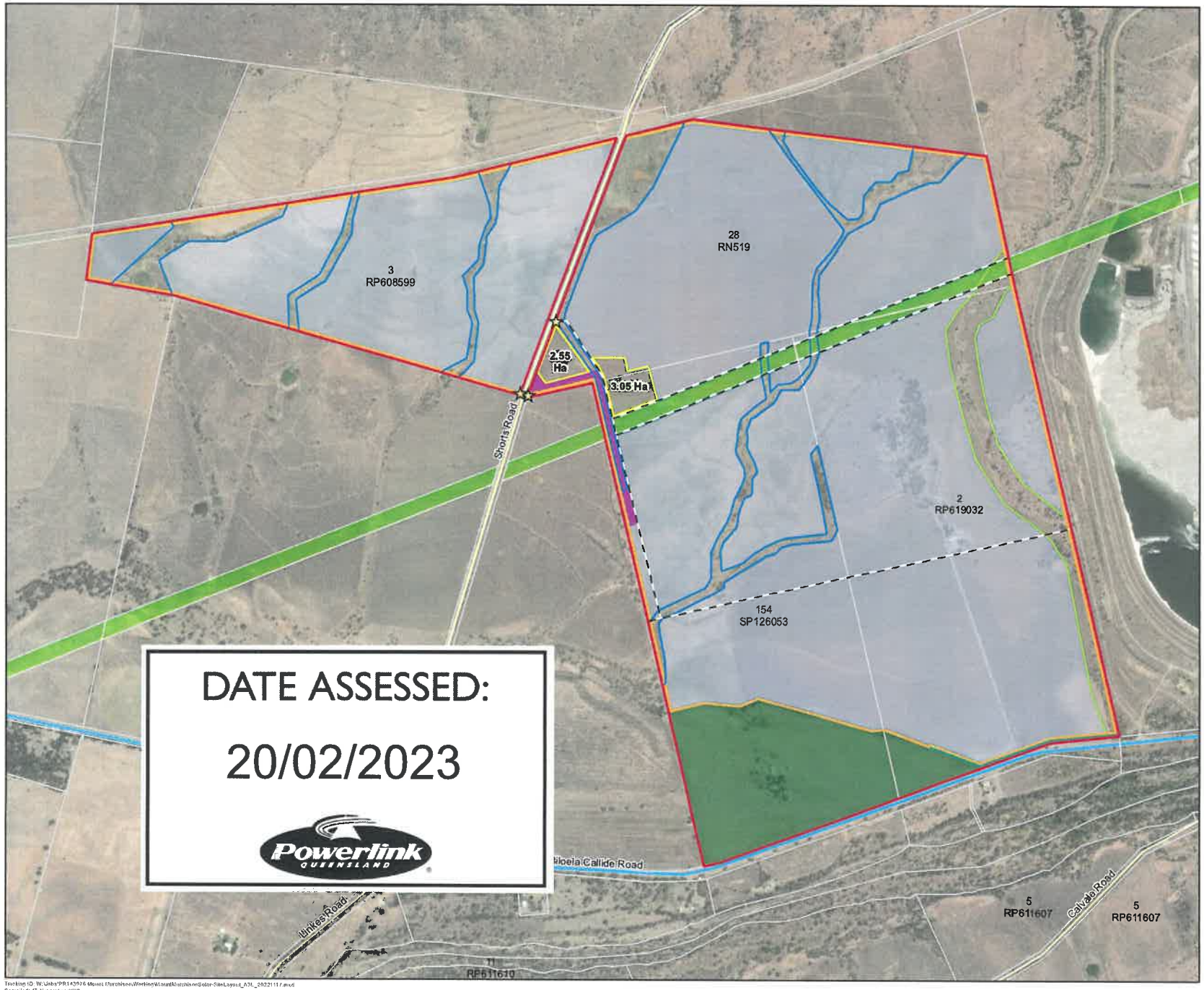
   We will require practical access (typically by 4WD vehicle – but to standard no less than existing) to the Powerlink structures.

   If it is envisaged that there will be any interference or alteration to our current access arrangements prior, during or after the completion of your works, we require that the applicant contacts our Works Control Manager Easements (Mr Ehren Wittmer – ph 0418 233 916)

6. Compliance with the Electrical Safety Act 2002 including any Code of Practice under the Act and the Electrical Safety Regulation 2013 including any safety exclusion zones defined in the Regulation.

   In respect of this application, the exclusion zone for untrained persons and for operating plant operated by untrained persons is three (3) metres from the 132,000-volt wires and exposed electrical parts.

   If works have the potential to come within the prescribed clearance to the conductors and electrical infrastructure, then the applicant must seek advice from Powerlink by completing the attached Application for Safety Advice – Form and submitting to property@powerlink.com.au



LEGEND

- Proposed / Existing Access
- New Farm Fences
- State Controlled Road
- Local Road
- Cadastre
- Site Boundary
- Solar Array Disturbance Footprint
- Vegetation Buffer (Up to 10m wide)
- Bushfire/Access Buffer (10m wide) and Security Fence
- Livestock Laneway / Yard
- Optional BESS Location / Site Office / Laydown Area / Substation
- Drainage Line
- Watercourse Buffer (approx. 100m wide)
- Transmission Line Easement
- Flood Area

N

0 250 500 750 1,000

Metres
APPROX SCALE 14,000 @ A3
GDA 2020 MGA Zone 56

Disclaimer: While all reasonable care has been taken to ensure the information contained on this map is up to date and accurate, no guarantee is given that the information portrayed is free from error or omission. Please verify the accuracy of all information prior to use.

DATA SOURCES
Imagery from Esri basemap layer.
Cadastral data © Department of Resources 2022

PR151484-1
Preliminary Site Layout
Callide Solar Power Station

Tracking ID: W:\Jobs\PR143976 Mount Murchison\Working\Layout\MurchisonSolar-SiteLayout_A3L_20221117.mxd
Compiled: 17 November 2022

ANNEXURE A – GENERIC REQUIREMENTS

The conditions contained in this Annexure have been compiled to assist persons (the applicant) intending to undertake work within the vicinity of high-voltage electrical installations and infrastructure owned or operated by Powerlink. The conditions are supplementary to the provisions of the Electrical Safety Act 2002, Electrical Safety Regulation 2013 and the Terms and Conditions of Registered Easements and other forms of Occupational Agreements hereinafter collectively referred to as the "Easement". Where any inconsistency exists between this Annexure and the Easement, the Easement shall take precedence.

1. POWERLINK INFRASTRUCTURE

You may not do any act or thing which jeopardises the foundations, ground anchorages, supports, towers or poles, including (without limitation) inundate or place, excavate or remove any soil, sand or gravel within a distance of twenty (20) metres surrounding the base of any tower, pole, foundation, ground anchorage or support.

2. STRUCTURES

No structures should be placed within twenty (20) metres of any part of a tower or structure foundation or within 5m of the conductor shadow area. Any structures on the easement require prior written consent from Powerlink.

3. EXCLUSION ZONES

Exclusion zones for operating plant are defined in Schedule 2 of the Electrical Safety Regulation 2013 for Untrained Persons. All Powerlink infrastructure should be regarded as "electrically live" and therefore potentially dangerous at all times.

In particular your attention is drawn to Schedule 2 of the Electrical Safety Regulation 2013 which defines exclusion zones for untrained persons in charge of operating plant or equipment in the vicinity of electrical facilities. If any doubt exists in meeting the prescribed clearance distances from the conductors, the applicant is obliged under this Act to seek advice from Powerlink.

4. ACCESS AND EGRESS

Powerlink shall at all times retain the right to unobstructed access to and egress from its infrastructure. Typically, access shall be by 4WD vehicle.

5. APPROVALS (ADDITIONAL)

Powerlink's consent to the proposal does not relieve the applicant from obtaining statutory, landowner or shire/local authority approvals.

6. MACHINERY

All mechanical equipment proposed for use within the easement must not infringe the exclusion zones prescribed in Schedule 2 of the Electrical Safety Regulation 2013. All operators of machinery, plant or equipment within the easement must be made aware of the presence of live high-voltage overhead wires. It is recommended that all persons entering the Easement be advised of the presence of the conductors as part of on site workplace safety inductions. The use of warning signs is also recommended.

ANNEXURE A – GENERIC REQUIREMENTS

7. EASEMENTS

All terms and conditions of the easement are to be observed. Note that the easement takes precedence over all subsequent registered easement documents. Copies of the easement together with the plan of the Easement can be purchased from the Department of Environment & Resource Management.

8. EXPENDITURE AND COST RECOVERY

Should Powerlink incur costs as a result of the applicant's proposal, all costs shall be recovered from the applicant.

Where Powerlink expects such costs to be in excess of $10 000.00, advanced payments may be requested.

9. EXPLOSIVES

Blasting within the vicinity (500 metres) of Powerlink infrastructure must comply with AS 2187. Proposed blasting within 100 metres of Powerlink infrastructure must be referred to Powerlink for a detailed assessment.

10. BURNING OFF OR THE LIGHTING OF FIRES

We strongly recommend that fires not be lit or permitted to burn within the transmission line corridor and in the vicinity of any electrical infrastructure placed on the land. Due to safety risks Powerlink's written approval should be sort.

11. GROUND LEVEL VARIATIONS

Overhead Conductors

Changes in ground level must not reduce statutory ground to conductor clearance distances as prescribed by the Electrical Safety Act 2002 and the Electrical Safety Regulation 2013.

Underground Cables

Any change to the ground level above installed underground cable is not permitted without express written agreement of Powerlink.

12. VEGETATION

Vegetation planted within an easement must not exceed 3.5 metres in height when fully matured. Powerlink reserves the right to remove vegetation to ensure the safe operation of the transmission line and, where necessary, to maintain access to infrastructure.

13. INDEMNITY

Any use of the Easement by the applicant in a way which is not permitted under the easement and which is not strictly in accordance with Powerlink's prior written approval is an unauthorised use. Powerlink is not liable for personal injury or death or for property loss or damage resulting from unauthorized use. If other parties make damage claims against Powerlink as a result of unauthorized use then Powerlink reserves the right to recover those damages from the applicant.

ANNEXURE A – GENERIC REQUIREMENTS

14. INTERFERENCE

The applicant's attention is drawn to s.230 of the Electricity Act 1994 (the "Act"), which provides that a person must not wilfully, and unlawfully interfere with an electricity entity's works. "Works" are defined in s.12 (1) of the Act. The maximum penalty for breach of s.230 of the Act is a fine equal to 40 penalty units or up to 6 months imprisonment.

15. REMEDIAL ACTION

Should remedial action be necessary by Powerlink as a result of the proposal, the applicant will be liable for all costs incurred.

16. OWNERS USE OF LAND

The owner may use the easement land for any lawful purpose consistent with the terms of the registered easement; the conditions contained herein, the Electrical Safety Act 2002 and the Electrical Safety Regulation 2013.

17. ELECTRIC AND MAGNETIC FIELDS

Electric and Magnetic Fields (EMF) occur everywhere electricity is used (e.g. in homes and offices) as well as where electricity is transported (electricity networks).

Powerlink recognises that there is community interest about Electric and Magnetic Fields. We rely on expert advice on this matter from recognised health authorities in Australia and around the world. In Australia, the Federal Government agency charged with responsibility for regulation of EMFs is the Australian Radiation Protection and Nuclear Safety Agency (ARPANSA). ARPANSA's *Fact Sheet – Magnetic and Electric Fields from Power Lines*, concludes:

"On balance, the scientific evidence does not indicate that exposure to 50Hz EMF's found around the home, the office or near powerlines is a hazard to human health."

Whilst there is no scientifically proven causal link between EMF and human health, Powerlink nevertheless follows an approach of "*prudent avoidance*" in the design and siting of new powerlines. This includes seeking to locate new powerline easements away from houses, schools and other buildings, where it is practical to do so and the added cost is modest.

The level of EMF decreases rapidly with distance from the source. EMF readings at the edge of a typical Powerlink easement are generally similar to those encountered by people in their daily activities at home or at work. And in the case of most Powerlink lines, at about 100 metres from the line, the EMF level is so small that it cannot be measured.

Powerlink is a member of the ENA's EMF Committee that monitors and compiles up-to-date information about EMF on behalf of all electricity network businesses in Australia. This includes subscribing to an international monitoring service that keeps the industry informed about any new developments regarding EMF such as new research studies, literature and research reviews, publications, and conferences.

We encourage community members with an interest in EMF to visit ARPANSA's website: www.arpansa.gov.au Information on EMF is also available on the ENA's website: www.ena.asn.au

Attachment 2
Planning Act 2016 Extract on Appeal Rights

Part 1 Appeal rights

229 Appeals to tribunal or P&E Court

(1) Schedule 1 states–
(a) matters that may be appealed to–
(i) either a tribunal or the P&E Court; or
(ii) only a tribunal; or
(iii) only the P&E Court; and
(b) the person–
(i) who may appeal a matter (the appellant); and
(ii) who is a respondent in an appeal of the matter; and
(iii) who is a co-respondent in an appeal of the matter; and
(iv) who may elect to be a co-respondent in an appeal of the matter.

(2) An appellant may start an appeal within the appeal period.

(3) The appeal period is–
(a) For an appeal by a building advisory agency–10 business days after a decision notice for the decision is given to the agency; or
(b) For an appeal against a deemed refusal–at any time after the deemed refusal happens; or
(c) for an appeal against a decision of the Minister, under chapter 7, part 4, to register premises or to renew the registration of premises–20 business days after a notice is published under section 269(3)(a) or (4); or
(d) for an appeal against an infrastructure charges notice–20 business days after the infrastructure charges notice is given to the person; or
(e) for an appeal about a deemed approval of a development application for which a decision notice has not been given–30 business days after the applicant gives the deemed approval notice to the assessment manager; or
(f) for any other appeal–20 business days after a notice of the decision for the matter, including an enforcement notice, is given to the person. Note– See the P&E Court Act for the court's power to extend the appeal period.

(4) Each respondent and co-respondent for an appeal may be heard in the appeal.

(5) If an appeal is only about a referral agency's response, the assessment manager may apply to the tribunal or P&E Court to withdraw from the appeal.

(6) To remove any doubt, it is declared that an appeal against an infrastructure charges notice must not be about–
(a) the adopted charge itself; or
(b) for a decision about an offset or refund–
(i) the establishment cost of trunk infrastructure identified in a LGIP; or
(ii) The cost of infrastructure decided using the method included in the local government's charges resolution.

Attachment 3
Approved Drawings

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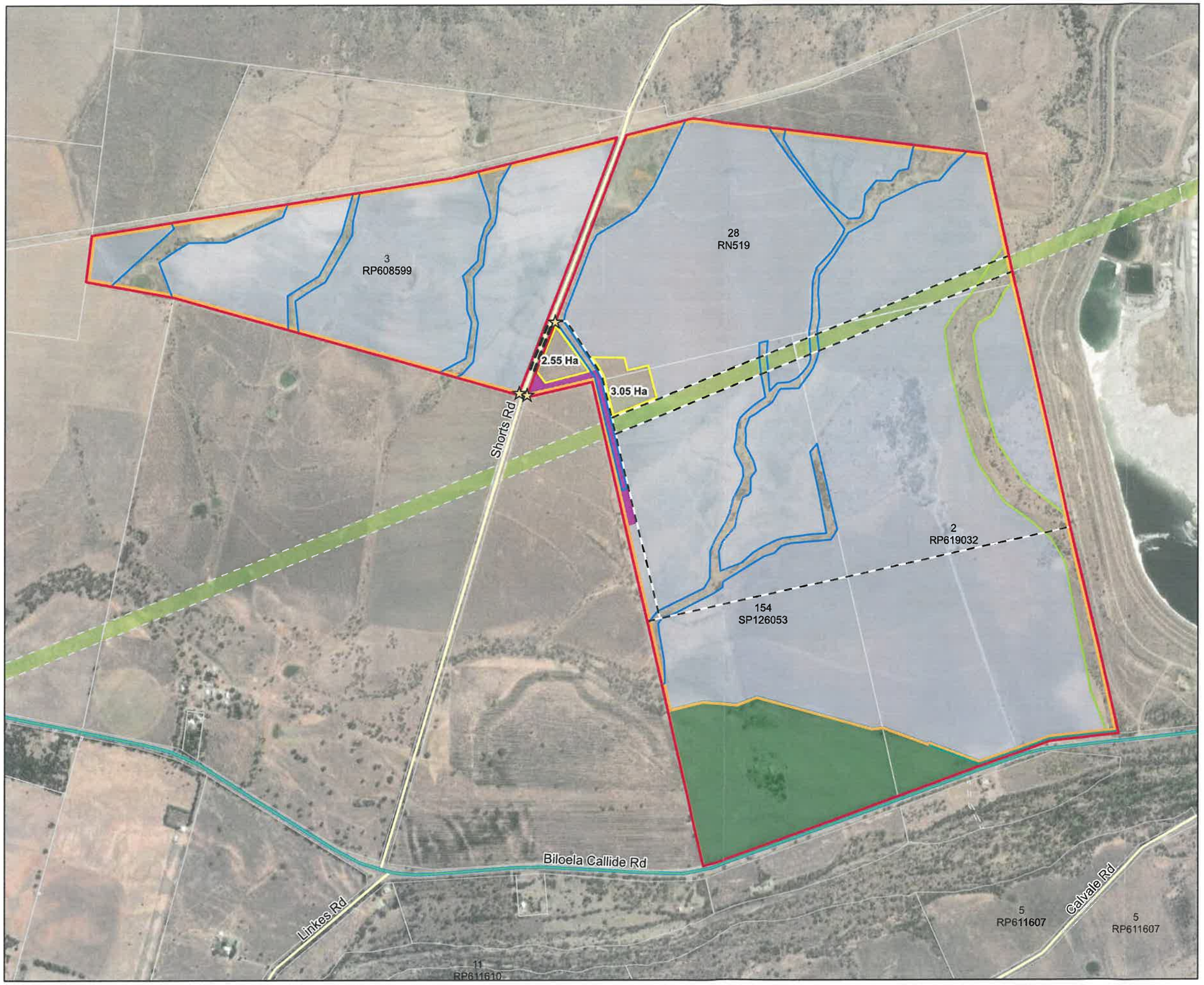

LEGEND

- Proposed / Existing Access
- Site Boundary
- Solar Array Disturbance Footprint
- Vegetation Buffer (7m wide)
- Bushfire/Access Buffer (10m wide) and Security Fence
- Livestock Laneway / Yard
- Optional BESS Location / Site Office / Laydown Area / Substation
- Drainage Line
- Watercourse Buffer (approx. 100m wide)
- Transmission Line Easement
- Flood Area
- Easement
- Strata
- Cadastre
- New Farm Fences
- State Controlled Road
- Local Road
- Shorts Road Vegetation Buffer (5m Wide)

Banana Shire Council
PLANNING APPROVAL

25 OCT 2023

mcu011122/23

N

0 250 500 750 1,000

Metres
APPROX SCALE @ A3
GDA 2020 MGA Zone 56

Disclaimer: While all reasonable care has been taken to ensure the information contained on this map is up to date and accurate, no guarantee is given that the information portrayed is free from error or omission. Please verify the accuracy of all information prior to use.

DATA SOURCES
Imagery from Esri basemap layer.
Cadastral data © Department of Resources 2022

PR151484-1
Preliminary Site Layout
Mount Murchison Solar Project

Tracking ID: C:\OnedrivePortal\RPS Group\AAP Geospatial Intelligence Services - Projects\ProjectFiles\Edify\Callide Solar Power Station\03_Projects\MountMurchisonSolar.aprx
Compiled: 06 September 2023

NORTHERN CONSULTING engineers

TRAFFIC IMPACT ASSESSMENT (STATE AGENCY)

CALLIDE SOLAR POWER STATION
551 & 641 BILOELA CALLID ROAD & SHORTS ROAD, MOUNT MURCHISON

Banana Shire Council
PLANNING APPROVAL

19 JUL 2023

mau11-22/23

FOR
EDIFY ENERGY

JOB No: MJ2370
Doc Ref: MJ2370-TIA

Phone: 07 4725 5550
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Email: mail@nceng.com.au
50 Punari Street Currajong Qld 4812
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NORTHERN CONSULTING engineers

DOCUMENT CONTROL

Rev	Author	Reviewed	Approved		Date	Issued To:	Purpose
A	Brendan Blair	Derek Saw	Derek Saw (RPEQ7363)		18/11/2022	Edify Energy	Draft - For review
B	Brendan Blair	Derek Saw	Derek Saw (RPEQ7363)		28/11/2022	Edify Energy	In support of DA
C	Brendan Blair	Derek Saw	Derek Saw (RPEQ7363)		30/11/2022	Edify Energy	Figure 1-1 and Appendix A updated
					-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-			

NORTHERN CONSULTING engineers

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NORTHERN CONSULTING engineers

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NORTHERN CONSULTING engineers

EXECUTIVE SUMMARY

Northern Consulting Engineers (NCE) have been commissioned by Edify Energy to undertake a Traffic Impact Assessment (TIA) relating to the proposed solar power station at 551 & 641 Biloela Callide Road & Shorts Road, Mount Murchison. The development site is situated on lands described as Lot 154 SP126053, Lot 2 RP619032, Lot 28 RN519 and Lot 3 RP608599.

The proposed development involves the establishment of a solar power station with a solar generating capacity of 200MW and a battery energy storage system (BESS) capable of storing 200MW/800MWh.

This report summarises the analysis and results of the traffic study associated with the proposed development, including the likely impacts and mitigation measures required to ensure the development can proceed whilst maintaining an acceptable level of service within the state-controlled road network.

As survey data was not available for the assessed intersection NCE completed calculations utilising from the available TMR Traffic Analysis and Reporting System (TARS) data. The 10-year background traffic growth rates from the TMR data were adopted and applied to the TMC data to forecast future background traffic.

Development generated traffic rates were determined separately for both the construction and operation phases of the development. A first principles assessment of the development generated traffic has been used due to a lack of available data relating to the generation of traffic from this type of development. Edify Energy has provided NCE with information pertaining to the generation of traffic for the construction and operation of previous solar farms. NCE have utilised calculations from past solar farms and the information provided by Edify Energy to calculate the number of containers of construction material and operational traffic to be generated per MW of solar farm.

A SIDRA analysis concluded that the existing state-controlled Biloela Callide Road / Shorts Road / Linkes Road intersection performed satisfactorily without and with the development with the average delay in the worst-case scenario not exceeding 26.0 seconds (LOS D).

An intersection turn warrant assessment was completed for the Shorts Road turn movements which were found to only require a proper basic left turn configuration. No right turn treatment is required as the existing conditions and development do not utilise that movement. NCE have provided a sketch of the widening required for a proper basic left turn treatment.

A swept path assessment found that the existing intersection configuration was not adequate for a 19m prime mover and semi-trailer design vehicle. NCE have provided a sketch of the extra pavement and upgrades required to the intersection to accommodate this type of vehicle.

Sight distance checks were completed for the stopping sight distance and safe intersection sight distance for each approach at the Biloela Callide Road / Shorts Road / Linkes Road intersection. Generally, the stopping sight distances were adequate, aside from the Linkes Road approach which provides appropriate signage to warn traffic of the upcoming intersection and stop sign. Not enough sight distance is provided along the Biloela Callide Road for the safe intersection sight distance requirements. This is already addressed by signage warning of the upcoming intersection. As the stopping sight distance is provided NCE believe the current intersection creates no major safety risks.

NCE provide the AADT data over the impacted railway level crossing for assessment of the impacted railway level crossing on the Dawson Highway.

1.0 INTRODUCTION

1.1 Background

Northern Consulting Engineers (NCE) have been commissioned by Edify Energy to undertake a Traffic Impact Assessment (TIA) relating to the proposed solar power station at 551 & 641 Biloela Callide Road & Shorts Road, Mount Murchison. The development site is situated on lands described as Lot 154 SP126053, Lot 2 RP619032, Lot 28 RN519 and Lot 3 RP608599.

1.2 Previous work

NCE are not aware of any previous traffic studies relating to the site.

1.3 Study area and report purpose

The proposed development is located within the Banana Shire Council (BSC) rural locality of Mount Murchison (QLD, 4715). The site is to be located over multiple land parcels listed below.

- Lot 154 on SP126053
- Lot 2 on RP619032
- Lot 28 on RN519
- Lot 3 on RP608599

The land is zoned rural in BSC Zoning Map as indicated in **Figure 1-1** below. The development is located 8km north east of the rural town of Biloela. The Biloela Callide Road runs parallel to the southern boundary of the development site with the Moura System rail line bordering the northern boundaries.

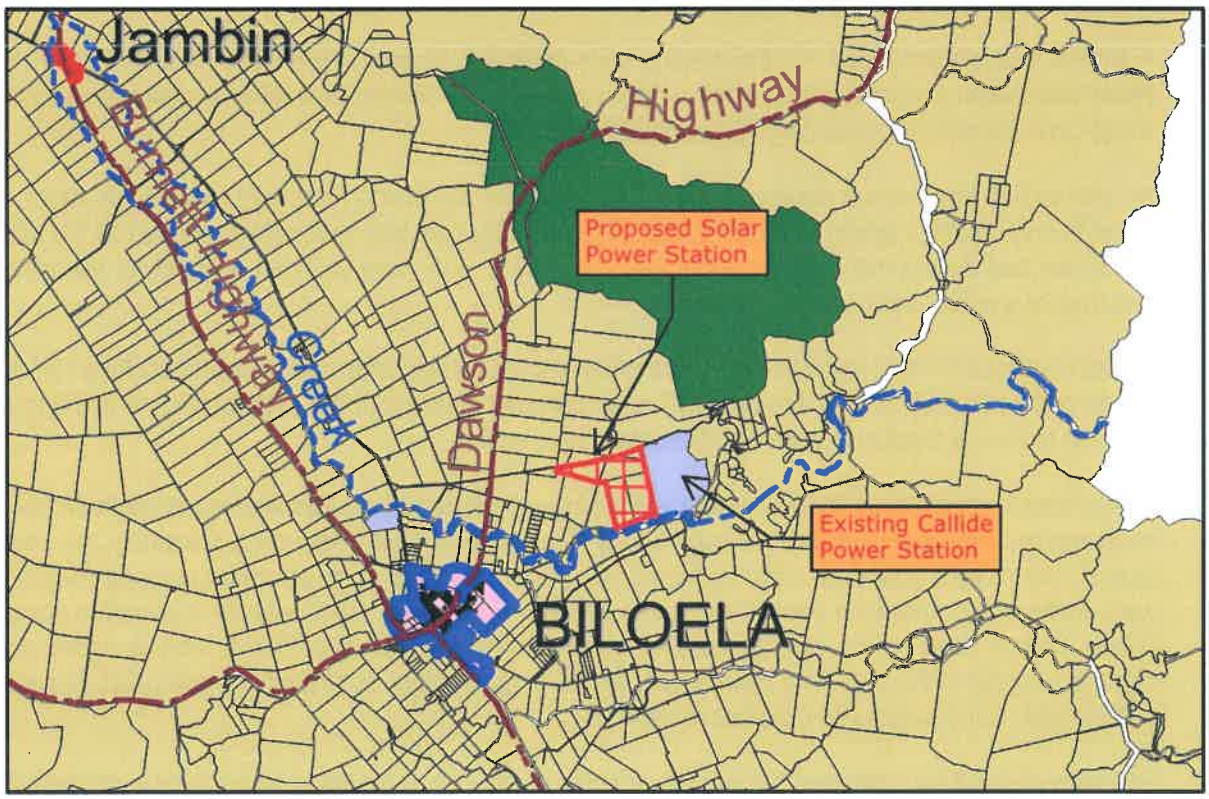

Figure 1-1 Site locality and zoning

Appendix A shows the full zoning map.

The purpose of the report is to document the traffic analysis undertaken, which has focused on the potential impacts upon the Biloela Callide Road and Shorts Roads inclusive of the intersection between Biloela Callide Road and Shorts Road, as a direct result of the traffic generated from the proposed development.

1.4 Proposed development

The proposed development involves the establishment of a solar power station with a solar generating capacity of 200MW and a battery energy storage system (BESS) capable of storing 200MW/800MWh. **Figure 1-2** depicts the preliminary site layout plan.

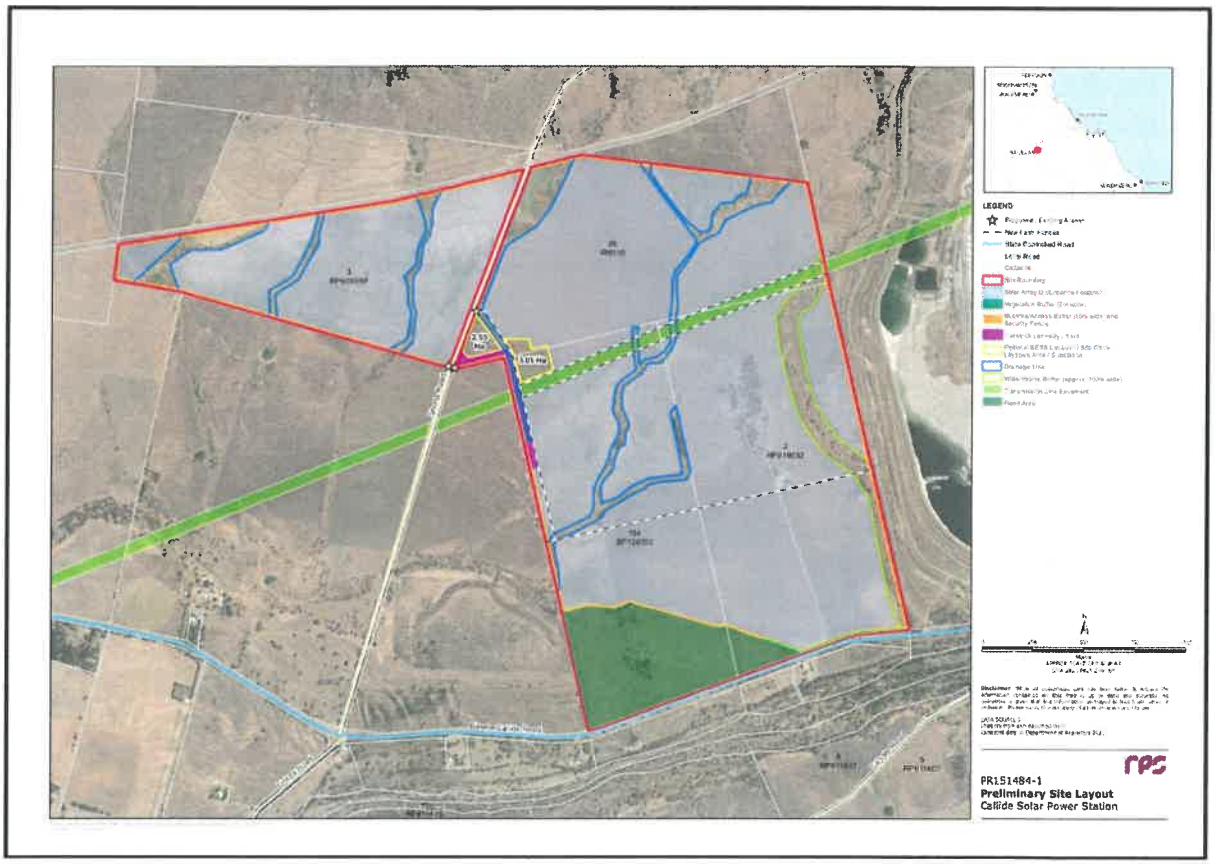

Figure 1-2 Preliminary site layout

1.4.1 Proposed access and parking

All generated traffic to/from the development is expected to use Shorts Road and by extension the Biloela Callide Road in order to gain access to the site. The proposed and existing access locations are indicated in in **Figure 1-2.** Therefore, all generated traffic during both the operation and construction will be required to use the Biloela Callide Road / Shorts Road intersection.

The specific footprint for on-site parking has not been indicated, however, it is expected that all traffic will be contained on-site during the construction and operation of the facilities. It is expected that traffic generated during the operation of the development site will park in/around the anticipated substation footprint. A total of 12 parking bays are currently planned to be provided as part of the site works and operation, no specific dimensions have been provided.

Internal roads will be required to be constructed to provide access to various work fronts for construction and maintenance. The internal roads are expected to be unsealed, single to dual vehicle width and include areas for manoeuvring. The internal roads will be located entirely within the development footprint. Indicative

internal roads are yet to be mapped and thus a driveway and frontage assessment has not been completed. For a review of Shorts Road refer to the Capability and Suitability Report provided in **Appendix B**.

2.0 EXISTING CONDITIONS

2.1 Land use and zoning

The proposed development is over land with an existing use of small crops and fodder that is not irrigated. There are a number of existing drainage lines to be maintained as well as vegetation management watercourse through the north-east to south-east boundaries of the lot. A portion of the south western corner of the lot has also been indicated as a flood area and will not be developed. The development will ensure that all existing drainage lines and watercourses will not be impacted/developed and the development will avoid these areas. Refer to **Figure 1-2** which indicates the drainage and flooding areas.

2.2 Surrounding road network details

The surrounding road network is made up of both local government roadways and State Controlled Roadways.

The proposed development is situated within the Central Region (D6) District of the Queensland Department of Transport and Main Roads (TMR), the district mapping from TMR is provided in **Appendix C**. The adjacent State Controlled Road Network (SCRN) comprises:

- Biloela Callide Road – Road Section 472 – Site IDs 60126 & 61286 – access corridor connecting the Callide Power Station and Mine with the Dawson Highway. Designated as a PBS 2A route for up to 25m b-doubles (B25 route).
- Dawson Highway – Road Section 46A – Site IDs 60067, 61084 & 160067 – highway connecting Gladstone to Biloela. Designated as a PBS 2A route for up to 25m b-doubles (B25 route).

Figure 2-1 is an excerpt from Queensland Globe showing the heavy vehicles routes and restrictions for the areas expected to be used by the development generated traffic, labels are based on the most up to date routes and restrictions given by the NHVR.

The existing local road network consists of the following:

- Shorts Road – unsealed minor road. Utilised by local traffic only from the surrounding rural use lots. Only accessible by general access vehicles.
- Linkes Road – sealed minor road. Connects traffic from Biloela through to Biloela Callide Road via Calvale Road. Only accessible by general access vehicles.

Figure 2-2 shows the surrounding road network to be utilised by the development.

Figure 2-1 Heavy vehicle routes and restrictions

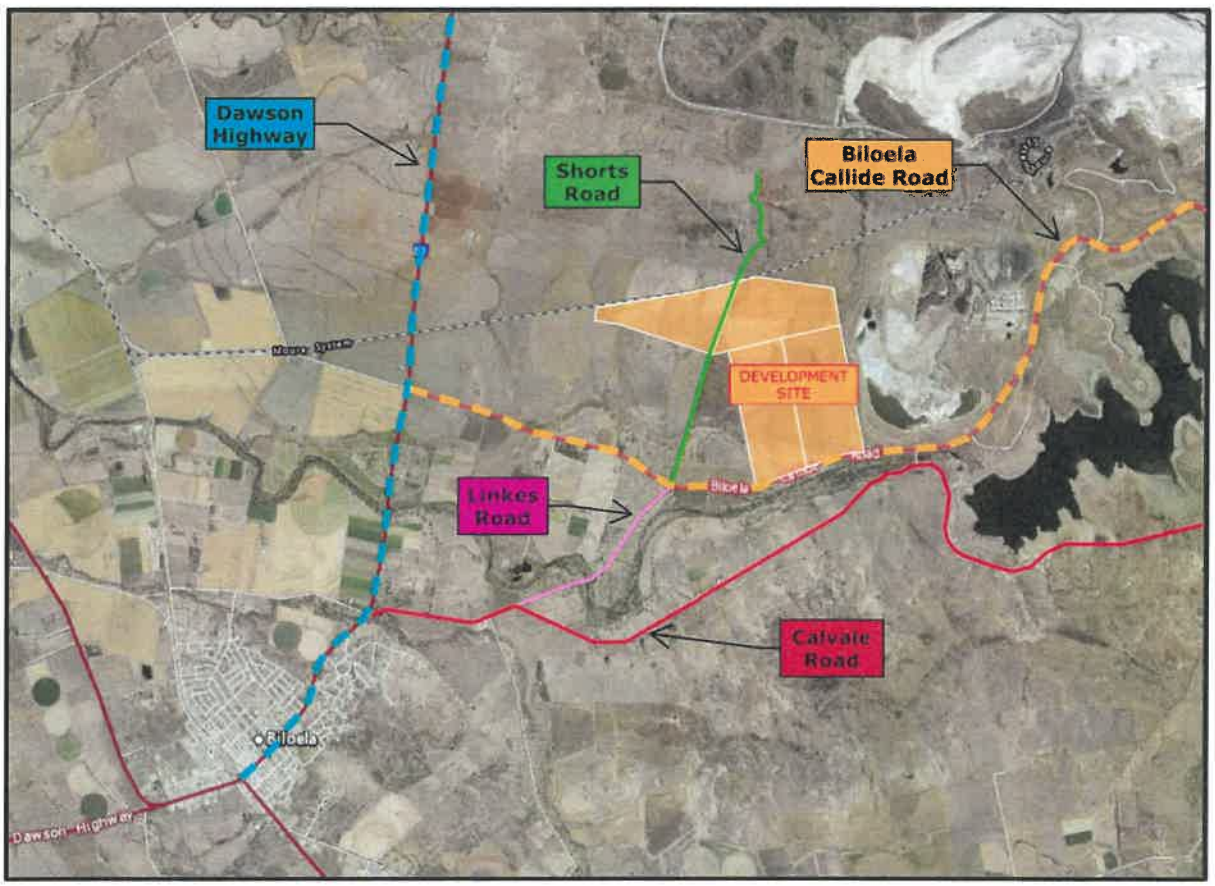

Figure 2-2 Road network surrounding the development

2.3 Background traffic volumes

Background traffic volumes utilised within the analysis were derived from the available TMR Traffic Analysis and Reporting System (TARS) data.

- TMR's traffic analysis and reporting data was used for traffic volumes on SCRNs.
  - Dawson Highway – Road Section 46A – Data collected in 2022 at Site ID 61084, data collected in 2019 for Site ID 60067 and data collected in 2022 for Site ID 160067
  - Biloela Callide Road – Road Section 472 – Data collected in 2022 for Site ID 60126 and data collected in 2022 for Site ID 61286.

The TARS data locations are depicted in **Figure 2-3** below.

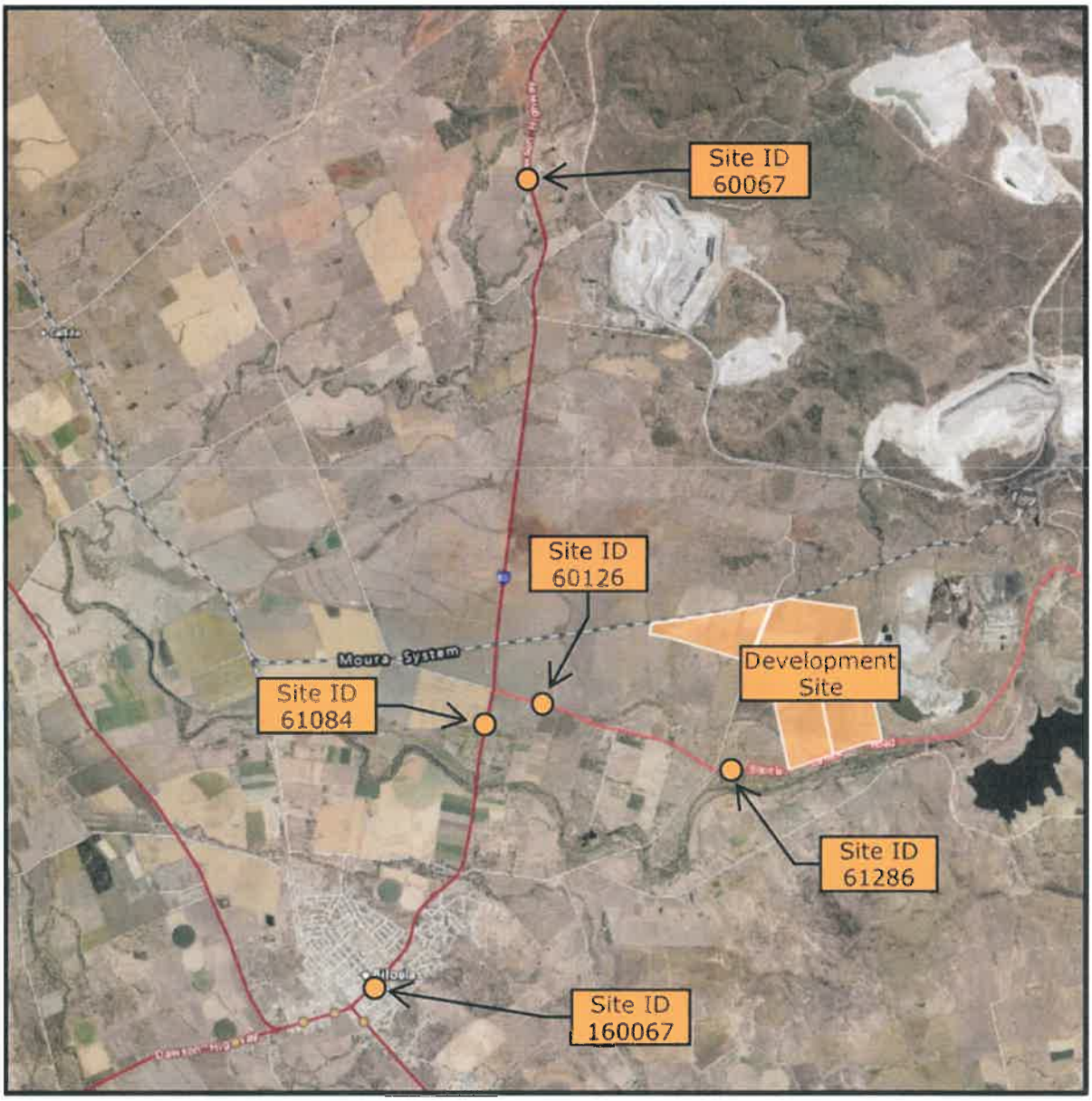

Figure 2-3 TMR TARS data survey locations

2.3.1 Background traffic calculations and assessment

The traffic data provided for Site ID 60126 and 61286 were used to calculate the intersection movements through the Biloela Callide Road / Shorts Road / Linkes Road intersection. Differences between the traffic counts on either side of the intersection were calculated and utilised as the turn movement counts.

Figure 2-4 depicts the calculation for the background traffic directional calculations from the given TARS data.

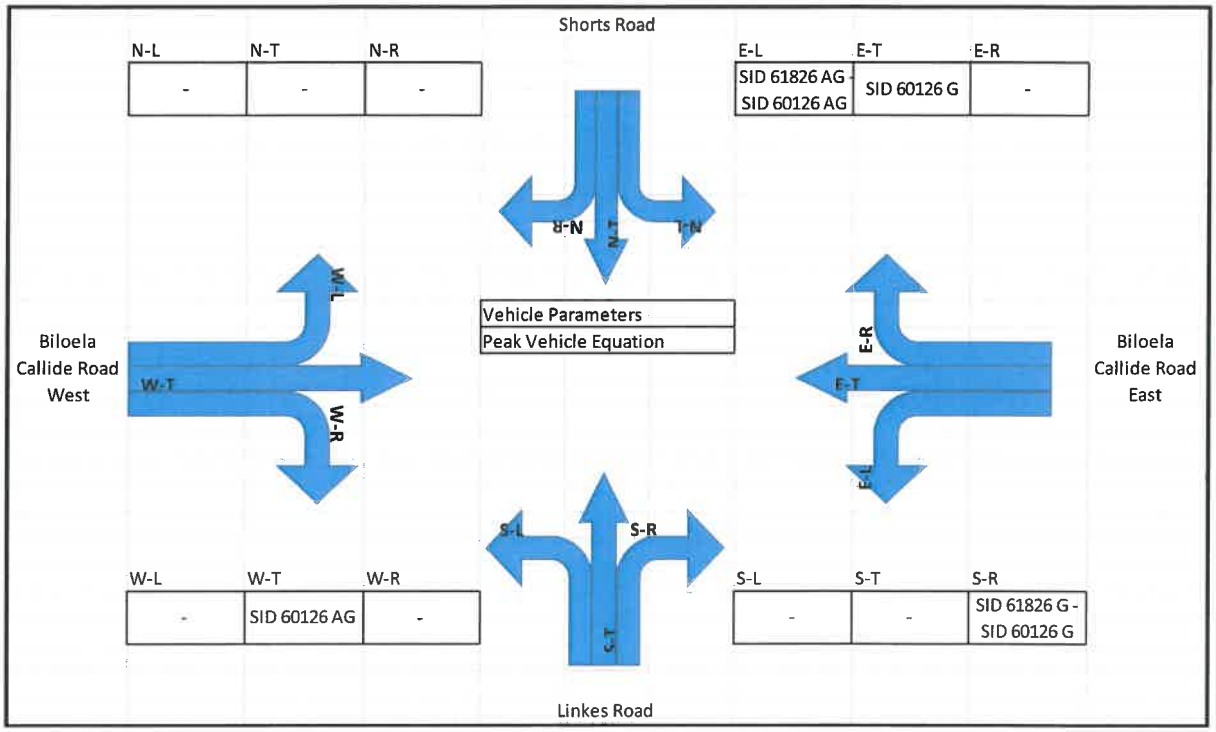

Figure 2-4 Background traffic calculations

Copies of the TARS data relied upon as part of the assessment are contained within **Appendix D**.

There are no other available sources of data for the intersection and approach roads and no survey has been completed. Therefore, NCE believe the available data and the above calculations to be an acceptable interpretation of the traffic movements at the intersection.

The background traffic volumes for each site ID were forecast utilising the 10-year growth percent for each direction (gazettal and against-gazettal) respectively.

The TMR traffic composition data has been utilised to split the peak total traffic values calculated between the TMR traffic classifications, 1A, 1B, 1C and 1D. The heavy vehicle percentages and splits between the different TMR traffic classifications have been averaged for the two (2) sites and each direction and applied to the calculated traffic movements at the intersection. The traffic classification separation is given below.

- Class 1A – Light Vehicles – Average percentage split: 82.5%
- Class 1B – Truck or Bus – Average percentage split: 12.4%
- Class 1C – Articulated Vehicles – Average percentage split: 1.5%
- Class 1D – Road Trains / B-doubles – Average percentage split: 3.6%

2.4 Road safety issues

Crash data was obtained for the area via the Department of Transport and Main Roads. The crash data was requested from TMR in the following locations:

- 1km either side of Biloela Callide Road / Shorts Road intersection along the Biloela Callide Road
- Full length of Shorts Road
- Biloela Callide Road from chainage 5km to 8km
- 2km either side of Dawson Highway / Biloela Callide Road intersection along Dawson Highway and Biloela Road

The data returned contained a total of four (4) accidents from 2020 to 2021, this included three (3) hospitalisations and one (1) fatal crash.

Queensland Globe data has also been obtained for the areas to be impacted by the development including but not limited to the locations listed below:

- Dawson Highway / Biloela Callide Road intersection
- Biloela Callide Road / Shorts Road / Linkes Road intersection
- Biloela Callide Road from chainages 0 km to 7.5km
- Full length of Shorts Road
- Linkes Road from the Biloela Callide Road intersection up to, but not including, the Calvale Road intersection

This data returned a much larger number of crashes with fifteen (15) separate crashes reported from 2004 to 2020, this includes property damage crashes which are not contained in the TMR data set. The Queensland Globe data includes one (1) fatal crash, six (6) crashes resulting in hospitalisations, four (4) crashes requiring medical treatment, two (2) crashes resulting in minor injuries and two (2) crashes resulting in property damage only. The Queensland Globe crash data generally includes all the data provided by TMR aside from some items outside of the assessment area.

The TMR and Queensland Globe data are compiled in **Table 2-1** below which includes the details of the events.

NCE have assessed the crash data and find that the majority of crashes are not a result of improper road safety design and are a result of driver error. The TMR data includes two (2) crashes which are a result of vehicles driving off the carriageway and another which was out of control on the carriageway. The Queensland Globe data is also a majority of crashes resultant from driving off the carriageway or out of control on the carriageway, with a total of seven (7) crashes resulting from this type. NCE have assessed the horizontal curve at the Biloela Callide Road / Shorts Road intersection and found it to be compliant for the 85th percentile road speed. Whilst a deficiency in the geometry of the road does not seem to be the cause of the crashes, the lack of trafficable verge width due to the relatively deep table drains may not provide errant vehicles adequate room to regain control. The depth of the table drains may also account for the severity of the crash types when combined with the high-speed environment. Traffic safety on Shorts Road has been addressed as part of the Capability and Suitability Report provided in **Appendix B**.

Table 2-1 Crash Data from QLD Globe

Location	Date and Time	Occupancy	Nature of Crash
TMR Crash Report Data			
Biloela - Callide Rd /	August 2020, Thursday at 6:00 AM	(1) Hospitalisation	Multi-vehicle, Rear-end, Veh'S Same Direction: Rear End
Biloela - Callide Rd /	April 2020, Sunday at 2:00 PM	(1) Hospitalisation	Single vehicle, Hit object, Off Path-Curve: Off Cway Rt Bend Hit Obj
Calvale Rd /	April 2021, Tuesday at 2:00 PM	(1) Fatal	Single vehicle, Hit object, Off Path-Straight:Right Off Cway Hit Obj
Biloela - Callide Rd /	December 2020, Friday at 11:00 PM	(2) Hospitalisation	Single vehicle, Fall from vehicle, Off Path-Straight:Out Of Control On Cway
QLD Globe Crash Data			
Dawson Highway / Biloela Callide Road	March 2007, Saturday at 2:00 PM	(2) Fatal	Multi-Vehicle, Angle, Veh'S Adjacent Approach: Thru-Thru
Biloela Callide Road	April 2020, Sunday at 2:00 PM	(1) Hospitalisation	Single Vehicle, Hit object, Off Path-Curve: Off Cway Rt Bend Hit Obj
Biloela Callide Road	August 2004, Wednesday at 1:00 PM	(3) Hospitalisation	Single Vehicle, Hit object, Off Path-Straight: Left Off Cway Hit Obj
Biloela Callide Road	August 2020, Thursday at 6:00 AM	(1) Hospitalisation	Multi-Vehicle, Rear-end, Veh'S Same Direction: Rear End
Biloela Callide Road	December 2020, Friday at 11:00 PM	(2) Hospitalisation	Single Vehicle, Fall from vehicle, Off Path-Straight:Out Of Control On Cway
Dawson Highway / Biloela Callide Road	February 2005, Sunday at 6:00 AM	(1) Hospitalisation	Multi-Vehicle, Angle, Veh'S Adjacent Approach: Thru-Thru
Biloela Callide Road / Linkes Road	August 2007, Wednesday at 7:00 AM	(2) Hospitalisation	Multi-Vehicle, Angle, Veh'S Adjacent Approach: Right-Thru
Linkes Road	December 2019, Tuesday at 12:00 PM	(2) Medical treatment	Single Vehicle, Hit object, Veh'S On Path: Temporary Object On C'Way
Biloela Callide Road	September 2005, Friday at 2:00 PM	(1) Medical treatment	Single Vehicle, Hit object, Off Path-Curve: Off Cway Rt Bend Hit Obj
Biloela Callide Road	May 2006, Thursday at 6:00 PM	(1) Medical treatment	Multi-Vehicle, Sideswipe, Veh'S Same Direction: Lane Side Swipe
Shorts Road	November 2006, Monday at 9:00 AM	(1) Medical treatment	Single Vehicle, Hit object, Off Path-Curve: Off Cway Rt Bend Hit Obj
Biloela Callide Road	March 2006, Sunday at 12:00 PM	(3) Minor injury	Multi-Vehicle, Angle, Veh'S Manoeuvring: Leaving Driveway
Dawson Highway / Biloela Callide Road	March 2007, Sunday at 6:00 AM	(2) Minor injury	Multi-Vehicle, Angle, Veh'S Adjacent Approach: Thru-Thru
Linkes Road / Calvale Road	April 2010, Friday at 11:00 AM	(0) Property damage only	Multi-Vehicle, Angle, Veh'S Overtaking: Overtake-Right Turn
Biloela Callide Road / Linkes Road	June 2005, Thursday at 11:00 AM	(0) Property damage only	Multi-Vehicle, Angle, Veh'S Adjacent Approach: Thru-Thru

2.5 Current intersection configuration

Figure 2-5 depicts the current intersection from the Queensland Globe Imagery.

There are currently no turning lanes in place for any turn movements around the intersection. The westbound lane for the Biloela Callide Road includes an approximately 100m widening at the intersection with the hold line of the Linkes Road leg being approximately 4m away. This widening allows vehicles entering Linkes Road from the east to deviate off Biloela Callide Road and therefore reduce the impacts upon the state-controlled roadway. The additional pavement footprint also enables vehicles travelling west along Biloela Callide Road opportunity to avoid vehicles turning into Shorts Road.

Aside from the widening at the intersection there are no other turn treatments provided at the intersection aside from the hold line being located approximately 2m from the Biloela Callide Road eastbound lane. The required shoulder widening conditions to classify the turns as basic left (BAL) or base right turn (BAR) treatments are not evident on-site.

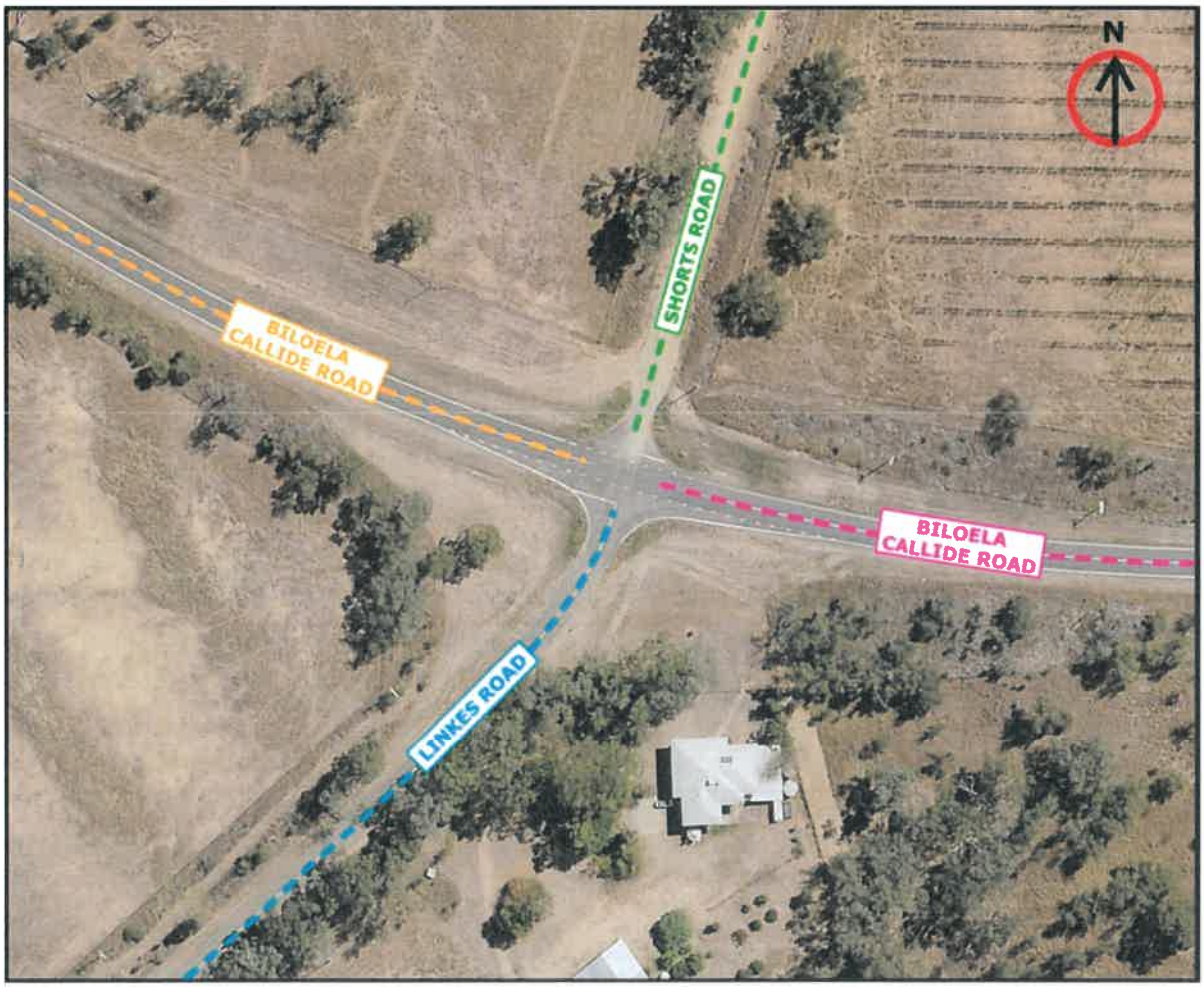

Figure 2-5 Existing intersection configuration

2.6 Railway level crossings

The Moura System railway line runs adjacent to the Biloela Callide Road to the north. This line crosses two (2) roads within the assessment area. This includes a stop-sign controlled railway level crossing over Shorts Road and an active trackside road sign with lights over the Dawson highway. The location of the two railway level crossings is shown in **Figure 2-6** below. The imagery inset into the figure showing the Dawson Highway

and Shorts Road railway crossings were obtained from Google Street View and the Shorts Road visual inspection recording respectively.

Figure 2-6 Railway level crossing within the assessed area

The railway level crossing on Shorts Road is located approximately 3km from the Biloela Callide Road intersection. Adequate vertical and horizontal site distance is provided and prior warning in the form of signage is also in place. As Shorts Road is unsealed no line marking is provided. Access across the Shorts Road level crossing is not anticipated as part of either the construction phase and or operational phase of the development. Therefore, no further assessment of this crossing is warranted or provided.

The railway level crossing on Dawson highway is located approximately 750m north of the Biloela Callide Road intersection. Adequate vertical and horizontal site distance is provided and prior warning in the form of signage is also in place. Hold lines are in place and set behind the crossing signage and lights.

2.7 Sight distances

An assessment of available sight distances for traffic using the Biloela Callide Road / Shorts Road intersection is undertaken to ensure the existing intersection configuration is adequate for the types of vehicles using it.

Calculations have been completed using the coefficient of deceleration for a truck as a worst-case situation. As-constructed plans supplied by TMR have been provided for the Biloela Callide Road, these plans were used to calculate the approach grades for the intersection. A reaction time of two (2) seconds has been assumed as the average case for the environment.

It must be noted that due to the relatively flat landscape of the area it is assumed that longitudinal sight triangles will not be obstructed by undulations or dips in the road and thus have not been specifically assessed.

2.7.1 Intersection sight distances

The approach sight distance for vehicles required to stop at the intersection hold lines on both Shorts Road and Linkes Road were assessed. No data was available to calculate the approach grades, thus a grade of 0% has been assumed. An approach speed of 80km/hr has been adopted as the 85th percentile speed for the Shorts Road approach as there is no sign posted speed and it is an unsealed road. An approach speed of 110km/hr has been adopted as the 85th percentile speed for the Linkes Road approach as the signposted speed is 100km/hr. Sight triangles for approach sight distances can be seen in **Figure 2-7**.

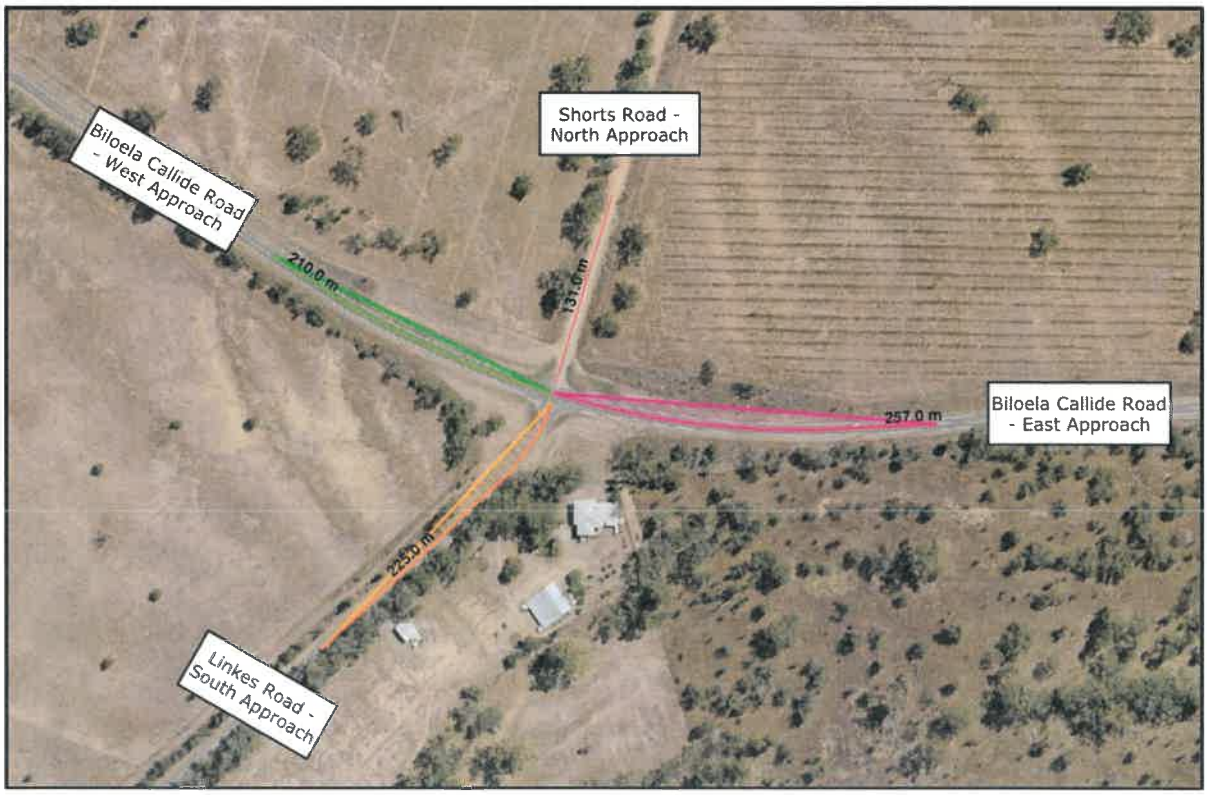

Figure 2-7 Biloela Callide Road / Shorts Road / Linkes Road intersection – minimum stopping sight distances

Vehicles approaching the intersection from Shorts Road (north approach) have more than adequate approach sight distance to see the intersection and signage and come to a complete stop. Although the road is unsealed the approach is relatively straight for approximately the first 1.5km meaning the approach will not generate an unsafe interaction.

Vehicle approaching the intersection from Linkes Road (south approach) do not have adequate approach sight distances due to a bend in the road right before the intersection. Whilst the minimum approach sight distance is not available the road does provide adequate signage (approaching intersection and approaching stop sign) to warn the driver of the upcoming intersection and to begin to slow down. This intersection has been operating in the current configuration without significant safety issues being raised, therefore it is suggested the approach is safe given the existing signage.

Vehicles approaching the intersection along Biloela Callide Road from the west have more than adequate stopping sight distance to see any obstructions or vehicles at the intersection and come to a complete stop. However, the bend in the road means there is not much additional sight distance past the minimum.

Vehicles approaching the intersection along Biloela Callide Road from the east have adequate stopping sight distance to see any obstructions or vehicles at the intersection and come to a complete stop. Although the bend in the road means there is no additional sight distance past the minimum, the conservative approach adopted means the road is generally acceptable.

The safe intersection sight distances for the Biloela Callide / Shorts Road / Linkes Road intersection have been calculated based on the previously mentioned variables. Sight triangles for traffic from Linkes Road are shown in **Figure 2-8** and sight triangles for traffic from shorts road are shown in **Figure 2-9**.

Figure 2-8 Biloela Callide Road / Shorts Road / Linkes Road intersection – Linkes Road approach safe intersection sight distances

Figure 2-9 Biloela Callide Road / Shorts Road / Linkes Road intersection – Shorts Road approach safe intersection sight distances

The current available safe intersection sight distances to the Linkes Road approach are generally not acceptable. The west and east approaches have sufficient clearance to be able to see a vehicle stopped at hold line and through the intersection.

The current available safe intersection sight distances to the Shorts Road approach are generally not acceptable. The sight triangles seen in the figure both extend outside of the road verge and into the adjacent properties. Google Street View shows a number of trees and taller vegetation through the area which effectively blocks sight line to the intersection. An approaching intersection sign is provided for each approach around the corner which does give prior warning to the intersection.

All sight distance values have been calculated in accordance with the Austroads Guide to Road Design Part 3 & 4A – Geometric Road Design and Unsignalised and Signalised Intersections 2021.

2.7.2 Access Sight Distances

For any assessments relating to the access off of Shorts Road refer to the Capability and Suitability Report provided in **Appendix B**

2.8 Intersection and network performance

An analysis of the existing intersection for the years 2024 and 2034 was undertaken using SIDRA intersection 9.1 (Version 9.1.1.200). The intersection was analysed in the AM and PM peaks and the results of the anaysis are given in **Table 2-2**.

The intersection performance is assessed via the level of service (LOS) which represents the average delay for a vehicle making a movement. The Department of Transport and Main Roads (DTMR) Guide to Traffic Impact Assessment (GTIA) indicates that that a limit of 42 seconds of average delay should not be exceeded by any movement at a priority-controlled intersection. As indicated by the SIDRA analysis the existing intersection configuration will be adequate in all cases in the end of construction 2024 and the end of the operational design horizon 2034.

NCE have utilised a user defined vehicle class to accurately depict the class 1D vehicles. The data input into the user defined class is equivalent to a triple road train, this creates a conservative situation for the average class 1D vehicle. Feedback on the input data has been provided by the SIDRA support team which is provided in **Appendix F** with the rest of the SIDRA results reports.

Table 2-2 Biloela Callide Road / Shorts Road / Linkes Road intersection analysis results – background traffic

Critical Movement comparisons						
Time Status	Approach Leg	Movement Ref	Deg. Saturation	LOS	Delay (sec)	Back of Queue Dist (m)
Background 2024 AM	Biloeala Callide Road East	T1	0.086	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road East	T1	0.115	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road East	L2	0.086	A	8.3	0.1
Background 2034 AM	Biloeala Callide Road East	L2	0.115	A	8.5	0.1
Background 2024 AM	Biloeala Callide Road West	T1	0.021	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road West	T1	0.035	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road West	L2	0.021	A	7.8	0.1
Background 2034 AM	Biloeala Callide Road West	L2	0.035	A	7.8	0.1
Background 2024 AM	Shorts Road	T1	0.003	A	9.9	0.1
Background 2034 AM	Shorts Road	T1	0.003	B	10.4	0.1
Background 2024 AM	Shorts Road	R2	0.003	A	9.6	0.1
Background 2034 AM	Shorts Road	R2	0.003	B	10.1	0.1
Background 2024 AM	Linkes Road	T1	0.161	A	10.0	5.0
Background 2034 AM	Linkes Road	T1	0.191	B	10.5	5.9
Background 2024 AM	Linkes Road	R2	0.161	B	12.0	5.0
Background 2034 AM	Linkes Road	R2	0.191	B	13.1	5.9
Background 2024 PM	Biloeala Callide Road East	T1	0.094	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road East	T1	0.104	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road East	L2	0.094	A	8.4	0.1
Background 2034 PM	Biloeala Callide Road East	L2	0.104	A	8.4	0.1
Background 2024 PM	Biloeala Callide Road West	T1	0.039	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road West	T1	0.059	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road West	L2	0.039	A	7.8	0.1
Background 2034 PM	Biloeala Callide Road West	L2	0.059	A	7.8	0.1
Background 2024 PM	Shorts Road	T1	0.003	B	10.1	0.1
Background 2034 PM	Shorts Road	T1	0.003	B	10.4	0.1
Background 2024 PM	Shorts Road	R2	0.003	A	9.3	0.1
Background 2034 PM	Shorts Road	R2	0.003	A	9.6	0.1
Background 2024 PM	Linkes Road	T1	0.038	A	9.8	1.0
Background 2034 PM	Linkes Road	T1	0.041	B	10.1	1.1
Background 2024 PM	Linkes Road	R2	0.038	B	11.5	1.0
Background 2034 PM	Linkes Road	R2	0.041	B	12.0	1.1

3.0 DEVELOPMENT TRAFFIC

3.1 Traffic generation

In accordance with the Department of Transport and Main Roads Guide to Traffic Impact Assessment December 2018, a first principles assessment of the development generated traffic has been used due to a lack of available data relating to the generation of traffic from this type of development.

Edify Energy has provided NCE with information pertaining to the generation of traffic for the construction and operation of previous solar farms. NCE have utilised calculations from past solar farms and the information provided by Edify Energy to calculate the number of containers of construction materials to be generated per MW of solar farm. NCE considered this container generation method and using the following theory developed a trip generation method.

(1) Solar Panel Modules – calculated to be 0.18 MW per container.

(2) Inverter Stations – calculated to be 2.00 MW per container.

(3) Fixing Systems – calculated to be 0.15 MW per container.

(4) Switchgear – one container per station.

(5) Power Transformer – one container per station.

(6) Piles – calculated to be 0.13 MW per container.

(7) Tube – calculated to be 0.13 MW per container.

(8) Tracker – calculated to be 0.13 MW per container

(9) Balance of System – calculated to be 0.75 MW per container.

(10) Earthworks / Road Materials – calculated to be 0.13 MW per container.

(11) 1 container per semi-trailer (Class 9 Heavy Vehicle) (Average payload of all containers used).

(12) LV movements for construction workers using buses, personal vehicles and additional daily cars based on numbers provided by Edify Energy.

   a. Daily On-Site Labour – calculated to be 0.08 vehicles per day per MW.

   b. Daily Mini Bus Activity – calculated to be 0.03 vehicles per day per MW.

   c. Additional Daily Cars – calculated to be 0.03 vehicles per day per MW.

(13) Numbers of workers and vehicles generated by the operation and maintenance of the solar power station were provided by Edify Energy.

   a. Management / Office – 3 total vehicles per day one way.

   b. Electrical maintenance crew – 1 total vehicle per day one way.

   c. Cleaning (Panel) crew – Annually 20 vehicles per event to be negligible over the duration of the operation.

(14)Deliveries required by the operational activities were also outlined by Edify Energy.

a. Water supply for drinking deliveries – 1 heavy vehicle monthly.

b. Water supply for cleaning deliveries – Annual event 2 heavy vehicles per event.

A 10-year design horizon starting in 2024 for the life of the development has been used for the purpose of the traffic impact assessment.

Table 3-1 below outlines the construction and operation particulars.

Table 3-1 Development information

Development - Particulars		
Construction Commencement	2023	year
Construction Duration	52	weeks
Construction Duration (Assumed 6 days/week)	312	days
Operations Commencement	2024	year
Operational Design Life	30	years
Operational Design Life	1560	weeks
Operational Design Life (Assumed 5 days/week)	7800	days

3.1.1 Traffic generation during construction

Using the calculations from above the total heavy vehicle movements generated by the development site is 9,213 HV. It is expected that the site deliveries are to be spaced out over the entire duration of the construction, therefore, these have been averaged over the duration of the construction which comes out to 30 HV/day. The construction delivery traffic calculated is outlined in **Table 3-2**. The construction site labour has been calculated at 26 LV/day as per the calculations outlined above. The construction labour traffic calculated is outlined in **Table 3-3**.

Table 3-2 Construction delivery traffic

Solar Array System				
Expected MWp	**200**			
	MWp per Container	Movements (One way only)	Weight per Container (kg)	Total Weight Moved
Modules	0.18	1,111	15,500	8,610,250
Inverter Stations	2.00	100	15,500	775,000
Fixing System	0.15	1,333	20,500	13,663,250
Switchgear	200.00	1	50,000	25,000
Power Transformer	200.00	1	50,000	25,000
Pile	0.13	1,600	20,500	16,400,000
Tube	0.13	1,600	19,500	15,600,000
Tracker	0.13	1,600	13,000	10,400,000
Balance of System	0.75	267	15,500	2,069,250
Earthworks / Road Materials	0.13	1,600	15,500	12,400,000
Total Heavy Vehicle Movements		9,213	Average Payload (kg)	8,875
Average Daily Heavy Vehicle Movements		30	HV/day	

Table 3-3 Construction labour traffic

Construction Labour (Vehicle trips per day one way)		
Daily on Site Labour	0.08	15
Daily Mini Bus Activity	0.03	6
Additional Daily Cars	0.03	5
Total Light Vehicle Movements (per day)		26

Construction deliveries are expected to be generated from Gladstone or that direction whereas construction labour is expected to be local and arrive from the direction of Biloela. The relative locations and expected routes are depicted in **Figure 3-1** which is shown in the following sections.

3.1.2 Traffic generation during operation

The opeartion of the solar power station is expected to generate significantly less traffic with the average daily light traffic generated being 7 LV/day. The operational traffic light vehicles calculations are depicted in **Table 3-4**. The heavy vehicle movements which are solely generated from the delivery of drinking water and cleaning water creates an average of 1 HV/day. The heavy vehicle generation calculations are outlined in **Table 3-5**.

Table 3-4 Operational traffic generation – light vehicles

Operational traffic Light Vehicles (per day one way)	
Management / Office	6.000
Electrical maintenance crew	1.000
Cleaning (Panel) crew (Annually - 20 veh per event)	0.077
Total Light Vehicle Movements (per day one way)	7.077

Table 3-5 Operational traffic generation – heavy vehicles

Operational traffic Heavy Vehicles (per day one way)	
Water Supply drinking (1 x monthly)	0.046
Water Supply Cleaning (Annually - 2 HV's per event)	0.008
Total Heavy Vehicle Movements (per day one way)	0.054

Edify Energy have indentified that once in full operation, the normal operating hours of the solar power station will be during daylight hours between the hours of sunrise approximately 7am to 6pm. For the purpose of this TIA the generated traffic will be applied to the identified AM and PM peaks.

Figure 3-1 Generated traffic routes and locations

3.2 Traffic composition

Generated traffic contribution is split between light vehicles including buses and cars and heavy vehicles which will generally be semi-trailers. The number of each vehicle type is depicted in the tables outlined in **Section 3.1**.

All light vehicles will be classed as under the 0A / 1A TMR traffic data classifications.

All development heavy vehicles are to be Class 9 – Six Axle Articulated (Semi Trailer) which is the equivalent of a Class 2I in the TMR traffic data classifications. The Class 2I is specified under a Class 1C. Whilst some heavy vehicles such as water deliveries could be classed as a lower vehicle class they are to be adopted as semi-trailers to take a conservative approach.

3.3 Trip distribution (In/Out) splits

Based on a worse-case assumption that all construction deliveries can arrive during the peak hour and that there will be a high turnover of delivery vehicles an in / out split of 100% / 100%. This is a very conservative assumption which will also account for the possibilities of deliveries not being distributed exactly over the span of the development.

The light vehicle in / out split is to be 100% / 0% for the AM peak hour and 0% / 100% for the PM peak hour. This will simulate the arrival of workers to the site in the morning and the departure of workers in the afternoon.

3.4 Development traffic volumes on the network

The full set of traffic calculations and intersection warrants are given in **Appendix E**. The following sheets show the split of the development traffic through the intersection:

- DEV 2024 AM – this shows the construction traffic in the AM peak added to the background traffic. This traffic is applied at the end of the construction period as a worst-case scenario of the construction traffic.
- DEV 2024 PM – this shows the construction traffic in the PM peak added to the background traffic. This traffic is applied at the end of the construction period as a worst-case scenario of the construction traffic.
- DEV 2034 AM – this shows the operation traffic in the AM peak added to the background traffic. This traffic is applied at the end of the operation design horizon as a worst-case scenario of the operational traffic.
- DEV 2034 PM – this shows the operation traffic in the PM peak added to the background traffic. This traffic is applied at the end of the operation design horizon as a worst-case scenario of the operational traffic.

Heavy vehicles during both the construction and operation phases will approach the Biloela Callide Road / Shorts Road / Linkes Road from the west and turn left onto Shorts Road to enter the development site. Heavy vehicles during both the construction and operation phases will turn right out of Shorts Road to drive towards Gladstone via the Biloela Callide Road and Dawson Highway as depicted in **Figure 3-1**.

Light vehicles are expected to arrive from and depart to Biloela via Calvale and Linkes Roads as depicted in **Figure 3-1**. This traffic will drive straight through the Biloela Callide Road / Shorts Road / Linkes Road from the Linkes Road approach and depart in the opposite direction.

3.5 Heavy vehicle payloads

The average payload for heavy vehicles is given in **Table 3-2**.

4.0 IMPACT ASSESSMENT AND MITIGATION

For comparison to existing conditions refer to **Section 2.0**.

4.1 With development scenario traffic volumes

The full traffic generation spreadsheet is contained in **Appendix E**.

4.1.1 Construction and establishment of solar power station

Figure 4-1 shows the 2024 AM traffic peak at the Biloela Callide Road / Shorts Road / Linkes Road intersection, inclusive of the construction traffic for the development. **Figure 4-2** shows the 2024 PM traffic peak at the Biloela Callide Road / Shorts Road / Linkes Road intersection, inclusive of the construction traffic for the development.

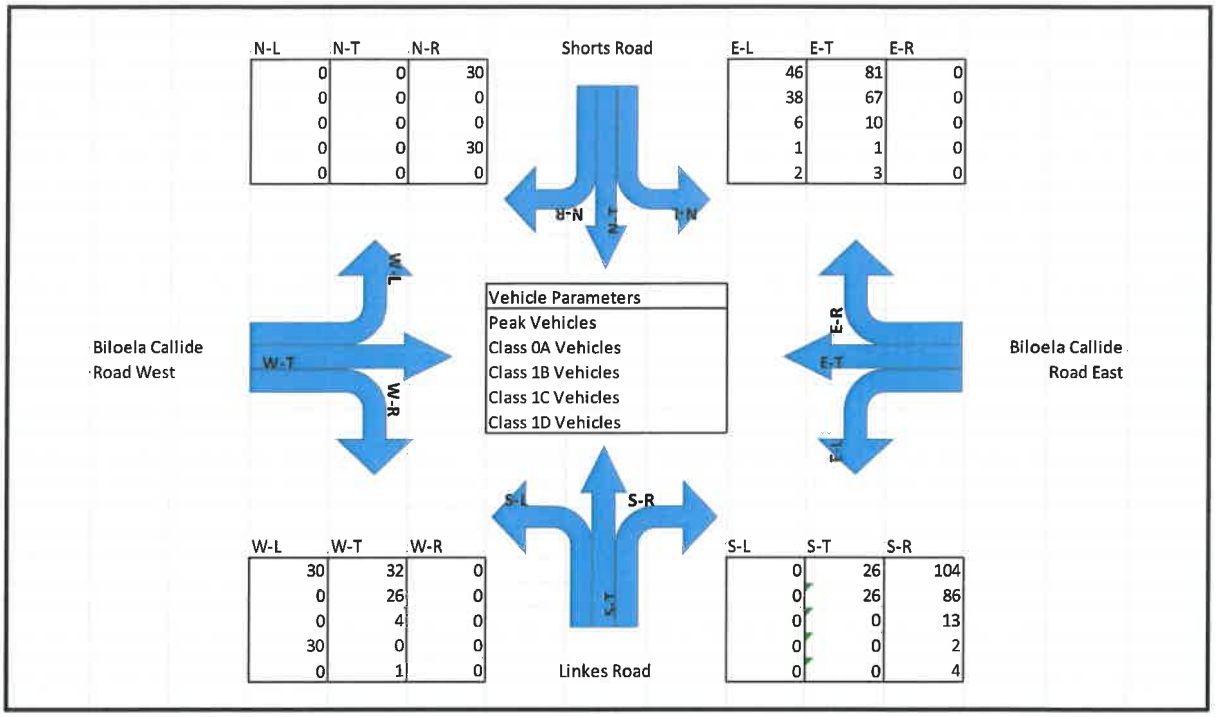

Figure 4-1 2024 AM traffic peak – construction traffic

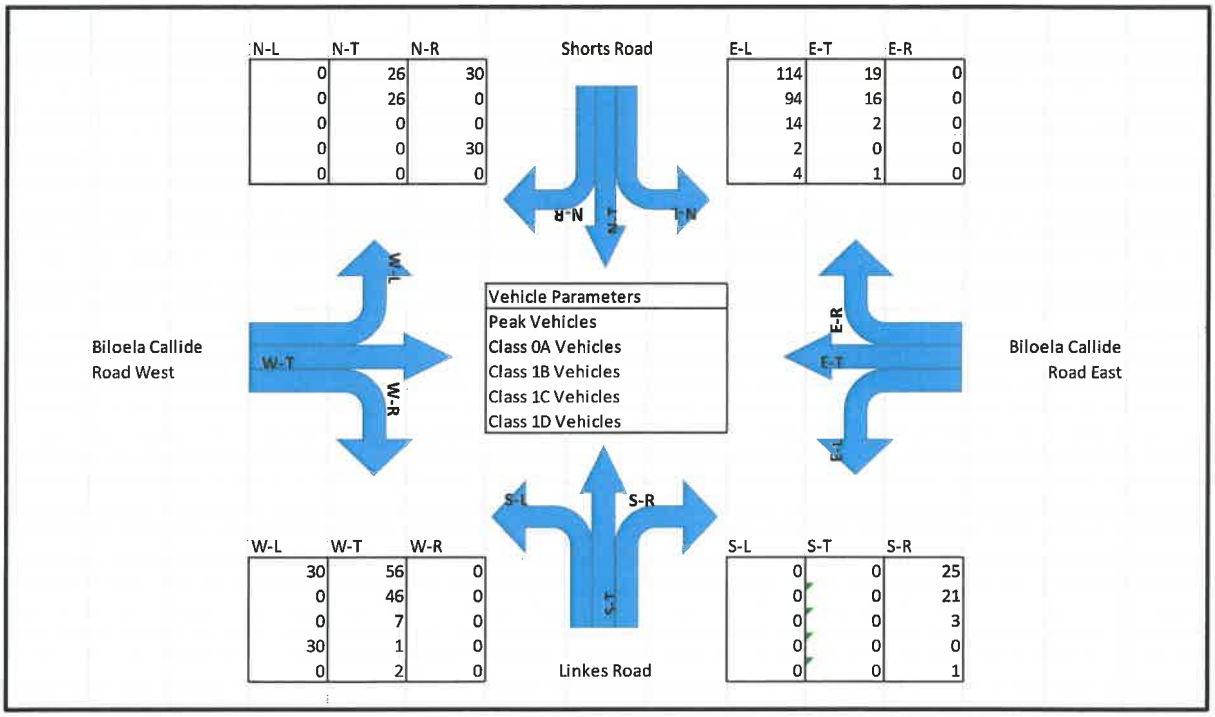

Figure 4-2 2024 PM traffic peak – construction traffic

4.1.2 Operation of the solar power station

Figure 4-3 shows the 2034 AM traffic peak at the Biloela Callide Road / Shorts Road / Linkes Road intersection, inclusive of the operation traffic for the development. **Figure 4-4** shows the 2034 PM traffic peak at the Biloela Callide Road / Shorts Road / Linkes Road intersection, inclusive of the operation traffic for the development.

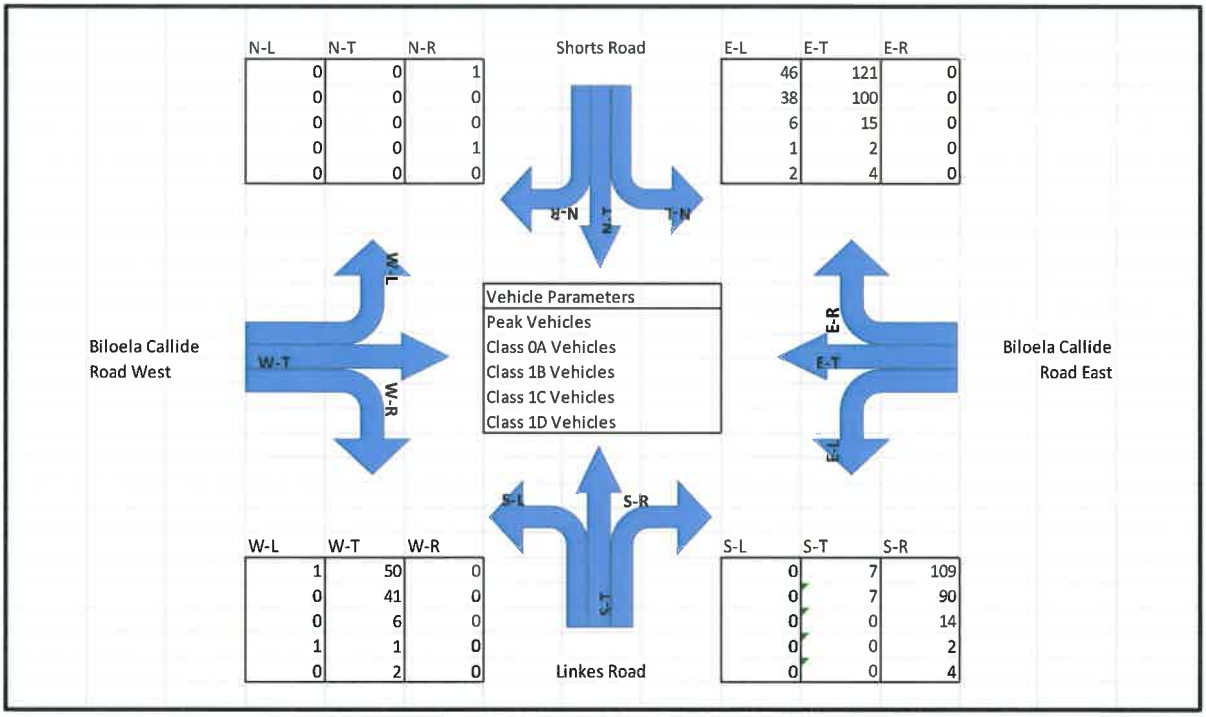

Figure 4-3 2034 AM traffic peak – operational traffic

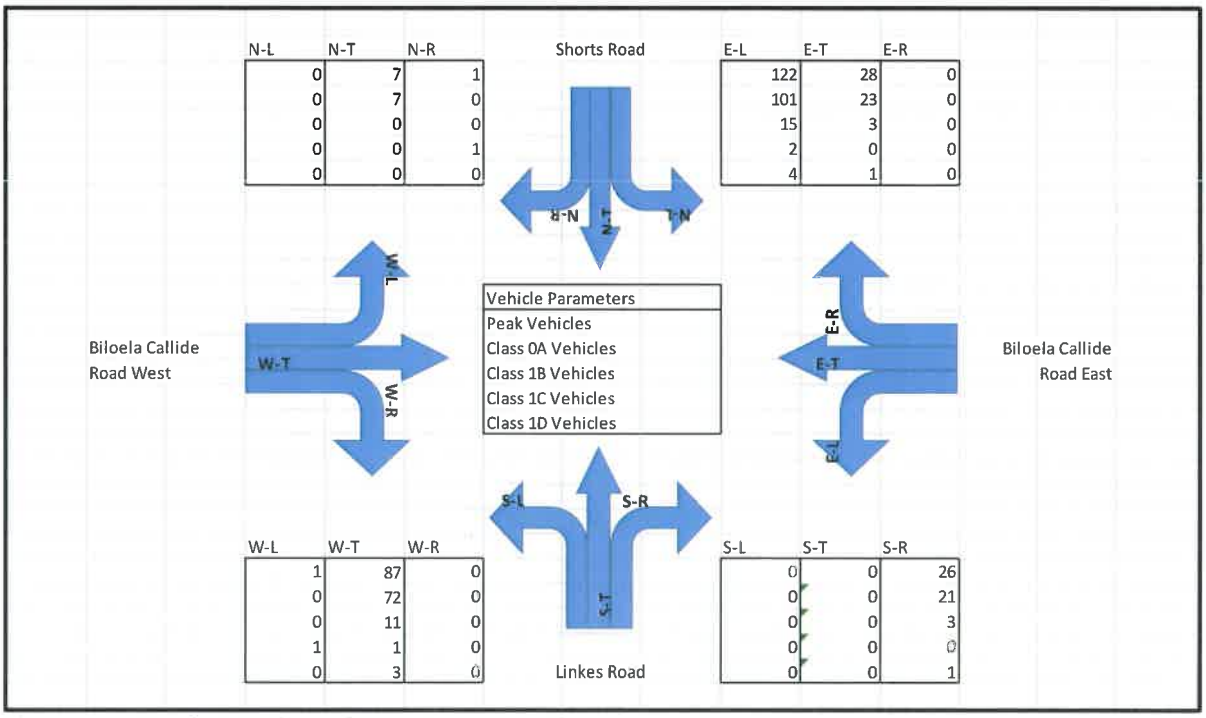

Figure 4-4 2034 PM traffic peak – operational traffic

4.2 Intersection warrant assessment

The Biloela Callide Road / Shorts Road intersection has been assessed using the intersection warrant method outlined by the Austroads Guide to Traffic Management Part 6. The Shorts Road leg has been assessed only as the development will not generate additional traffic in any other turn scenarios. The intersection has been assessed for the development generated traffic calculated for the end of construction and end of the operational design horizon. NCE have assessed Shorts Road and consider that the peak hour

locally generated traffic will be offset to the development peak hours, therefore no background traffic along Shorts Road has been added.

The intersection turn warrants for the worst case PM peak during the construction of the development in 2024 is depicted in **Figure 4-5** below.

As can be seen the low volume of traffic utilising the Biloela Callide Road results in the left turn only requiring a basic left turn treatment. The current intersection does not provide a basic left turn treatment as there is no applicable widening and taper at the intersection left turn. As the development does not generate any right turns onto Shorts Road from Biloela Callide Road it is suggested that no turn treatment is required as the number of vehicles turning right into Shorts Road will be negligible and will not have an impact on the operation and safety of the intersection.

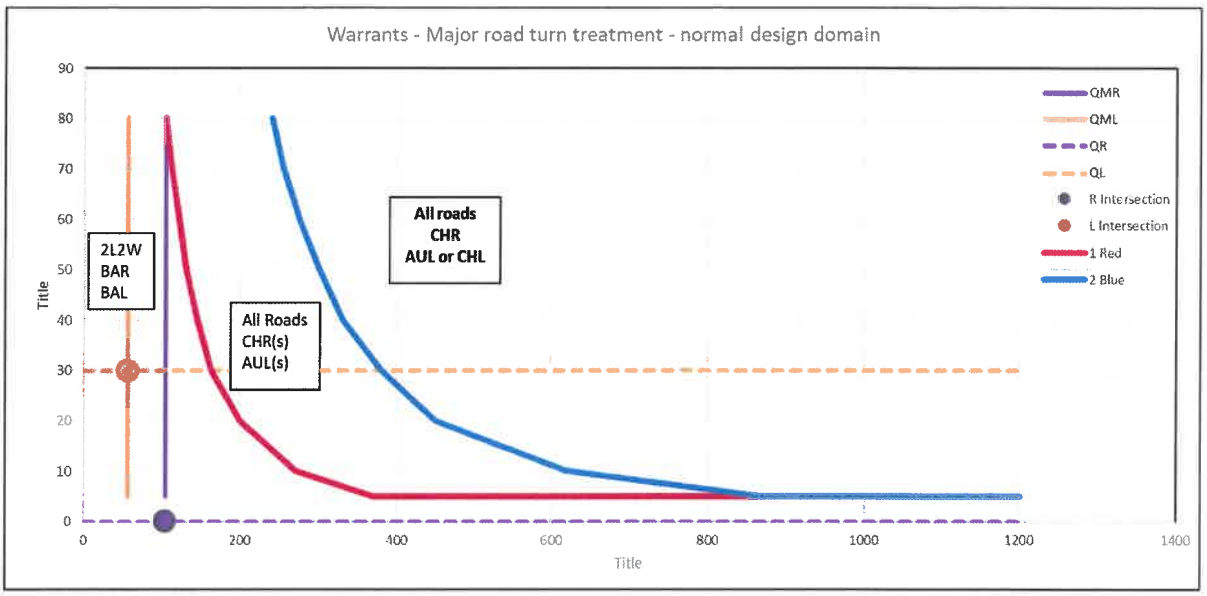

Figure 4-5 Intersection warrants for construction traffic – 2024 PM Peak

The intersection turn warrants for the worst case PM peak at the end of the operational design horizon in 2034 is depicted in **Figure 4-6** below.

As can be seen despite the background traffic increasing the turn movements from Biloela Callide Road have decreased due to the commencement of operation. This results in a betterment to the intersection conditions from the construction phase. Therefore, it is suggested that no further mitigation is required as the basic left turn treatment will cater for the decreased number of turning vehicles. The full intersection warrant assessment spreadsheet is also contained within the spreadsheet in **Appendix E**.

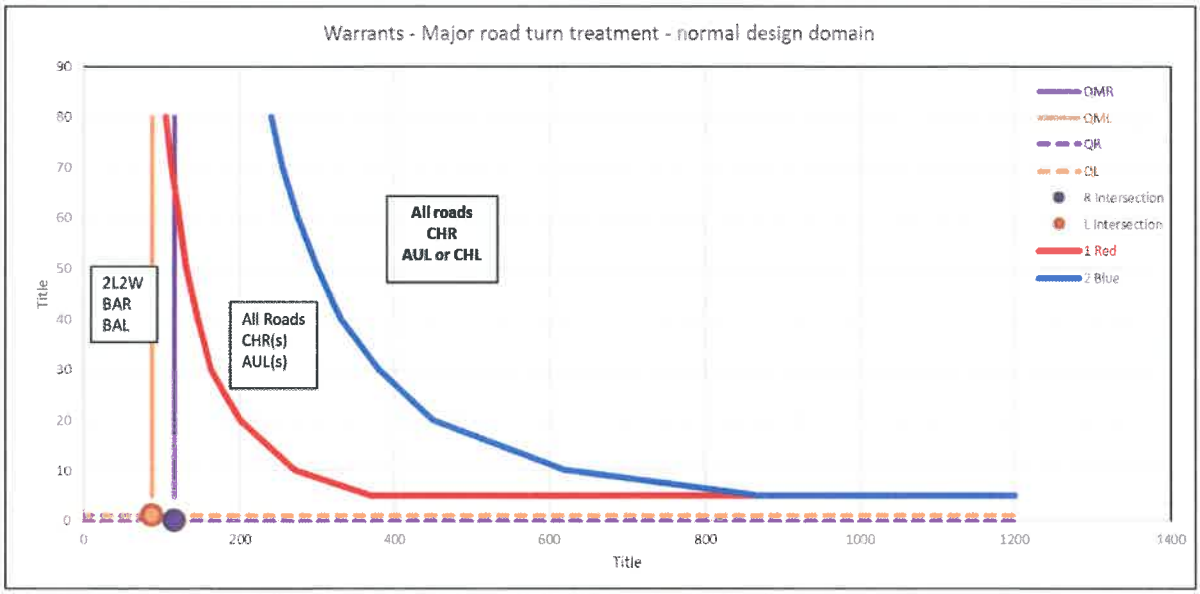

Figure 4-6 Intersection warrants for operational traffic – 2034 PM Peak

4.3 SIDRA analysis

An analysis of the existing intersection with development traffic for the expected end of construction and expected end of operation design horizon was undertaken using SIDRA Intersection 9.1 (Version 9.1.1.200). The intersection was analysed in the AM and PM peaks for each traffic generation scenario. The results of the SIDRA modelling are shown in **Figure 4-1** and **Figure 4-2** for the AM and PM peak assessments respectively.

The intersection performance is assessed via the level of service (LOS) which represents the average delay for a vehicle making a movement. The GTIA indicates that a limit of 42 seconds of average delay should not be exceeded by any movement at a priority-controlled intersection. As indicated by the SIDRA analysis the existing intersection configuration will be adequate in all cases including the expected finish year of construction (2024) and end of operational design horizon (2034). The worst-case LOS is a D for right turn movements out of Shorts Road in the AM peak during the construction and operation, this worsens the movement from an LOS of A and B respectively when compared to the background traffic only. Therefore, NCE consider no additional mitigation or upgrades to the intersection are required by the SIDRA analysis.

Table 4-1 Biloela Callide Road / Shorts Road / Linkes Road intersection analysis results – AM peak development generated traffic

Critical Movement comparisons						
Time Status	Approach Leg	Movement Ref	Deg. Saturation	LOS	Delay (sec)	Back of Queue Dist (m)
Background 2024 AM	Biloeala Callide Road East	T1	0.086	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road East	T1	0.115	A	0.0	0.1
Construction 2024 AM	Biloeala Callide Road East	T1	0.086	A	0.0	0.1
Operation 2034 AM	Biloeala Callide Road East	T1	0.115	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road East	L2	0.086	A	8.3	0.1
Background 2034 AM	Biloeala Callide Road East	L2	0.115	A	8.5	0.1
Construction 2024 AM	Biloeala Callide Road East	L2	0.086	A	8.3	0.1
Operation 2034 AM	Biloeala Callide Road East	L2	0.115	A	8.5	0.1
Background 2024 AM	Biloeala Callide Road West	T1	0.021	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road West	T1	0.035	A	0.0	0.1
Construction 2024 AM	Biloeala Callide Road West	T1	0.064	A	0.0	0.1
Operation 2034 AM	Biloeala Callide Road West	T1	0.036	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road West	L2	0.021	A	7.8	0.1
Background 2034 AM	Biloeala Callide Road West	L2	0.035	A	7.8	0.1
Construction 2024 AM	Biloeala Callide Road West	L2	0.064	B	11.1	0.1
Operation 2034 AM	Biloeala Callide Road West	L2	0.036	B	11.1	0.1
Background 2024 AM	Shorts Road	T1	0.003	A	9.9	0.1
Background 2034 AM	Shorts Road	T1	0.003	B	10.4	0.1
Construction 2024 AM	Shorts Road	T1	0.143	B	10.3	11.9
Operation 2034 AM	Shorts Road	T1	0.007	B	10.4	0.3
Background 2024 AM	Shorts Road	R2	0.003	A	9.6	0.1
Background 2034 AM	Shorts Road	R2	0.003	B	10.1	0.1
Construction 2024 AM	Shorts Road	R2	0.143	D	26.0	11.9
Operation 2034 AM	Shorts Road	R2	0.007	D	25.9	0.3
Background 2024 AM	Linkes Road	T1	0.161	A	10.0	5.0
Background 2034 AM	Linkes Road	T1	0.191	B	10.5	5.9
Construction 2024 AM	Linkes Road	T1	0.189	B	10.5	5.9
Operation 2034 AM	Linkes Road	T1	0.198	B	10.5	6.1
Background 2024 AM	Linkes Road	R2	0.161	B	12.0	5.0
Background 2034 AM	Linkes Road	R2	0.191	B	13.1	5.9
Construction 2024 AM	Linkes Road	R2	0.189	B	12.1	5.9
Operation 2034 AM	Linkes Road	R2	0.198	B	13.1	6.1

Table 4-2 Biloela Callide Road / Shorts Road / Linkes Road intersection analysis results – AM peak development generated traffic

Critical Movement comparisons						
Time Status	Approach Leg	Movement Ref	Deg. Saturation	LOS	Delay (sec)	Back of Queue Dist (m)
Background 2024 PM	Biloeala Callide Road East	T1	0.094	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road East	T1	0.104	A	0.0	0.1
Construction 2024 PM	Biloeala Callide Road East	T1	0.094	A	0.0	0.1
Operation 2034 PM	Biloeala Callide Road East	T1	0.104	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road East	L2	0.094	A	8.4	0.1
Background 2034 PM	Biloeala Callide Road East	L2	0.104	A	8.4	0.1
Construction 2024 PM	Biloeala Callide Road East	L2	0.094	A	8.4	0.1
Operation 2034 PM	Biloeala Callide Road East	L2	0.104	A	8.4	0.1
Background 2024 PM	Biloeala Callide Road West	T1	0.039	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road West	T1	0.059	A	0.0	0.1
Construction 2024 PM	Biloeala Callide Road West	T1	0.082	A	0.0	0.1
Operation 2034 PM	Biloeala Callide Road West	T1	0.060	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road West	L2	0.039	A	7.8	0.1
Background 2034 PM	Biloeala Callide Road West	L2	0.059	A	7.8	0.1
Construction 2024 PM	Biloeala Callide Road West	L2	0.082	B	11.1	0.1
Operation 2034 PM	Biloeala Callide Road West	L2	0.060	B	11.1	0.1
Background 2024 PM	Shorts Road	T1	0.003	B	10.1	0.1
Background 2034 PM	Shorts Road	T1	0.003	B	10.4	0.1
Construction 2024 PM	Shorts Road	T1	0.146	B	10.6	8.7
Operation 2034 PM	Shorts Road	T1	0.013	B	10.5	0.4
Background 2024 PM	Shorts Road	R2	0.003	A	9.3	0.1
Background 2034 PM	Shorts Road	R2	0.003	A	9.6	0.1
Construction 2024 PM	Shorts Road	R2	0.146	C	22.1	8.7
Operation 2034 PM	Shorts Road	R2	0.013	C	21.6	0.4
Background 2024 PM	Linkes Road	T1	0.038	A	9.8	1.0
Background 2034 PM	Linkes Road	T1	0.041	B	10.1	1.1
Construction 2024 PM	Linkes Road	T1	0.039	B	10.3	1.1
Operation 2034 PM	Linkes Road	T1	0.041	B	10.1	1.1
Background 2024 PM	Linkes Road	R2	0.038	B	11.5	1.0
Background 2034 PM	Linkes Road	R2	0.041	B	12.0	1.1
Construction 2024 PM	Linkes Road	R2	0.039	B	11.7	1.1
Operation 2034 PM	Linkes Road	R2	0.041	B	12.0	1.1

The SIDRA results reports can be found in **Appendix F**.

4.4 Road safety impact assessment and mitigation

4.4.1 Safe intersection sight distance

As discussed in **Section 2.7** the safe intersection sight distance and sight distances have been deemed acceptable given the safety measures in place.

4.4.2 Swept path assessment

A swept path assessment of the Biloela Callide Road / Shorts Road intersection was completed and can be seen in **Appendix G**. This drawing was completed using the Queensland Globe aerial imagery and thus exact dimensions of the intersection cannot be assured. A design vehicle of a 19m prime mover and semi-trailer has been run through the intersection.

The current intersection configuration cannot accommodate the left turn movement from the Biloela Callide Road onto Shorts Road without running off the road on the internal corner or crossing into an opposing lane. The current intersection configuration can however accommodate the right turn movement out of Shorts Road due to the intersection widening provided at the Linkes Road leg.

Due to this issue, it is expected that road alterations in the form of pavement widening would be required to accommodate the design vehicle. The expected increase in pavement area is shown in the drawing in **Appendix G**. The widening of the pavement shoulder at this turn will also require the extension of the existing culvert in the table drain which is indicated in the provided drawing.

4.4.3 Impacts on railway level crossings

The location of the railway level crossings within the assessment area are indicated in **Figure 2-6**. It should be noted that the level crossing on Shorts Road is positioned after the proposed accesses to the development site and thus will not be impacted by the development traffic.

The railway level crossing on the Dawson highway will be utilised by the heavy vehicle traffic generated by the development. These vehicles will be 100% class 9 semi-trailers with a total of 30 HV/day for the duration of the construction and 1 HV/day for the operation.

The AADT over the railway crossing with and without the development traffic is given in **Table 4-3** for use in ALCAM assessments. The background traffic and growth has been calculated using the Site ID 60067 with the 10-year growth percent applied to forecast the background conditions in 2022, 2023 and 2024.

Table 4-3 ALCAM - AADT Over Railway Crossing

AADT over railway level crossing Manton Quarry Road / Flinders Highway Intersection			
Year	Without development (background growth)	With development	No. and dimensions/type of heavy vehicles
2020 (current scenario)	302 HV/day 970 LV/day	302 HV/day 970 LV/day	153 Class 1B HV/day 62 Class 1C HV/day 87 Class 1D HV/day
Commencement of construction (2023)	307 HV/day 988 LV/day	337 HV/day 988 LV/day	155 Class 1B HV/day 93 Class 1C HV/day 89 Class 1D HV/day
Commencement of operation (2024)	313 HV/day 1008 LV/day	314 HV/day 1008 LV/day	158 Class 1B HV/day 65 Class 1C HV/day 91 Class 1D HV/day
Ten-year design horizon (2034)	367 HV/day 1180 LV/day	368 HV/day 1180 LV/day	186 Class 1B HV/day 76 Class 1C HV/day 106 Class 1D HV/day

Note: Operational daily vehicle counts represent worst case daily only as some vehicles are only generated on a greater than daily basis

5.0 PAVEMENT IMPACT ASSESSMENT

A pavement impact assessment has not been completed as part of this TIA.

6.0 CONCLUSIONS AND RECOMMENDATIONS

6.1 Summary of impacts and mitigation measures proposed

NCE have undertaken a traffic study for the proposed Callide Solar Power Station at 551 & 641 Biloela Callide Road & Shorts Road, Mount Murchison. The findings of this assessment are summarised below:

- Intersection impact assessment and mitigation
    - A SIDRA analysis found that the existing intersection configuration will be adequate in all cases including the expected year of the end of construction (2024) and the end of the operational design horizon (2034).
    - An intersection warrant assessment was completed for the turn movements into Shorts Road only. This assessment found that a basic left turn (BAL) treatment configuration is required onto Shorts Road. However, no upgrades to the existing right turn treatment are required. NCE have indicated the extent of pavement widening required to incorporate a BAL treatment in the swept path plan in **Appendix G**.

- Road safety impact assessment and mitigation
    - The current intersection provides adequate stopping sight distance for the Biloela Callide Road east and west approaches and the Shorts Road approach. Due to a bend at the end of Linkes Road the stopping sight distance is inadequate. The safety issues generated by the lack of stopping sight distance for Linkes Road are mitigated by providing adequate signage warning of the approaching intersection and stop sign.
    - The current major road approaches for the Biloela Callide Road do not provide adequate safe intersection sight distance. The safety issues generated by the lack of safe intersection sight distance are mitigated by providing adequate signage warning of the approaching intersection. As the stopping sight distance is also available in conjunction with this signage NCE suggest the intersection does not pose any major safety risks.
    - NCE have completed a swept path assessment for a design vehicle of a 19m prime mover and semi-trailer turning left into and right out of Shorts Road. The swept path assessment indicates a widening at the internal left corner is required. The extent of widening is indicated in the swept path plan in **Appendix G** along with the required extension of the culvert to incorporate the widening.
    - NCE provide the AADT data over the impacted railway level crossing for assessment of the impacted railway level crossing on the Dawson Highway.

6.2 Certification statement and authorisation

A signed Traffic Impact Assessment Certification can be found in **Appendix H**.

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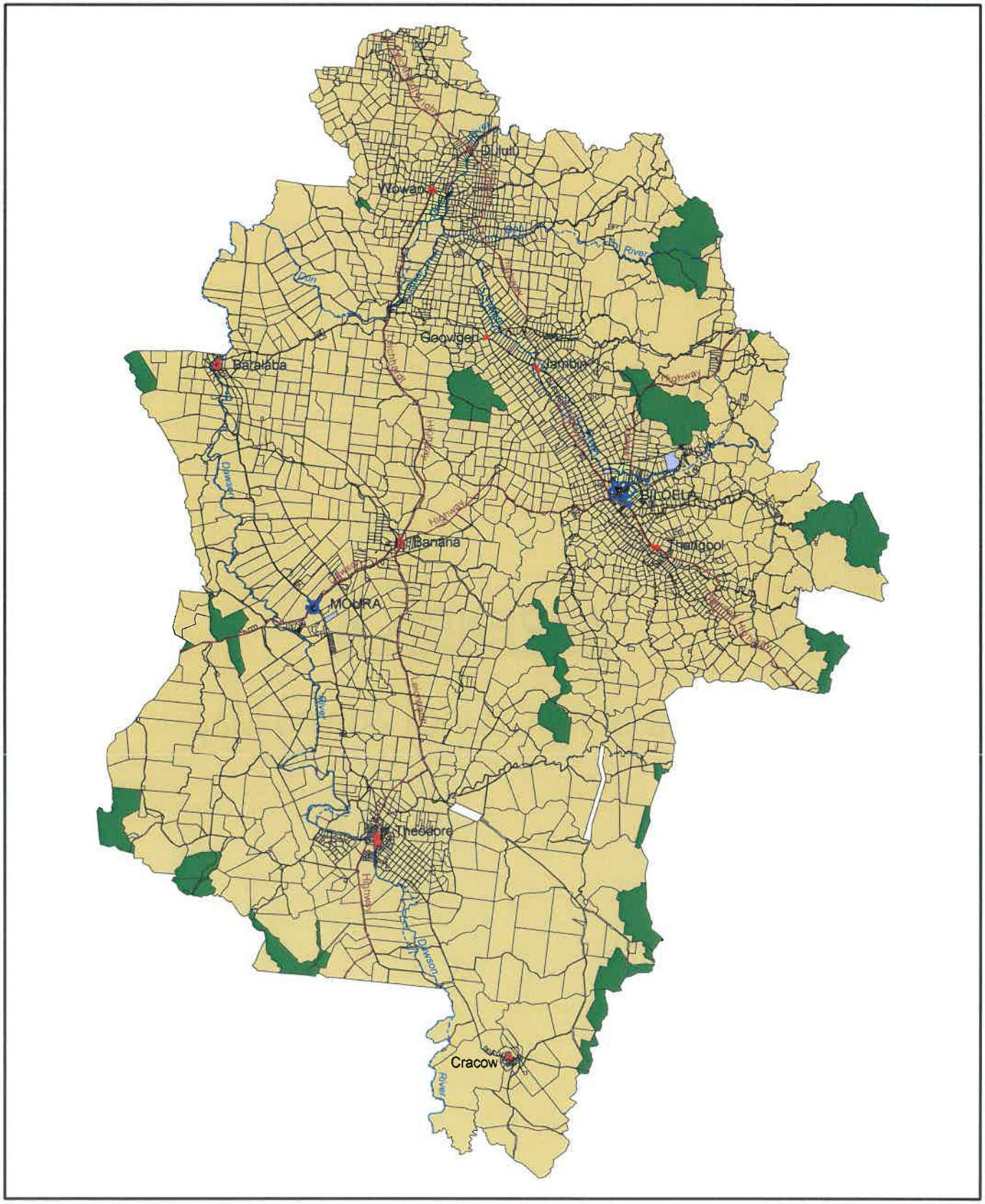
APPENDIX A

Banana Shire Council Zoning Map



ZONING
Banana Shire Planning Scheme
Map No. ZONE-1

Scale 1:650 000

10 0 10 20 30 40

Kilometres

Key

Zones

- Open Space
- Rural
- Special Industrial
- Town
- Village

Digital Cadastral Database (DCDB) supplied by Natural Resources and Mines.

June 2005

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APPENDIX B

Shorts Road Capability and Suitability Report

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SHORTS ROAD – CAPABILITY AND SUITABILITY REPORT

CALLIDE SOLAR POWER STATION

FOR
EDIFY ENERGY

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ISSUE	AUTHOR	APPROVED FOR ISSUE			ISSUED TO:	REASON
		NAME	SIGNATURE	DATE		
A	Derek Saw	Derek Saw (REPQ 7363)		17/11/2022	Edify	DRAFT For Review / Comment
B	Derek Saw	Derek Saw (REPQ 7363)		28/11/2022	Edify	In support of DA

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1.0 Executive Summary

The objective of this report is to complete an assessment of Shorts Road in terms of the infrastructure capability to service the construction and operational phases of the Callide Solar Power Station development. This assessment is limited to Shorts Road between Biloela-Callide Road and the Moura system rail line, within the jurisdiction of the Banana Shire Council.

The review of Shorts Road has been completed in accordance with ARRB – Unsealed Roads Best Practise Guide 2 (October 2020). Based upon the existing characteristics of Shorts Road, combined with the predicted traffic volumes during the construction period. Shorts Road can be classified as:

- Laying slightly above the upper bounds of a Class 4B roadway and,
- Laying within the lower bounds of a Class 4A roadway.

The results of the compliance assessment against geometric design standards assigned to Class 4B roadways found Shorts Road was considered deficient with respect to the below design parameter:

- Minimum crossfall %:
    - Shorts Road falls short of the recommended 5% crossfall.

Recommendations

1. Shorts Road profile (crossfall) can be improved by importing a suitable gravel overlay material, incorporating into the existing granular profile and re-shaping to introduce the preferred cross fall grades
2. Signage relating the isolated humps within the roadway section can be installed to improve awareness for unfamiliar drivers.
3. Improvement in some aspects of the drainage and potential increased maintenance during the construction period would assisting in limiting or eliminating surface erosion through runoff scouring and reduce the potential for rutting and shoving in the few locations observed.

2.0 DEVELOPMENT CONTEXT

2.1 Site details

The site is located at Shorts Road, Biloela, within the jurisdiction of the Banana Shire Council.

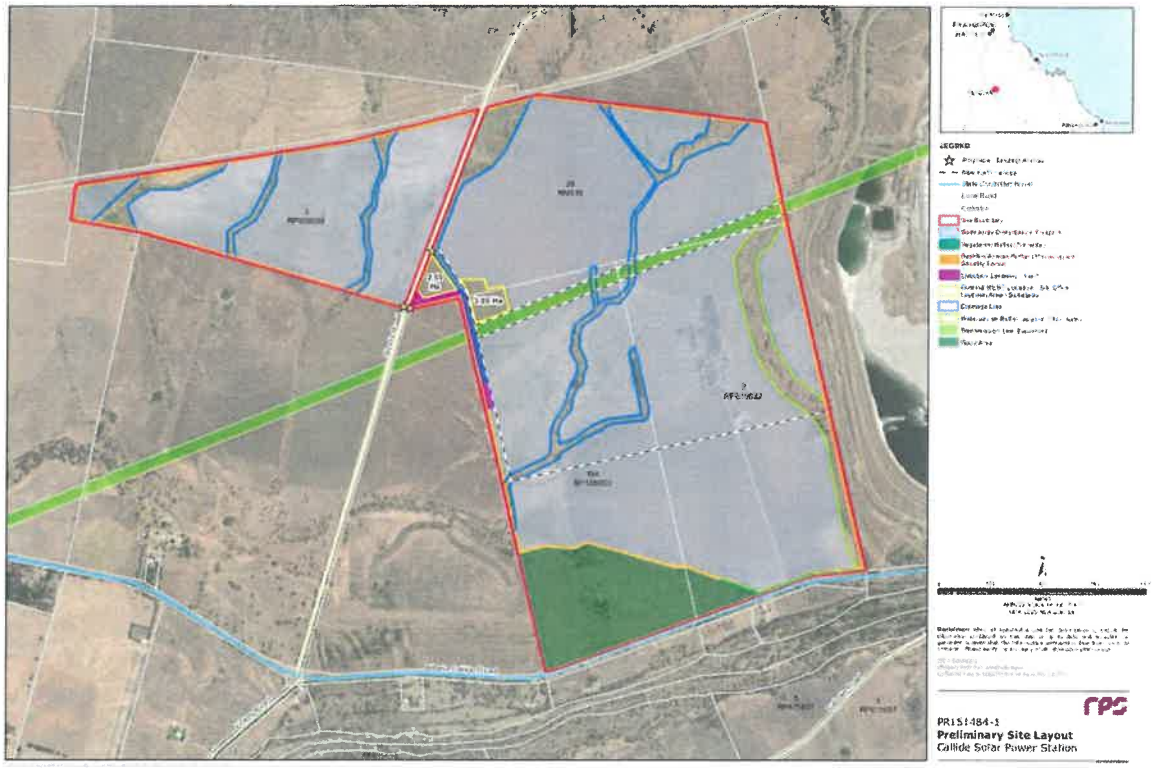

Figure 2-1 - Site Location Map

2.2 Site Access

Access to the site for the purposes of delivering materials for the construction of the solar farm will be obtained via the Dawson Highway, Biloela-Callide Road and Shorts Road.

During the operational phase vehicles are expected to access the site via Links Road and or the Biloela-Callide Road then Shorts Road.

Utilisation of Shorts Road will occur between Ch00 (Biloela-Callide Road) and Ch 2,940 (Moura System Rail Line).

The Main access route will occur over Ch00 to Ch1,870/Ch2,160 being the point at which construction traffic will exit Shorts Road to access the construction lay down area, construction hub, substation and Maintenance operations.

Shorts Road between Ch2,160 and Ch2,940 may be utilised to access Lot 3 on RP608599 via secondary access gates.

3.0 Roadway Classification assessment

An assessment of Shorts Road has been completed in accordance with ARRB – Unsealed Roads Best Practise Guide 2 (October 2020).

Table 3-1 Unsealed Roads – Table 3.9 Classification system.

Table 3.9: Unsealed roads classification system

Road class	Class type	Service function description	Road type description
4A	Main road > 150 ADT	This type of road is used for major movements between population centres and connection to adjacent areas. High traffic volumes occur, and the road can carry large vehicles.	• All weather road, predominantly two-lane and unsealed. Can be sealed if economically justified. • Operating speed standard of 50–80 km/h according to terrain. • Minimum carriageway width is 7 m.
4B	Minor road 50–150 ADT	This type of road is used for connection between local centres of population and links to the primary network. Roads may or may not be sealed depending on the importance and function of the road.	• All-weather two-lane road formed and gravelled or single-lane sealed road with gravel shoulders. • Operating speed standard of 30–70 km/h according to terrain. • Minimum carriageway width is 5.5 m.
4C	Access road 10–50 ADT	Provides access to low use areas or individual rural property sites and forest areas. Caters for low travel speed and a range of vehicles and may be seasonally closed.	• Substantially a single lane two-way, generally dry weather, formed road. • Operating speeds standard of < 20–40 km/h according to terrain. • Minimum carriageway width is 4 m. • May be restricted to four-wheel drive vehicles.
4D	Tracks < 10 ADT	Mainly used for fire protection purposes, management access and limited recreational activities.	• Predominantly a single-lane two-way earth track (unformed) at or near the natural surface level. • Predominantly not conforming to any geometric design standards. • Minimum cleared width is 3 m. • Primarily for four-wheel drive vehicles.

3.1.1 Roadway data summary

3.1.2 Road Width:

7.0m to 7.5m typical carriageway width and in excess of the minimum 1.5m clear zone provided in almost all situations. The carriageway width is considered adequate for the purposes of two-way traffic volumes

3.1.3 Road Surface:

Gravel construction (pavement profile depths unknown), whilst the granular material varies in nature throughout the length of Shorts Road, it appears to be reasonably well graded for the purposes of a gravel wearing course with a mix of fine material and larger granular stone to balance skid resistance and dust emission. The formation/profile lacks sufficient cross fall to adequately shed stormwater and has resulted in isolated areas that display ponded run-off and locations in which surface deformation (sub-grade rutting) is evident. The lack of cross fall is also contributing to longitudinal scouring and loss of road base materials through longitudinal drainage paths eroding the granular surface.

3.1.4 Road Drainage:

Table drains and outlet drains evident in areas, maintenance required.

3.1.5 Road Horizontal alignment:

The horizontal alignment of Shorts Road is described as straight. A small deflection of between 4 and 5 degrees is located at Ch 1,870 which coincides with the main access to the Callide SPS substation and Site Office.

3.1.6 Road Vertical alignment:

Generally speaking, the vertical alignment of Shorts Road can be described as flat. There are however (3) isolated short humps in the roadway. These humps are assumed to serve as diversion structures associated with stormwater run-off.

3.1.7 Development Generated Traffic:

Construction phase traffic is expected to generate 112 total vehicle movement per day.

- 30 (Class 9) Heavy vehicles per day (Average payload 8.875 tonne) one way
- 26 (Class 2/3) Light vehicles per day one way

Operational phase traffic is expected to generate 8 total vehicle movement per day.

- 1 (Class 9) Heavy vehicles per day (Average payload 26) one way
- 7 (Class 2/3) Light vehicles per day one way

Whilst a specific Traffic Movement survey has not been completed with respect to Shorts Road it is expected the AADT for this section of roadway would be between 50 and 80 veh/day based upon 10 trips per residence plus rural farming traffic.

Therefore, an estimated AADT during the construction period is between 162 and 192 veh/day would be considered reasonable. The volume of Operational traffic is considered to be negligible and therefore no further analysis of those traffic loads is warranted.

3.1.8 Roadway Classification Determination

Based upon the existing characteristics of Shorts Road listed above combined with the predicted traffic volumes during the construction period. Shorts Road can be classified as:

Laying slightly above the upper bounds of a Class 4B roadway and,

Laying within the lower bounds of a Class 4A roadway.

3.2 Compliance Assessment Results

Based upon a compliance assessment against Type 4B and 4A classified unsealed roads, it can be seen from Table 3.9, that shorts Road satisfies the majority of geometric design standards.

Table 3-2 Unsealed Roads – Table 3.9 Classification system.

Table 3.10: Guidelines for the main geometric design standards for unsealed roads

Road classification	4A Main			4B Minor			4C Access			4D Tracks			Comments
Terrain type	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	
	Main geometric characteristic *based on safety, costs and environmental considerations*												
Operating speed value (km/h)	80	70	50	70	50	30	60	40	20	N/A	N/A	N/A	Based on 85th percentile speed
	Cross-section elements												
Number of traffic lanes	2	2	2	2	2	2	1	1	1	1	1	1	Unsealed lanes
Minimum cross fall (%)	5	5	5	5	5	5	5	5	5	4	4	4	Min. of 4% to drain rainfall off tracks
Maximum superelevation (%)[1]	6	7	8	6	8	10	6	8	10	N/A	N/A	N/A	
Minimum traffic lane width (m)[2]	3.5	3	3	3	3	3	3	3	3	3	3	3	
Minimum shoulder width (m)	1	1	0.5	0.5	0.5	0.5	1.5	1	0.5	0	0	0	
Minimum carriageway width (lanes + shoulder) (m)	9	8	7	7	7	7	6	5	4	3	3	3	
Minimum formation width (including verges) (m)[3]	11	10	9	9	9	9	8	7	6	3	3	3	
	Horizontal geometry												
Minimum curve radius (m)[4]	320	250	140	250	100	35	170	60	15	N/A	N/A	N/A	

Table 3-3 Unsealed Roads – Table 3.9 Classification system.

Road classification	4A Main			4B Minor			4C Access			4D Tracks			Comments
Terrain type	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	Flat	Rolling	Mountainous	
Minimum stopping sight distance (m)[5]	150	120	70	120	70	30	90	50	30	N/A	N/A	N/A	
Minimum meeting sight distance (m)[6]	290	230	130	230	130	60	180	100	60	N/A	N/A	N/A	
	Vertical geometry												
Maximum vertical grade (%)[7]	6	8	12	6	8	12	6	8	12	N/A	N/A	N/A	Avoid steep grades to reduce soil erosion along tracks
Minimum crest vertical curve (K value)[8]	50	30	10	30	10	5	19	8	2	N/A	N/A	N/A	
Minimum sag vertical curve (K value)[9]	11	8	4	8	4	3	8	3	2	N/A	N/A	N/A	

1 The maximum superelevation value will need to consider the number of loaded heavy vehicles, speed and curve radii
2 In cases where there are a high percentage of heavy vehicles (> 20%) minimum lane widths can be increased by 0.5 m
3 Allows for 1 m verge/table drain width. This must be reviewed based on actual locations where for drainage reasons greater width may be required
4 Values rounded up. For minimum radius curves, widening on the inside of a curve may be necessary to accommodate longer vehicles
5 Based on a reaction time of 2 seconds and surface coefficients relating to unsealed surfaces. Values rounded up. Values based on flat grades and allowances will need to be made for up and down grades
6 This is mainly a requirement for single lane two way roads. Values rounded up
7 In some cases, higher grades of up to 20% can be allowed for short sections (about 150 m). Keep grades on unsealed roads lower due to ravelling and scouring of surface
8 Calculation of these values is to be based on information contained in Austroads (2016c). The length of the vertical curve (L) is based on the product of K multiplied by the algebraic difference in grades percentage A (i.e. L = K × A)
9 Sag values are based on comfort control criteria

3.2.1 Identified deficiencies

The results of the compliance assessment against geometric design standards assigned to Class 4A and 4B roadways found Shorts Road was considered deficient with respect to the below design parameter:

- Minimum crossfall %:
  - Shorts Road falls short of the recommended 5% crossfall.
- Minimum carriageway:
  - Compliance with the 4A parameter's is not achieved.
- Minimum formation:
  - Compliance with the 4A parameter's is not achieved.
  - .

Whilst compliance with carriageway and formation widths are not compliant with Road class 4A. Reference to the estimated traffic volumes being just outside the 4B range and the construction period having a defined duration. It is considered suitable to assess Shorts Road against Class 4B criteria in this instance with respect to carriageway width and formation width. Both of which are considered reasonable in terms of the composition of traffic and volume.

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4.0 Visual Inspection and Video Record of Shorts Road

Shorts Road, were inspected/recorded on the 4th October 2022.

Inspecting personal:

- Damien Krauklis (Edify Energy)

Inspection technique:

- Go-Pro camera mounted on the front of inspection vehicle.

A digital copy of the video recording is available from Northern Consulting Engineers upon request.

The following criteria were utilised to classify existing failures within the road section.

- Deformation (Erosion/Scouring)
- Deformation (Shoving/Rutting)
- Deformation (Potholes)
- Fatigue Cracking (Sealed roads)

Nominated areas for comparison with subsequent dilapidation inspections and video records

Criteria	Chainage
Deformation (Erosion/Scouring)	(1,200-1,500), (1,600-1,870), (1,870-2,750)
Deformation (Shoving/Rutting)	2,800 (Subgrade failure)
Deformation (Potholes)	05
Fatigue Cracking (Sealed intersection)	-02

Snapshots of images recorded during the inspection have been included herewith for convenience, however it is anticipated that a full comparison of the pre and post construction videos will be undertaken following the completion of the construction period.

Figure 4-1 – Shorts Road Ch-02 – Biloela, QLD (Fatigue cracking above culvert – Sealed intersection)

Figure 4-2 – Shorts Road Ch00 – Biloela, QLD

Figure 4-3 – Shorts Road Ch260 – Biloela, QLD

Figure 4-4 – Shorts Road Ch430 – Biloela, QLD

Figure 4-5 – Shorts Road Ch770 – Biloela, QLD

Figure 4-6 – Shorts Road Ch930 – Biloela, QLD

Figure 4-7 – Shorts Road Ch1575 – Biloela, QLD

Figure 4-8 – Shorts Road Ch1855 – Biloela, QLD

Figure 4-9 – Shorts Road Ch2000 – Biloela, QLD

Figure 4-10 – Shorts Road Ch2100 – Biloela, QLD

Figure 4-11 – Shorts Road Ch4210 – Biloela, QLD

Figure 4-12 – Shorts Road Ch2510 – Biloela, QLD

Figure 4-13 – Shorts Road Ch2800 – Biloela, QLD

5.0 Conclusions and Recommendations

Northern Consulting Engineers have completed a compliance assessment of Shorts Road against ARRB – Unsealed Roads Best Practise Guide 2 (October 2020) in order to determine the suitability of the road to operate effectively as the Access / Haul route during the Construction phase and Operational phase of the Callide Solar Power Station development.

5.1.1 Conclusion

The results of the compliance assessment found:

Shorts Road was considered deficient with respect to the below design parameter:

- Minimum crossfall %:
  - Shorts Road falls short of the recommended 5% crossfall.

5.1.2 Recommendations

4. Shorts Road profile (crossfall) is recommended to be improved by importing a suitable gravel overlay material. The existing surface should be tyned and mixed with the new material, compacted and shaped to introduce the preferred 5% crossfall over the roadway section to be trafficked during the construction period.

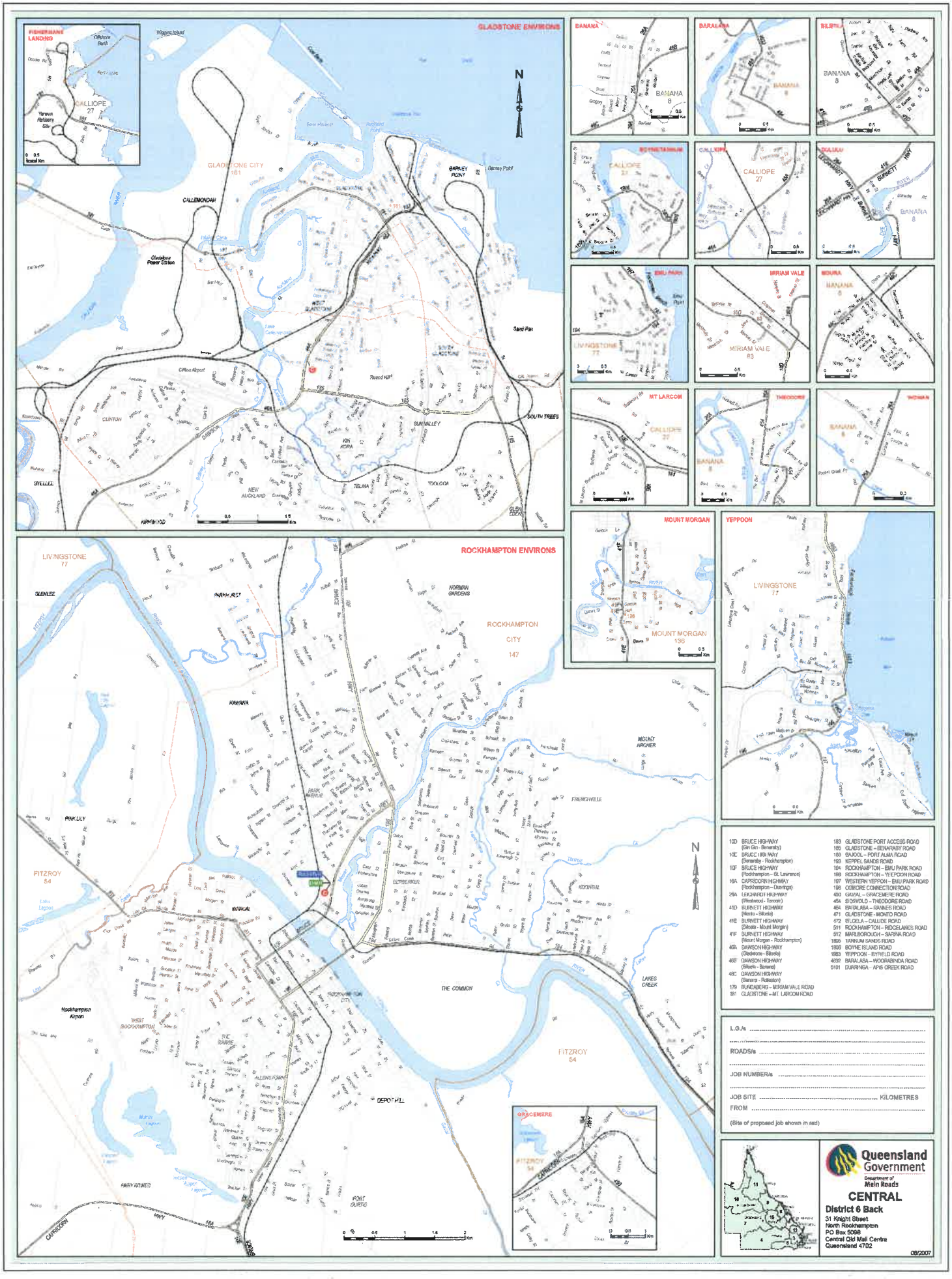
5. Signage of the isolated humps within the roadway section could be installed to improve awareness for unfamiliar drivers.

6. Improvement in some aspects of the drainage and potential increased maintenance during the construction period would assisting in limiting or eliminating surface erosion through runoff scouring and reduce soaking of subgrades leading to formal rutting and shoving in the few locations observed.

NORTHERN CONSULTING engineers

APPENDIX C

Central Region (D6) District Mapping - Queensland Department of Transport and Main Roads (TMR)



10D BRUCE HIGHWAY (Gin Gin - Benaraby)
10C BRUCE HIGHWAY (Benaraby - Rockhampton)
10F BRUCE HIGHWAY (Rockhampton - St. Lawrence)
16A CAPRICORN HIGHWAY (Rockhampton – Duaringa)
29A LEICHHARDT HIGHWAY (Westwood - Taroom)
41D BURNETT HIGHWAY (Monto - Biloela)
41E BURNETT HIGHWAY (Biloela - Mount Morgan)
41F BURNETT HIGHWAY (Mount Morgan - Rockhampton)
46A DAWSON HIGHWAY (Gladstone - Biloela)
46B DAWSON HIGHWAY (Biloela - Banana)
46C DAWSON HIGHWAY (Banana - Rolleston)
179 BUNDABERG – MIRIAM VALE ROAD
181 GLADSTONE – MT. LARCOM ROAD
183 GLADSTONE PORT ACCESS ROAD
185 GLADSTONE – BENARABY ROAD
188 BUNDOL – PORT ALMA ROAD
193 KEPPEL SANDS ROAD
194 ROCKHAMPTON – EMU PARK ROAD
198 ROCKHAMPTON – YEPPOON ROAD
197 WESTERN YEPPOON – EMU PARK ROAD
196 CQMORE CONNECTION ROAD
450 GRAVAL – GRACEMERE ROAD
454 EIDSVOLD – THEODORE ROAD
464 BARALABA – RANNES ROAD
471 GLADSTONE – MONTO ROAD
672 BILOELA – CALLIOE ROAD
511 ROCKHAMPTON – RIDGELANDS ROAD
512 MARLBOROUGH – SARINA ROAD
1806 TANNUM SANDS ROAD
1808 BOYNE ISLAND ROAD
1883 YEPPOON – BYFIELD ROAD
4692 BARALABA – WOORABINDA ROAD
5101 DUARINGA – APIS CREEK ROAD

L.G./s .....
ROADS/s .....
JOB NUMBER/s .....
JOB SITE ..... KILOMETRES
FROM .....
(Site of proposed job shown in red)

Queensland Government
Department of Main Roads
CENTRAL
District 6 Back
31 Knight Street
North Rockhampton
PO Box 5098
Central Old Mail Centre
Queensland 4702

08/2007

NORTHERN CONSULTING engineers

APPENDIX D

TMR Traffic Analysis and Reporting System (TARS) Data

DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

Region	Central Queensland	All
District	404 - Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	46A - DAWSON HIGHWAY ([illegible] -	All
Road Section Id	All	
Site Type	C - Coverage	All
Site	61084 - 600m S [illegible] Rd, Mt M..	61..
Thru Distance	114.33	
Gazette Direction	TA - Thru traffic - against gazettal	TA
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Wednesday 1[illegible]-May-2..	End Date: Monday 06-Jun-2022

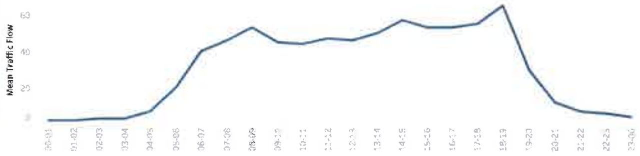
Select Time Window: 24 Hour

Days in Range: [illegible] Days Included: 20

	Daily Average Volume	Proportion
12-hour flow	610.0	82.14%
16-hour flow	[illegible]	95.97%
18-hour flow	[illegible]	95.19%
24-hour flow	742.7	100.00%

Site Type ■ C

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	1	0.5%	1	0.5%	1	0.5%	1	0.5%	4	0.5%
01-02	1	0.5%	1	0.5%	1	0.5%	1	0.5%	4	0.5%
02-03	0	0.0%	2	1.0%	1	0.5%	1	0.5%	4	0.5%
03-04	1	0.5%	1	0.5%	1	0.5%	1	0.5%	4	0.5%
04-05	1	0.5%	2	1.0%	1	0.5%	3	1.6%	7	0.9%
05-06	3	1.5%	5	2.5%	6	3.3%	7	3.6%	21	2.7%
06-07	8	4.1%	9	4.5%	11	6.0%	13	6.7%	41	5.3%
07-08	16	8.2%	11	5.5%	9	4.9%	11	5.7%	47	6.1%
08-09	14	7.2%	16	8.0%	11	6.0%	12	6.2%	53	6.9%
09-10	10	5.1%	12	6.0%	12	6.5%	12	6.2%	46	6.0%
10-11	11	5.6%	12	6.0%	12	6.5%	10	5.2%	45	5.8%
11-12	12	6.2%	12	6.0%	12	6.5%	12	6.2%	48	6.2%
12-13	12	6.2%	11	5.5%	12	6.5%	13	6.7%	48	6.2%
13-14	11	5.6%	13	6.5%	12	6.5%	14	7.3%	50	6.5%
14-15	12	6.2%	15	7.5%	18	9.8%	13	6.7%	58	7.5%
15-16	13	6.7%	12	6.0%	13	7.1%	15	7.8%	53	6.9%
16-17	14	7.2%	13	6.5%	14	7.6%	13	6.7%	54	7.0%
17-18	15	7.7%	16	8.0%	12	6.5%	13	6.7%	56	7.3%
18-19	16	8.2%	21	10.5%	13	7.1%	17	8.8%	67	8.7%
19-20	16	8.2%	7	3.5%	4	2.2%	3	1.6%	30	3.9%
20-21	3	1.5%	4	2.0%	3	1.6%	3	1.6%	13	1.7%
21-22	2	1.0%	2	1.0%	2	1.1%	2	1.0%	8	1.0%
22-23	2	1.0%	1	0.5%	2	1.1%	2	1.0%	7	0.9%
23-00	1	0.5%	1	0.5%	1	0.5%	1	0.5%	4	0.5%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

Region	Central Queensland	All
District	404 - Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	472 - BN 95LA - CALLIDE ROAD	All
Road Section Id	All	
Site Type	C - Commercial	All
Site	61298 - 275m East Shorts Rd, Mount Mur...	61...
Thru Distance	4.26	
Gazetta Direction	TG - Thru traffic in gazettal dirn	TG
Traffic Class	00 - All Vehicles	
Data Range (inclusive)	Start Date: Thursday 10-Feb-2022 — End Date: Tuesday 08-Mar-2022	

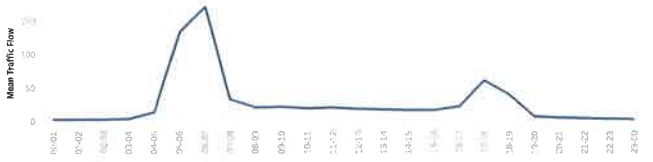
Select Time Window: 24 Hour

Days in Range: 27 Days Included: 27

	Daily Average Volume	Proportion
12-hour flow	285.3	40.13%
16-hour flow	467.3	76.52%
18-hour flow	470.2	76.01%
24-hour flow	636.6	100.00%

Site Type: C

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Total Volume	Volume %
00-01	0	0.0%	0	0.0%	1	0.5%	1	0.7%	2	0.3%
01-02	0	0.0%	1	0.7%	0	0.0%	0	0.0%	1	0.2%
02-03	0	0.0%	0	0.0%	0	0.0%	1	0.7%	1	0.2%
03-04	1	0.7%	0	0.0%	1	0.5%	1	0.7%	3	0.5%
04-05	1	0.7%	2	1.3%	3	1.5%	6	4.0%	12	1.9%
05-06	14	9.5%	26	17.1%	55	28.1%	39	26.2%	134	20.8%
06-07	44	29.7%	44	28.9%	53	27.0%	30	20.1%	171	26.5%
07-08	10	6.8%	8	5.3%	7	3.6%	6	4.0%	31	4.8%
08-09	5	3.4%	5	3.3%	5	2.6%	5	3.4%	20	3.1%
09-10	5	3.4%	5	3.3%	5	2.6%	5	3.4%	20	3.1%
10-11	4	2.7%	5	3.3%	5	2.6%	4	2.7%	18	2.8%
11-12	4	2.7%	5	3.3%	6	3.1%	5	3.4%	20	3.1%
12-13	5	3.4%	4	2.6%	5	2.6%	5	3.4%	19	2.9%
13-14	4	2.7%	4	2.6%	5	2.6%	4	2.7%	17	2.6%
14-15	4	2.7%	4	2.6%	4	2.0%	4	2.7%	16	2.5%
15-16	4	2.7%	5	3.3%	3	1.5%	4	2.7%	16	2.5%
16-17	5	3.4%	5	3.3%	6	3.1%	6	4.0%	22	3.4%
17-18	9	6.1%	14	9.2%	22	11.2%	16	10.7%	61	9.5%
18-19	23	15.5%	10	6.6%	5	2.6%	2	1.3%	40	6.2%
19-20	2	1.4%	2	1.3%	2	1.0%	1	0.7%	7	1.1%
20-21	1	0.7%	1	0.7%	1	0.5%	1	0.7%	4	0.6%
21-22	1	0.7%	1	0.7%	1	0.5%	1	0.7%	4	0.6%
22-23	1	0.7%	0	0.0%	1	0.5%	1	0.7%	3	0.5%
23-00	1	0.7%	1	0.7%	0	0.0%	1	0.7%	3	0.5%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

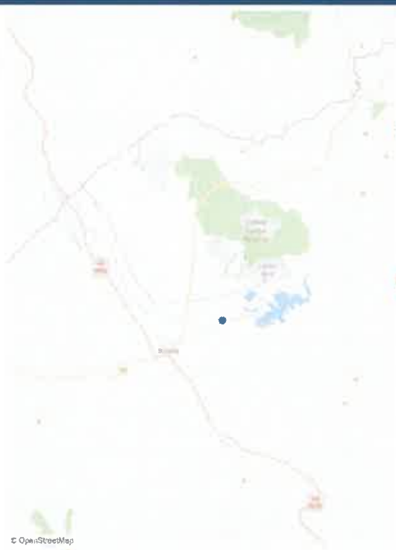
Click on this reset icon to reset all filters to default settings

Region	Central Queensland	All
District	404 - Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	472 - BILOELA - CALLIDE ROAD	All
Road Section Id	All	
Site Type	C - Coverage	All
Site	E129e - 275m E of St...rts Rd, Mount Mur...	61...
Thru Distance	4.26	
Gazette Direction	TG - Traffic in gazettal direction	TG
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Thursday 10-Feb-2022	End Date: Tuesday 08-Mar-2022

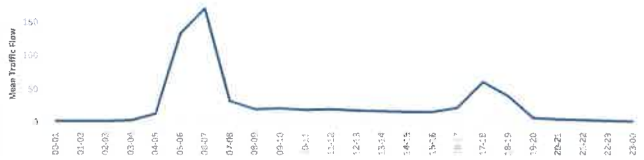
Select Time Window: 24 Hour

Days in Range: 27 Days Included: 27

	Daily Average Volume	Proportion
12-hour flow	[illegible]	[illegible]
16-hour flow	467.3	75.54%
18-hour flow	470.2	76.01%
24-hour flow	618.6	100.00%



	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	0	0.0%	0	0.0%	1	0.5%	1	0.7%	2	0.3%
01-02	0	0.0%	1	0.7%	0	0.0%	0	0.0%	1	0.2%
02-03	0	0.0%	0	0.0%	0	0.0%	1	0.7%	1	0.2%
03-04	1	0.7%	0	0.0%	1	0.5%	1	0.7%	3	0.5%
04-05	1	0.7%	2	1.3%	3	1.5%	6	4.0%	12	1.9%
05-06	14	9.5%	26	17.1%	55	28.1%	39	26.2%	134	20.8%
06-07	44	29.7%	44	28.9%	53	27.0%	30	20.1%	171	26.5%
07-08	10	6.8%	8	5.3%	7	3.6%	6	4.0%	31	4.8%
08-09	5	3.4%	5	3.3%	5	2.6%	5	3.4%	20	3.1%
09-10	5	3.4%	5	3.3%	5	2.6%	5	3.4%	20	3.1%
10-11	4	2.7%	5	3.3%	5	2.6%	4	2.7%	18	2.8%
11-12	4	2.7%	5	3.3%	6	3.1%	5	3.4%	20	3.1%
12-13	5	3.4%	4	2.6%	5	2.6%	5	3.4%	19	2.9%
13-14	4	2.7%	4	2.6%	5	2.6%	4	2.7%	17	2.6%
14-15	4	2.7%	4	2.6%	4	2.0%	4	2.7%	16	2.5%
15-16	4	2.7%	5	3.3%	3	1.5%	4	2.7%	16	2.5%
16-17	5	3.4%	5	3.3%	6	3.1%	6	4.0%	22	3.4%
17-18	9	6.1%	14	9.2%	22	11.2%	16	10.7%	61	9.5%
18-19	[illegible]	15.5%	10	6.6%	5	2.6%	2	1.3%	40	6.2%
19-20	2	1.4%	2	1.3%	2	1.0%	1	0.7%	7	1.1%
20-21	1	0.7%	1	0.7%	1	0.5%	1	0.7%	4	0.6%
21-22	1	0.7%	1	0.7%	1	0.5%	1	0.7%	4	0.6%
22-23	1	0.7%	0	0.0%	1	0.5%	1	0.7%	3	0.5%
23-00	1	0.7%	1	0.7%	0	0.0%	1	0.7%	3	0.5%



DAILY VOLUME REPORT

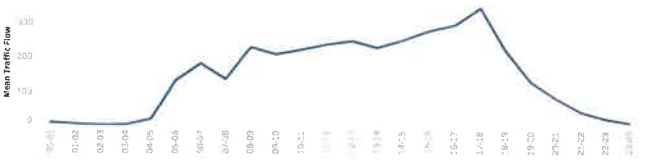
Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

Region	Central Queensland	All
District	404 - Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	46A - DAWSON HIGHWAY (GLADSTONE - ...	All
Road Section Id		All
Site Type	C - coverage	All
Site	[redacted]	16...
Thru Distance	129.27	
Gazette Direction	[redacted]	TA
Traffic Class	03 - All Vehicles	
Date Range (inclusive)	Start Date: Thursday 19-May-2022	End Date: Monday 06-Jun-2022

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	3	0.3%	4	0.4%	2	0.2%	2	0.2%	11	0.3%
01-02	3	0.3%	2	0.2%	1	0.1%	2	0.2%	8	0.2%
02-03	1	0.1%	1	0.1%	1	0.1%	1	0.1%	4	0.1%
03-04	2	0.2%	1	0.1%	1	0.1%	2	0.2%	6	0.2%
04-05	3	0.3%	3	0.3%	5	0.6%	10	1.2%	21	0.6%
05-06	28	2.0%	33	3.6%	38	4.3%	45	5.2%	134	3.8%
06-07	49	5.4%	55	6.0%	47	5.4%	32	3.7%	183	5.2%
07-08	29	3.2%	33	3.6%	36	4.1%	38	4.4%	136	3.8%
08-09	45	5.0%	61	6.7%	65	7.4%	58	6.8%	229	6.4%
09-10	49	5.4%	55	6.0%	54	6.2%	51	5.9%	209	5.9%
10-11	55	6.1%	56	6.1%	56	6.4%	56	6.5%	223	6.3%
11-12	59	6.5%	61	6.7%	59	6.7%	58	6.8%	237	6.7%
12-13	69	7.6%	64	7.0%	59	6.7%	55	6.4%	247	7.0%
13-14	64	7.1%	58	6.4%	52	5.9%	53	6.2%	227	6.4%
14-15	57	6.3%	53	5.8%	62	7.1%	77	9.0%	249	7.0%
15-16	78	8.6%	73	8.0%	61	7.0%	64	7.5%	276	7.8%
16-17	72	7.9%	72	7.9%	73	8.3%	77	9.0%	294	8.3%
17-18	98	10.8%	84	9.2%	84	9.6%	78	9.1%	344	9.7%
18-19	60	6.6%	63	6.9%	56	6.4%	44	5.1%	223	6.3%
19-20	40	4.4%	37	4.1%	29	3.3%	24	2.8%	130	3.7%
20-21	26	2.8%	21	2.3%	19	2.2%	17	2.0%	82	2.3%
21-22	15	1.7%	12	1.3%	9	1.0%	7	0.8%	43	1.2%
22-23	8	0.9%	6	0.7%	5	0.6%	5	0.6%	24	0.7%
23-00	5	0.6%	3	0.3%	2	0.2%	2	0.2%	12	0.3%



Select Time Window: 24 Hour

Days in Range: 19 Days Included: 19

	Daily Average Volume	Proportion
12-hour flow	2,977	93.70%
16-hour flow	3,310	94.02%
18-hour flow	3,344	94.96%
24-hour flow	3,521	100.00%

Site Type: C

DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on the reset icon to reset all filters to default settings

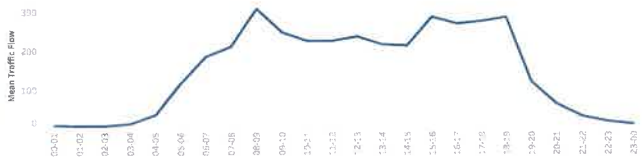
Region	Central Queensland	All
District	[illegible]	All
Local Government	[illegible]	All
Road Section Name	[illegible]	All
Road Section Id	All	
Site Type	C - Coverage	All
Site	[illegible]	16.
Thru Distance	[illegible]	
Gazette Direction	TG - Thru traffic in gazetted dirn	TG
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Thursday 19 May 2022	End Date: Monday 06 Jun 2022

Select Time Window: 24 Hour

Days in Range: [illegible] Days Included: 18

	Daily Average Volume	Proportion
12-hour flow	2 967	93.59%
16-hour flow	3 351	94.31%
18-hour flow	3 480	95.03%
24-hour flow	3 559	100.00%

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	2	0.2%	2	0.2%	2	0.2%	2	0.2%	8	0.2%
01-02	1	0.1%	1	0.1%	2	0.2%	2	0.2%	6	0.2%
02-03	1	0.1%	2	0.2%	1	0.1%	2	0.2%	6	0.2%
03-04	4	0.3%	3	0.3%	3	0.4%	4	0.4%	13	0.4%
04-05	5	0.5%	4	0.4%	9	1.1%	17	1.9%	35	1.0%
05-06	17	1.8%	27	3.0%	31	3.7%	41	4.5%	116	3.2%
06-07	44	4.7%	42	4.7%	39	4.6%	59	6.4%	184	5.1%
07-08	51	5.5%	46	5.1%	49	5.8%	65	7.1%	211	5.9%
08-09	62	6.7%	87	9.7%	78	9.2%	78	8.5%	305	8.5%
09-10	64	6.9%	57	6.3%	63	7.4%	62	6.8%	246	6.9%
10-11	54	5.8%	58	6.4%	55	6.5%	57	6.2%	224	6.2%
11-12	53	5.7%	57	6.3%	55	6.5%	59	6.4%	224	6.2%
12-13	61	6.6%	57	6.3%	55	6.5%	61	6.7%	234	6.5%
13-14	55	5.9%	52	5.8%	50	5.9%	58	6.3%	215	6.0%
14-15	53	5.7%	52	5.8%	54	6.4%	53	5.8%	212	5.9%
15-16	95	10.2%	65	7.2%	61	7.2%	64	7.0%	285	7.9%
16-17	67	7.2%	67	7.4%	67	7.9%	67	7.3%	268	7.5%
17-18	70	7.6%	70	7.8%	68	8.0%	66	7.2%	274	7.6%
18-19	87	9.4%	88	9.8%	54	6.4%	55	6.0%	284	7.9%
19-20	46	5.0%	32	3.6%	24	2.8%	18	2.0%	120	3.3%
20-21	18	1.9%	17	1.9%	14	1.7%	13	1.4%	62	1.7%
21-22	10	1.1%	7	0.8%	6	0.7%	7	0.8%	30	0.8%
22-23	5	0.5%	4	0.4%	4	0.5%	4	0.4%	17	0.5%
23-00	3	0.3%	3	0.3%	2	0.2%	2	0.2%	10	0.3%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

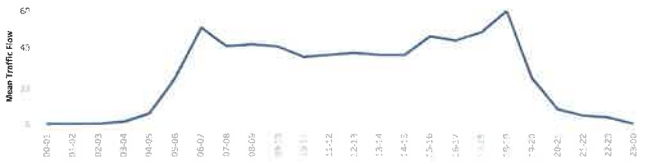
Region	Central Queensland	All
District	464 - Fitzroy	All
Local Government	Burnett Shire	All
Road Section Name	4EA - DAWSON HIGHWAY (C) AUSTONE -...	All
Road Section Id	All	
Site Type	C - Coverage	All
Site	[redacted]	60..
Thru Distance	105.09	
Gazette Direction	TA - Thru traffic - against gazettal	TA
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Saturday 09-Feb-2019	End Date: Thursday 28-Feb-2019

Select Time Window: 24 Hour

Days in Range: 20 Days Included: 20

	Daily Average Volume	Proportion
12-hour flow	490.7	77.37%
16-hour flow	581.4	91.67%
18-hour flow	589.7	92.98%
24-hour flow	634.2	100.00%

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%
01-02	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%
02-03	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%
03-04	1	0.6%	1	0.6%	2	1.2%	1	0.6%	5	0.8%
04-05	2	1.2%	2	1.2%	3	1.8%	3	1.9%	10	1.5%
05-06	4	2.3%	6	3.7%	8	4.7%	8	5.0%	26	3.9%
06-07	10	5.8%	12	7.5%	15	8.9%	13	8.2%	50	7.6%
07-08	12	6.9%	9	5.6%	9	5.3%	10	6.3%	40	6.0%
08-09	11	6.4%	10	6.2%	11	6.5%	10	6.3%	42	6.3%
09-10	11	6.4%	11	6.8%	10	5.9%	9	5.7%	41	6.2%
10-11	10	5.8%	9	5.6%	9	5.3%	8	5.0%	36	5.4%
11-12	10	5.8%	8	5.0%	9	5.3%	10	6.3%	37	5.6%
12-13	9	5.2%	9	5.6%	10	5.9%	10	6.3%	38	5.7%
13-14	9	5.2%	9	5.6%	9	5.3%	10	6.3%	37	5.6%
14-15	9	5.2%	9	5.6%	10	5.9%	10	6.3%	38	5.7%
15-16	11	6.4%	14	8.7%	11	6.5%	10	6.3%	46	6.9%
16-17	11	6.4%	11	6.8%	10	5.9%	11	6.9%	43	6.5%
17-18	12	6.9%	12	7.5%	13	7.7%	11	6.9%	48	7.3%
18-19	18	10.4%	12	7.5%	16	9.5%	11	6.9%	57	8.6%
19-20	12	6.9%	6	3.7%	4	2.4%	4	2.5%	26	3.9%
20-21	3	1.7%	3	1.9%	2	1.2%	3	1.9%	11	1.7%
21-22	2	1.2%	2	1.2%	2	1.2%	2	1.3%	8	1.2%
22-23	2	1.2%	2	1.2%	2	1.2%	1	0.6%	7	1.1%
23-00	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

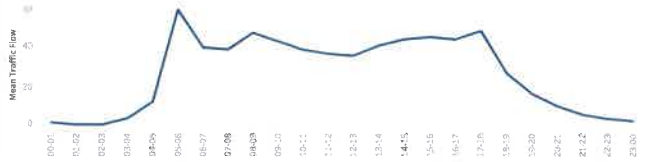
Region	All
District	All
Local Government	All
Road Section Name	All
Road Section Id	All
Site Type	All
Site	All
Thru Distance	
Gazette Direction	TG
Traffic Class	00 - All Vehicles
Data Range (inclusive)	Start Date: Saturday 09-Feb-2019 — End Date: Thursday 28-Feb-2019

Select Time Window: 24-Hour

Days in Range: 20 — Days Included: 20

	Daily Average Volume	Proportion
12-hour flow	474.0	74.78%
16-hour flow	548.9	86.27%
18-hour flow	554.5	87.47%
24-hour flow	634.0	100.00%

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%
01-02	1	0.6%	1	0.6%	1	0.6%	0	0.0%	3	0.4%
02-03	1	0.6%	1	0.6%	1	0.6%	1	0.6%	4	0.6%
03-04	1	0.6%	1	0.6%	2	1.2%	2	1.2%	6	0.9%
04-05	2	1.2%	2	1.1%	4	2.4%	7	4.3%	15	2.2%
05-06	9	5.5%	19	10.9%	18	10.8%	12	7.4%	58	8.7%
06-07	14	8.5%	7	4.0%	10	6.0%	9	5.6%	40	6.0%
07-08	10	6.1%	11	6.3%	9	5.4%	9	5.6%	39	5.8%
08-09	9	5.5%	13	7.5%	12	7.2%	13	8.0%	47	7.0%
09-10	10	6.1%	11	6.3%	11	6.6%	11	6.8%	43	6.4%
10-11	10	6.1%	9	5.2%	9	5.4%	10	6.2%	38	5.7%
11-12	9	5.5%	10	5.7%	8	4.8%	10	6.2%	37	5.5%
12-13	9	5.5%	9	5.2%	9	5.4%	9	5.6%	36	5.4%
13-14	9	5.5%	11	6.3%	10	6.0%	10	6.2%	40	6.0%
14-15	11	6.7%	10	5.7%	11	6.6%	11	6.8%	43	6.4%
15-16	11	6.7%	12	6.9%	12	7.2%	10	6.2%	45	6.7%
16-17	9	5.5%	12	6.9%	11	6.6%	12	7.4%	44	6.6%
17-18	14	8.5%	15	8.6%	10	6.0%	9	5.6%	48	7.2%
18-19	10	6.1%	6	3.4%	6	3.6%	6	3.7%	28	4.2%
19-20	6	3.6%	5	2.9%	4	2.4%	3	1.9%	18	2.7%
20-21	3	1.8%	3	1.7%	3	1.8%	3	1.9%	12	1.8%
21-22	2	1.2%	2	1.1%	2	1.2%	2	1.2%	8	1.2%
22-23	2	1.2%	1	0.6%	1	0.6%	1	0.6%	5	0.7%
23-00	2	1.2%	2	1.1%	1	0.6%	1	0.6%	6	0.9%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on this reset icon to reset all filters to default settings

Region	Central Queensland	All
District	404 - Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	472 - BILOELA - CALLIDE ROAD	All
Road Section Id	All	
Site Type		All
Site		60..
Thru Distance		
Gazette Direction	TA - Thru traffic - against gazettal	TA
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Thursday 10-Feb-2022	End Date: Tuesday 08-Mar-2022

Select Time Window: 24 Hour

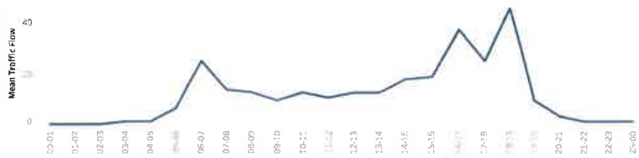
Days In Range: 27 | Days Included:

	Daily Average Volume	Proportion
12-hour flow	226.3	80.00%
16-hour flow	267.4	94.64%
18-hour flow	270.1	95.60%
24-hour flow	282.6	100.00%

© OpenStreetMap

Site Type: C

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	1	1.0%	1	1.5%	0	0.0%	0	0.0%	2	0.6%
01-02	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%
02-03	0	0.0%	1	1.5%	0	0.0%	1	1.4%	2	0.6%
03-04	1	1.0%	0	0.0%	1	1.3%	1	1.4%	3	1.0%
04-05	1	1.0%	1	1.5%	1	1.3%	1	1.4%	4	1.3%
05-06	2	2.0%	1	1.5%	2	2.6%	3	4.2%	8	2.5%
06-07	9	9.0%	6	8.8%	5	6.6%	6	8.5%	26	8.3%
07-08	7	7.0%	3	4.4%	3	3.9%	3	4.2%	16	5.1%
08-09	4	4.0%	3	4.4%	4	5.3%	4	5.6%	15	4.8%
09-10	2	2.0%	3	4.4%	3	3.9%	3	4.2%	11	3.5%
10-11	4	4.0%	3	4.4%	3	3.9%	4	5.6%	14	4.4%
11-12	3	3.0%	3	4.4%	4	5.3%	3	4.2%	13	4.1%
12-13	5	5.0%	4	5.9%	3	3.9%	3	4.2%	15	4.8%
13-14	3	3.0%	4	5.9%	4	5.3%	3	4.2%	14	4.4%
14-15	6	6.0%	5	7.4%	4	5.3%	4	5.6%	19	6.0%
15-16	4	4.0%	4	5.9%	6	7.9%	5	7.0%	19	6.0%
16-17	6	6.0%	6	8.8%	16	21.1%	9	12.7%	37	11.7%
17-18	7	7.0%	6	8.8%	6	7.9%	7	9.9%	26	8.3%
18-19	25	25.0%	8	11.8%	6	7.9%	7	9.9%	46	14.6%
19-20	6	6.0%	2	2.9%	1	1.3%	1	1.4%	10	3.2%
20-21	2	2.0%	1	1.5%	1	1.3%	1	1.4%	5	1.6%
21-22	1	1.0%	1	1.5%	1	1.3%	1	1.4%	4	1.3%
22-23	1	1.0%	1	1.5%	1	1.3%	0	0.0%	3	1.0%
23-00	0	0.0%	1	1.5%	1	1.3%	1	1.4%	3	1.0%



DAILY VOLUME REPORT

Tips and Tricks for a Faster Dashboard

Daily Volume Report Overview

Click on the reset icon to reset all filters to default settings

Region	Central Queensland	All
District	[illegible] Fitzroy	All
Local Government	Banana Shire	All
Road Section Name	[illegible]	All
Road Section Id	Alt	
Site Type	C - Coverage	All
Site	6012d - 075m b/t Dawson Hwy, Baker[illegible]	All
Thru Distance	0.6K	
Gazette Direction	TG - Thru traffic in gazetted dirn	TG
Traffic Class	00 - All Vehicles	
Date Range (inclusive)	Start Date: Thursday 10-Feb-2022	End Date: Tuesday 05-Mar-2022

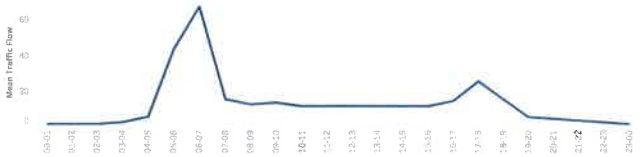
Site Type: C

Select Time Window: 24-Hour

Days In Range: [illegible] Days Included: 27

	Daily Average Volume	Proportion
12-hour flow	157.0	64.55%
16-hour flow	236.6	81.51%
18-hour flow	237.2	82.40%
24-hour flow	247.9	100.00%

	00 min - 15 min		15 min - 30 min		30 min - 45 min		45 min - 60 min		Grand Total	
	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %	Avg Volume	Volume %
00-01	0	0.0%	0	0.0%	1	1.1%	1	1.3%	2	0.6%
01-02	0	0.0%	1	1.3%	0	0.0%	0	0.0%	1	0.3%
02-03	0	0.0%	0	0.0%	1	1.1%	1	1.3%	2	0.6%
03-04	1	1.4%	0	0.0%	1	1.1%	1	1.3%	3	0.9%
04-05	1	1.4%	1	1.3%	1	1.1%	2	2.6%	5	1.6%
05-06	4	5.7%	9	11.4%	17	18.3%	13	17.1%	43	13.5%
06-07	14	20.0%	18	22.8%	23	24.7%	14	18.4%	69	21.7%
07-08	4	5.7%	5	6.3%	4	4.3%	3	3.9%	16	5.0%
08-09	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
09-10	4	5.7%	4	5.1%	4	4.3%	3	3.9%	15	4.7%
10-11	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
11-12	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
12-13	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
13-14	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
14-15	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
15-16	3	4.3%	3	3.8%	3	3.2%	3	3.9%	12	3.8%
16-17	3	4.3%	3	3.8%	4	4.3%	5	6.6%	15	4.7%
17-18	5	7.1%	7	8.9%	9	9.7%	6	7.9%	27	8.5%
18-19	7	10.0%	5	6.3%	2	2.2%	2	2.6%	16	5.0%
19-20	2	2.9%	2	2.5%	1	1.1%	1	1.3%	6	1.9%
20-21	1	1.4%	1	1.3%	1	1.1%	1	1.3%	4	1.3%
21-22	1	1.4%	1	1.3%	1	1.1%	1	1.3%	4	1.3%
22-23	1	1.4%	0	0.0%	1	1.1%	1	1.3%	3	0.9%
23-00	1	1.4%	1	1.3%	1	1.1%	0	0.0%	3	0.9%



YEAR	ROAD_SECTION_ID	THROUGH_DISTANCE_START	THROUGH_DISTANCE_END	SITE	GAZETTAL_DIRECTION	AADT	PC_CLASS_0A	PC_CLASS_0B	PC_CLASS_1A	PC_CLASS_1B	PC_CLASS_1C	PC_CLASS_1D
2020	46A	101.008	113.73	60067	TA	627	73.43	26.57	73.43	14.94	5.31	6.32
2020	46A	101.008	113.73	60067	TB	1,226	76.29	23.71	76.29	11.96	4.9	6.85
2020	46A	101.008	113.73	60067	TG	599	79.3	20.7	79.3	8.84	4.46	7.4
2020	46A	113.73	116.835	61084	TA	789	84.74	15.26	84.74	9.12	3.16	2.98
2020	46A	113.73	116.835	61084	TB	1,568	77.59	22.41	77.59	15.77	3.59	3.05
2020	46A	113.73	116.835	61084	TG	779	70.36	29.64	70.36	22.48	4.04	3.12
2020	46A	116.835	119.758	160067	TA	2,890	88.78	11.22	88.78	9.14	1.27	0.81
2020	46A	116.835	119.758	160067	TB	5,798	90.42	9.58	90.42	7.52	1.23	0.83
2020	46A	116.835	119.758	160067	TG	2,908	91.9	8.1	91.9	6.05	1.2	0.85
2020	472	0	3.988	60126	TA	336	85.69	14.31	85.69	8.87	1.46	3.98
2020	472	0	3.988	60126	TB	669	80.61	19.39	80.61	13.55	1.71	4.13
2020	472	0	3.988	60126	TG	333	75.48	24.52	75.48	18.28	1.96	4.28
2020	472	3.988	11.88	61286	TA	544	80.38	19.62	80.38	16.21	1.13	2.28
2020	472	3.988	11.88	61286	TB	1,120	83.43	16.57	83.43	13.14	1.06	2.37
2020	472	3.988	11.88	61286	TG	576	86.28	13.72	86.28	10.26	1.01	2.45
2021	46A	113.73	116.835	61084	TA	700	80.39	19.61	80.39	12.64	3.38	3.59
2021	46A	113.73	116.835	61084	TB	1,402	74.79	25.21	74.79	18.07	3.77	3.37
2021	46A	113.73	116.835	61084	TG	702	69.18	30.82	69.18	23.49	4.18	3.15
2021	46A	116.835	119.758	160067	TA	2,878	87.27	12.73	87.27	10.47	1.41	0.85
2021	46A	116.835	119.758	160067	TB	6,195						
2021	46A	116.835	119.758	160067	TG	3,317	87.37	12.63	87.37	10.46	1.38	0.79
2021	472	0	3.988	60126	TA	336	85.69	14.31	85.69	8.87	1.46	3.98
2021	472	0	3.988	60126	TB	669	80.61	19.39	80.61	13.55	1.71	4.13
2021	472	0	3.988	60126	TG	333	75.48	24.52	75.48	18.28	1.96	4.28
2021	472	3.988	11.88	61286	TA	624	86.68	13.32	86.68	9.34	1.17	2.81
2021	472	3.988	11.88	61286	TB	1,264	84.31	15.69	84.31	11.28	1.31	3.1
2021	472	3.988	11.88	61286	TG	640	82.03	17.97	82.03	13.16	1.43	3.38

GROWTH_PC_1YR	GROWTH_PC_5YR	GROWTH_PC_10YR	COLLECTION_YEAR	PC_CLASS_2A	PC_CLASS_2B	PC_CLASS_2C	PC_CLASS_2D	PC_CLASS_2E	PC_CLASS_2F	PC_CLASS_2G	PC_CLASS_2H	PC_CLASS_2I
1.46	0.91	2.14	2020	70.12	3.31	12.48	1.73	0.73	0.38	0.79	0.59	3.55
0.49	0.89	1.87	2020	72.74	3.55	9.64	1.64	0.68	0.27	0.53	0.63	3.47
-0.5	0.92	1.61	2020	75.51	3.79	6.66	1.56	0.62	0.15	0.26	0.68	3.37
		-0.85	2020	81.21	3.53	7.43	1.16	0.53	0.23	0.41	0.46	2.06
		-0.85	2020	74.62	2.97	13.99	1.2	0.58	0.42	0.65	0.44	2.08
		-0.83	2020	67.95	2.41	20.63	1.23	0.62	0.62	0.89	0.42	2.11
			2020	86.77	2.01	7.94	0.99	0.21	0.17	0.48	0.15	0.47
			2020	88.35	2.07	6.5	0.8	0.22	0.15	0.46	0.13	0.49
			2020	89.78	2.12	5.21	0.62	0.22	0.14	0.44	0.11	0.51
-9.43	7.27	6.83	2020	83.6	2.09	7.18	1.24	0.45	0.08	0.06	0.08	1.24
-13.12	6.48	6.27	2020	78.56	2.05	11.85	1.25	0.45	0.19	0.12	0.07	1.33
-16.54	5.73	5.74	2020	73.47	2.01	16.57	1.26	0.45	0.29	0.18	0.07	1.42
-13.92	1	0.77	2020	79.06	1.32	15.22	0.6	0.39	0.28	0.09	0.05	0.71
-11.04	1.79	1	2020	81.95	1.48	12.15	0.57	0.42	0.17	0.08	0.04	0.77
-8.13	2.57	1.22	2020	84.65	1.63	9.26	0.55	0.45	0.08	0.06	0.04	0.83
-11.28	-7.05	-3.28	2021	75.91	4.48	10.71	1.18	0.75	0.37	0.68	0.32	2.01
-10.59	-5.76	-3.14	2021	71.05	3.74	16.02	1.24	0.81	0.58	0.94	0.3	1.95
-9.88	-4.24	-2.97	2021	66.17	3.01	21.31	1.31	0.87	0.79	1.21	0.28	1.9
-0.42			2021	85.15	2.12	9.67	0.54	0.26	0.22	0.55	0.11	0.53
6.85			2021									
14.06			2021	85.45	1.92	9.61	0.58	0.27	0.26	0.53	0.09	0.5
-9.43	7.27	6.83	2021	83.6	2.09	7.18	1.24	0.45	0.08	0.06	0.08	1.24
-13.12	6.48	6.27	2021	78.56	2.05	11.85	1.25	0.45	0.19	0.12	0.07	1.33
-16.54	5.73	5.74	2021	73.47	2.01	16.57	1.26	0.45	0.29	0.18	0.07	1.42
14.71	4.51	2.47	2021	85.41	1.27	7.87	1.11	0.36	0.09	0.07	0.01	1
12.86	4.87	2.55	2021	83.18	1.13	10.19	0.76	0.33	0.09	0.11	0.01	1.1
11.11	5.24	2.63	2021	81.03	1	12.44	0.42	0.3	0.09	0.15	0	1.19

PC_CLASS_2J	PC_CLASS_2K	PC_CLASS_2L	LATITUDE_START	LONGITUDE_START	LATITUDE_END	LONGITUDE_END	DESCRIPTION_START	DESCRIPTION_END
6.26	0.06	0	-24.245398	150.553723	-24.35562259	150.5354321	Dawson Hwy to Biloela @ Argoon T/O	Dawson(46A)/Callide(472)/Shepherdsons Rd
6.78	0.07	0	-24.245398	150.553723	-24.35562259	150.5354321	Dawson Hwy to Biloela @ Argoon T/O	Dawson(46A)/Callide(472)/Shepherdsons Rd
7.32	0.08	0	-24.245398	150.553723	-24.35562259	150.5354321	Dawson Hwy to Biloela @ Argoon T/O	Dawson(46A)/Callide(472)/Shepherdsons Rd
2.84	0.14	0	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
2.92	0.13	0	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
3.01	0.11	0	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
0.73	0.08	0	-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
0.77	0.06	0	-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
0.81	0.04	0	-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
3.94	0.04	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
4.1	0.03	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
4.26	0.02	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
2.27	0.01	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	
2.36	0.01	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	
2.44	0.01	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	
3.4	0.18	0.01	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
3.21	0.15	0.01	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
3.02	0.12	0.01	-24.35562259	150.5354321	-24.38308674	150.5296141	Dawson(46A)/Callide(472)/Shepherdsons Rd	Calvale Rd / Tognolini Baldwin Rd, Bilo
0.77	0.08	0	-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
			-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
0.74	0.05	0	-24.38308674	150.5296141	-24.40315218	150.5118787	Calvale Rd / Tognolini Baldwin Rd, Bilo	Dawson Hwy(46A/46B) / Burnett Hwy (41D)
3.94	0.04	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
4.1	0.03	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
4.26	0.02	0	-24.35562	150.535429	-24.36749223	150.5721537	Biloela-Callide Rd to Mine @ Dawson Hwy	Biloela-Callide Rd To Biloela @ Links Rd
2.78	0.03	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	
3.07	0.03	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	
3.35	0.03	0	-24.36749223	150.5721537	-24.338074	150.632114	Biloela-Callide Rd To Biloela @ Links Rd	

SITE	SITE_TYPE	DESCRIPTION	SPEED_LIMIT	ROAD_SECTION_ID	THROUGH_DISTANCE	LATITUDE	LONGITUDE	DATUM	ROAD_NAME
60067	Coverage	1,475m N of Forestry Rd, Mt Murchison	100	46A	105.086	-24.27884411	150.541122	GDA94	DAWSON HIGHWAY (GLADSTONE - BILOELA)
60126	Coverage	875m E of Dawson Hwy, Dakenba	100	472	0.875	-24.35783671	150.5436966	GDA94	BILOELA - CALLIDE ROAD
61084	Coverage	600m S Biolela Callide Rd, Mt Murchison	100	46A	114.325	-24.3607998	150.5341753	GDA94	DAWSON HIGHWAY (GLADSTONE - BILOELA)
61286	Coverage	275m E of Shorts Rd, Mount Murchison	100	472	4.265	-24.36763794	150.5748615	GDA94	BILOELA - CALLIDE ROAD
160067	Coverage	55m S of Bell St, Biloela	60	46A	119.265	-24.4002212	150.5154645	GDA94	DAWSON HIGHWAY (GLADSTONE - BILOELA)

LOCALITY	DISTRICT	DISTRICT_NAME	REGION	REGION_NAME	LGA_ID	LGA_NAME
Callide	404	Fitzroy	314	Central Queensland	370	Banana Shire
Mount Murchison	404	Fitzroy	314	Central Queensland	370	Banana Shire
Dakenba	404	Fitzroy	314	Central Queensland	370	Banana Shire
Mount Murchison	404	Fitzroy	314	Central Queensland	370	Banana Shire
Biloela	404	Fitzroy	314	Central Queensland	370	Banana Shire

NORTHERN CONSULTING engineers

APPENDIX E

Northern Consulting Engineers – Traffic Generation Spreadsheet and Intersection Warrants

Development - Particulars		
Construction Commencement	2023	year
Construction Duration	52	weeks
Construction Duration (Assumed 6 days/week)	312	days
Operations Commencement	2024	year
Operational Design Life	30	years
Operational Design Life	1560	weeks
Operational Design Life (Assumed 5 days/week)	7800	days

Construction Phase - Development Generated Traffic Volumes

Solar Array System				
Expected MWp	**200**			
	MWp per Container	Movements (One way only)	Weight per Container (kg)	Total Weight Moved
Modules	0.18	1,111	15,500	8,610,250
Inverter Stations	2.00	100	15,500	775,000
Fixing System	0.15	1,333	20,500	13,663,250
Switchgear	200.00	1	50,000	25,000
Power Transformer	200.00	1	50,000	25,000
Pile	0.13	1,600	20,500	16,400,000
Tube	0.13	1,600	19,500	15,600,000
Tracker	0.13	1,600	13,000	10,400,000
Balance of System	0.75	267	15,500	2,069,250
Earthworks / Road Materials	0.13	1,600	15,500	12,400,000
Total Heavy Vehicle Movements		9,213	Average Payload (kg)	8,875
Average Daily Heavy Vehicle Movements		30	HV/day	

Construction Labour (Vehicle trips per day one way)		
Daily on Site Labour	0.08	15
Daily Mini Bus Activity	0.03	6
Additional Daily Cars	0.03	5
Total Light Vehicle Movements (per day)		26

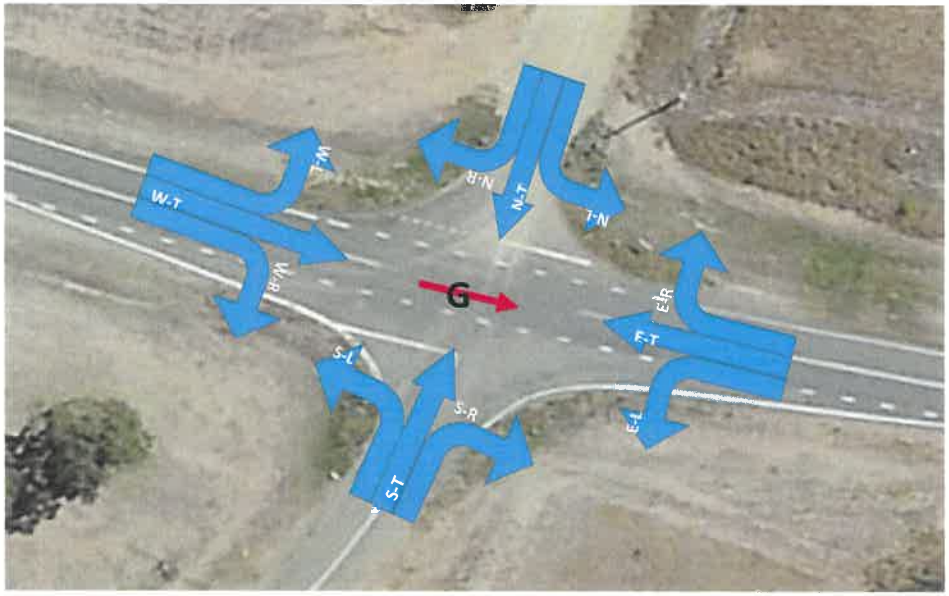
Operational Phase - Development Generated Traffic Volumes

Operational traffic Light Vehicles (per day one way)	
Management / Office	6.000
Electrical maintenance crew	1.000
Cleaning (Panel) crew (Annually - 20 veh per event)	0.077
Total Light Vehicle Movements (per day one way)	7.077

Operational traffic Heavy Vehicles (per day one way)	
Water Supply drinking (1 x monthly)	0.046
Water Supply Cleaning (Annually - 2 HV's per event)	0.008
Total Heavy Vehicle Movements (per day one way)	0.054

YEAR	ROAD_SECTION_ID	SITE	GAZETTAL_DIRECTION	AADT	PC_CLASS_1A	PC_CLASS_1B	PC_CLASS_1C	PC_CLASS_1D	GROWTH_PC_5YR	GROWTH_PC_10YR	COLLECTION_YEAR
2020	46A	60067	TA	627	73.43	14.94	5.31	6.32	0.91	2.14	2020
2020	46A	60067	TB	1,226	76.29	11.96	4.9	6.85	0.89	1.87	2020
2020	46A	60067	TG	599	79.3	8.84	4.46	7.4	0.92	1.61	2020
2020	46A	61084	TA	789	84.74	9.12	3.16	2.98		-0.85	2020
2020	46A	61084	TB	1,568	77.59	15.77	3.59	3.05		-0.85	2020
2020	46A	61084	TG	779	70.36	22.48	4.04	3.12		-0.83	2020
2020	46A	160067	TA	2,890	88.78	9.14	1.27	0.81			2020
2020	46A	160067	TB	5,798	90.42	7.52	1.23	0.83			2020
2020	46A	160067	TG	2,908	91.9	6.05	1.2	0.85			2020
2020	472	60126	TA	336	85.69	8.87	1.46	3.98	7.27	6.83	2020
2020	472	60126	TB	669	80.61	13.55	1.71	4.13	6.48	6.27	2020
2020	472	60126	TG	333	75.48	18.28	1.96	4.28	5.73	5.74	2020
2020	472	61286	TA	544	80.38	16.21	1.13	2.28	1	0.77	2020
2020	472	61286	TB	1,120	83.43	13.14	1.06	2.37	1.79	1	2020
2020	472	61286	TG	576	86.28	10.26	1.01	2.45	2.57	1.22	2020
2021	46A	61084	TA	700	80.39	12.64	3.38	3.59	-7.05	-3.28	2021
2021	46A	61084	TB	1,402	74.79	18.07	3.77	3.37	-5.76	-3.14	2021
2021	46A	61084	TG	702	69.18	23.49	4.18	3.15	-4.24	-2.97	2021
2021	46A	160067	TA	2,878	87.27	10.47	1.41	0.85			2021
2021	46A	160067	TB	6,195							2021
2021	46A	160067	TG	3,317	87.37	10.46	1.38	0.79			2021
2021	472	60126	TA	336	85.69	8.87	1.46	3.98	7.27	6.83	2021
2021	472	60126	TB	669	80.61	13.55	1.71	4.13	6.48	6.27	2021
2021	472	60126	TG	333	75.48	18.28	1.96	4.28	5.73	5.74	2021
2021	472	61286	TA	624	86.68	9.34	1.17	2.81	4.51	2.47	2021
2021	472	61286	TB	1,264	84.31	11.28	1.31	3.1	4.87	2.55	2021
2021	472	61286	TG	640	82.03	13.16	1.43	3.38	5.24	2.63	2021

Sites 60126 and 61286 are to be used in conjunction to calculate the traffic at the Biloela Callide Road / Shorts Road / Linkes Road intersection



East Approach = ID 61286
West Approach = ID 60126

Site ID	Gazettal / AM/PM	Approach Direction	Volume	Growth %	Year Recorded	Class 1A	Class 1B	Class 1C	Class 1D
60126	G / AM	W	69	5.74%	2021	75.48	18.28	1.96	4.28
60126	G / PM	W	27	5.74%	2021	75.48	18.28	1.96	4.28
60126	G / PM 2	W	16	5.74%	2021	75.48	18.28	1.96	4.28
60126	AG / AM	W	26	6.83%	2021	85.69	8.87	1.46	3.98
60126	AG / PM	W	26	6.83%	2021	85.69	8.87	1.46	3.98
60126	AG / PM 2	W	46	6.83%	2021	85.69	8.87	1.46	3.98
61286	G / AM	E	171	2.63%	2021	82.03	13.16	1.43	3.38
61286	G / PM	E	61	2.63%	2021	82.03	13.16	1.43	3.38
61286	G / PM 2	E	40	2.63%	2021	82.03	13.16	1.43	3.38
61286	AG / AM	E	72	2.47%	2021	86.68	9.34	1.17	2.81
61286	AG / PM	E	75	2.47%	2021	86.68	9.34	1.17	2.81
61286	AG / PM 2	E	158	2.47%	2021	86.68	9.34	1.17	2.81

Approach ID	Approach Road	Turn Movement	Peak AM Traffic	Peak PM Traffic	Peak PM 2 Traffic
W-T	Biloela Callide Road	West Approach - Through	26	26	46
W-L	Biloela Callide Road	West Approach - Left	0	0	0
W-R	Biloela Callide Road	West Approach - Right	0	0	0
N-T	Shorts Road	North Approach - Through	0	0	0
N-L	Shorts Road	North Approach - Left	0	0	0
N-R	Shorts Road	North Approach - Right	0	0	0
E-T	Biloela Callide Road	East Approach - Through	69	27	16
E-L	Biloela Callide Road	East Approach - Left	46	49	112
E-R	Biloela Callide Road	East Approach - Right	0	0	0
S-T	Linkes Road	South Approach - Through	0	0	0
S-L	Linkes Road	South Approach - Left	0	0	0
S-R	Linkes Road	South Approach - Right	102	34	24

BACKGROUND PEAKS

Description of entity		SID 60126
Linear Growth Equation **A = rt+P**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2024*
AADT (G) [Traffic Flow in Gazettal Direction]		*69*
AADT (A) [Traffic Flow Against Gazettal Direction]		*26*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**80.9**
(A)	Future value including growth rate	**31.3**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percent)	5.74%
r	Annual growth rate AG (percent)	6.83%

Description of entity		SID 60126
Continuos Compound Growth Equation **A = P .e^{rt}**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2024*
AADT (G) [Traffic Flow in Gazettal Direction]		*69*
AADT (A) [Traffic Flow Against Gazettal Direction]		*26*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**82.0**
(A)	Future value including growth rate	**31.9**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percent)	5.74%
r	Annual growth rate AG (percent)	6.83%
e	Continous Growth	exp
t	Number of year projected.	3.0

Description of entity		SID 60126
Linear Growth Equation **A = rt+P**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2024*
AADT (G) [Traffic Flow in Gazettal Direction]		*16*
AADT (A) [Traffic Flow Against Gazettal Direction]		*46*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**18.8**
(A)	Future value including growth rate	**55.4**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percentl)	5.74%
r	Annual growth rate AG (percent)	6.83%

Description of entity		SID 60126
Continuos Compound Growth Equation **A = P .e^{Art}**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2024*
AADT (G) [Traffic Flow in Gazettal Direction]		*16*
AADT (A) [Traffic Flow Against Gazettal Direction]		*46*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**19.0**
(A)	Future value including growth rate	**56.5**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percent)	5.74%
r	Annual growth rate AG (percent)	6.83%
e	Continous Growth	exp
t	Number of year projected.	3.0

Description of entity		SID 60126
Linear Growth Equation A = rt+P		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2034*
AADT (G) [Traffic Flow in Gazettal Direction]		*69*
AADT (A) [Traffic Flow Against Gazettal Direction]		*26*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**120.5**
(A)	Future value including growth rate	**49.1**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percentl)	5.74%
r	Annual growth rate AG (percent)	6.83%

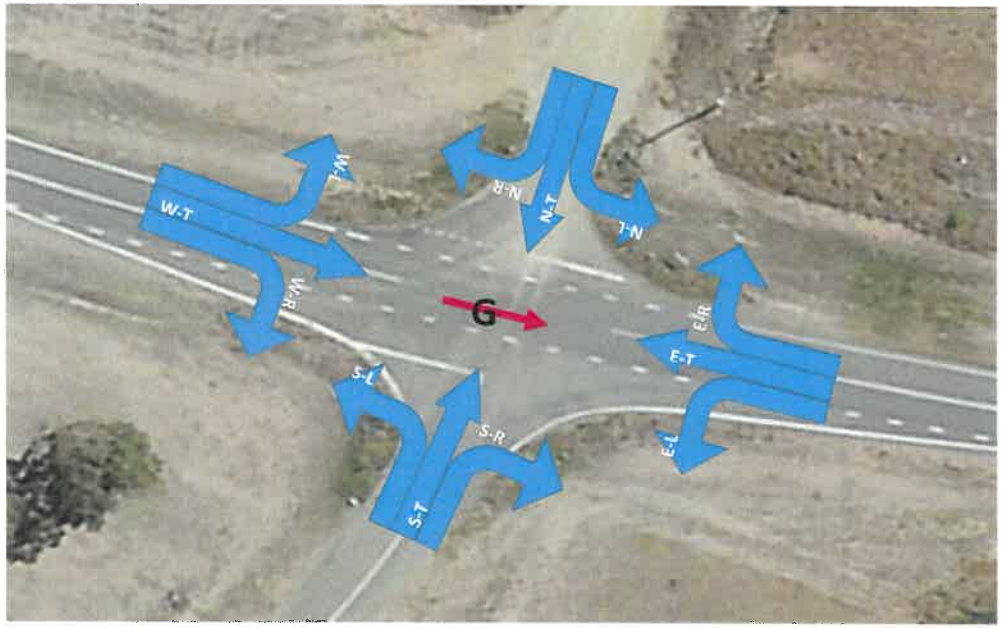
Description of entity		SID 60126
Continuos Compound Growth Equation $A = P.e^{rt}$		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2034*
AADT (G) [Traffic Flow in Gazettal Direction]		*69*
AADT (A) [Traffic Flow Against Gazettal Direction]		*26*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**145.5**
(A)	Future value including growth rate	**63.2**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percent)	5.74%
r	Annual growth rate AG (percent)	6.83%
e	Continous Growth	exp
t	Number of year projected.	13.0

Description of entity		SID 60126
Linear Growth Equation **A = rt+P**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2034*
AADT (G) [Traffic Flow in Gazettal Direction]		*16*
AADT (A) [Traffic Flow Against Gazettal Direction]		*46*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**27.9**
(A)	Future value including growth rate	**86.8**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percentl)	5.74%
r	Annual growth rate AG (percent)	6.83%

Description of entity		SID 60126
Continuos Compound Growth Equation **A = P .e^{rt}**		
Year - Traffic Survey Data Collected		2021
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2034*
AADT (G) [Traffic Flow in Gazettal Direction]		*16*
AADT (A) [Traffic Flow Against Gazettal Direction]		*46*
AADT (B) [Traffic Flow Both Directions]		-
(G)	Future value including growth rate	**33.7**
(A)	Future value including growth rate	**111.8**
(B)	Future value including growth rate	-
P	Initial value	(G), (A) or (B) above
r	Annual growth rate G (percent)	5.74%
r	Annual growth rate AG (percent)	6.83%
e	Continous Growth	exp
t	Number of year projected.	13.0

Description of entity		SID 60067
Linear Growth Equation **A = rt+P**		
Year - Traffic Survey Data Collected		2020
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2022*
AADT (B) [Traffic Flow Both Directions]		*1226*
(B)	Future value including growth rate	**1271.9**
P	Initial value	(G), (A) or (B) above
r	Annual growth rate B (percentl)	1.87%

Description of entity		SID 60067
Continuos Compound Growth Equation **A = P .e$^{\wedge rt}$**		
Year - Traffic Survey Data Collected		2020
Year - Commencement of Use		*2024*
Year - Projected Design Horizon		*2022*
AADT (B) [Traffic Flow Both Directions]		*1226*
(B)	Future value including growth rate	**1272.7**
P	Initial value	(G), (A) or (B) above
r	Annual growth rate B (percent)	1.87%
e	Continous Growth	exp
t	Number of year projected.	2.0



East Approach = ID 61286
West Approach = ID 60126

Site ID	Gazettal / AM/PM	Approach Direction	Volume 2024	Volume 2034	Year Recorded	Class 1A	Class 1B	Class 1C	Class 1D
60126	G / AM	W	81	121	2021	75.48	18.28	1.96	4.28
60126	G / PM 2	W	19	28	2021	75.48	18.28	1.96	4.28
60126	AG / AM	W	32	50	2021	85.69	8.87	1.46	3.98
60126	AG / PM 2	W	56	87	2021	85.69	8.87	1.46	3.98
61286	G / AM	E	185	230	2021	82.03	13.16	1.43	3.38
61286	G / PM 2	E	44	54	2021	82.03	13.16	1.43	3.38
61286	AG / AM	E	78	96	2021	86.68	9.34	1.17	2.81
61286	AG / PM 2	E	170	209	2021	86.68	9.34	1.17	2.81
						82.47	12.4125	1.505	3.6125

Approach ID	Approach Road	Turn Movement	Peak AM 2024 Traffic	Peak PM 2 2024 Traffic	Peak AM 2034	Peak PM 2 2034 Traffic
W-T	Biloela Callide Road	West Approach - Through	32	56	50	87
W-L	Biloela Callide Road	West Approach - Left	0	0	0	0
W-R	Biloela Callide Road	West Approach - Right	0	0	0	0
N-T	Shorts Road	North Approach - Through	0	0	0	0
N-L	Shorts Road	North Approach - Left	0	0	0	0
N-R	Shorts Road	North Approach - Right	0	0	0	0
E-T	Biloela Callide Road	East Approach - Through	81	19	121	28
E-L	Biloela Callide Road	East Approach - Left	46	114	46	122
E-R	Biloela Callide Road	East Approach - Right	0	0	0	0
S-T	Linkes Road	South Approach - Through	0	0	0	0
S-L	Linkes Road	South Approach - Left	0	0	0	0
S-R	Linkes Road	South Approach - Right	104	25	109	26

BACKGROUND PEAKS 2024-2034

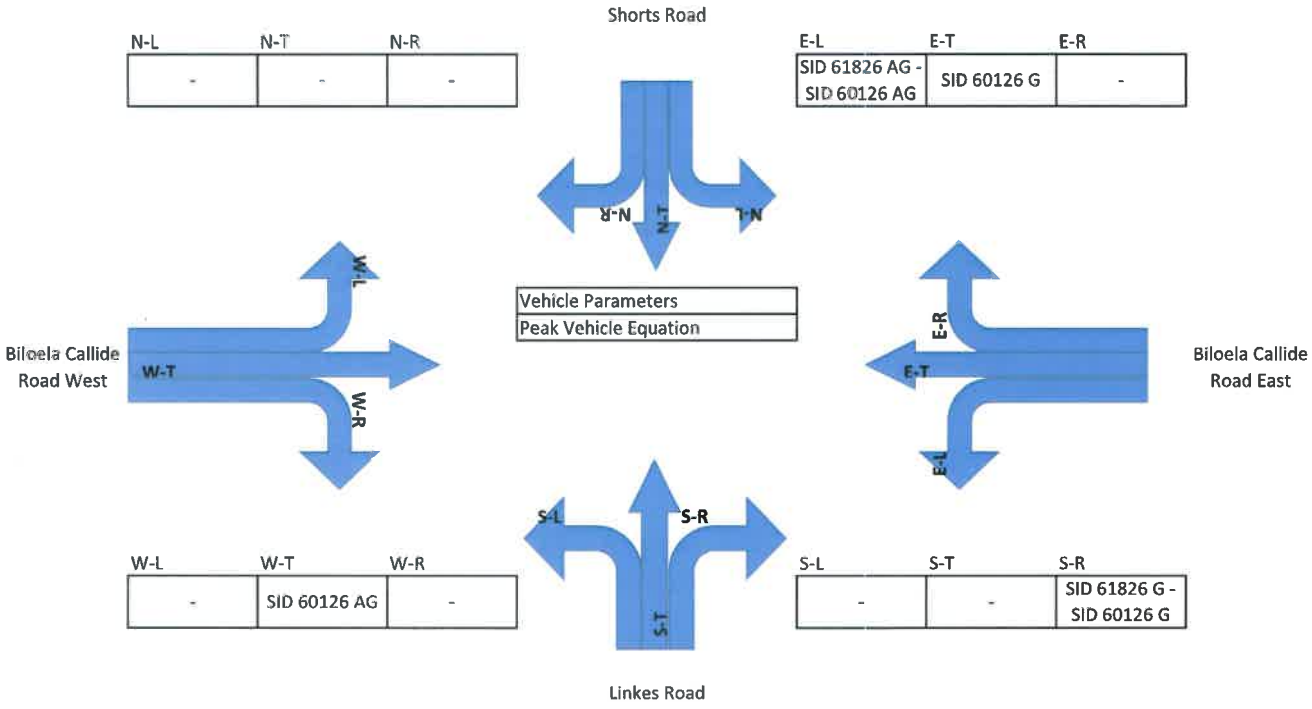
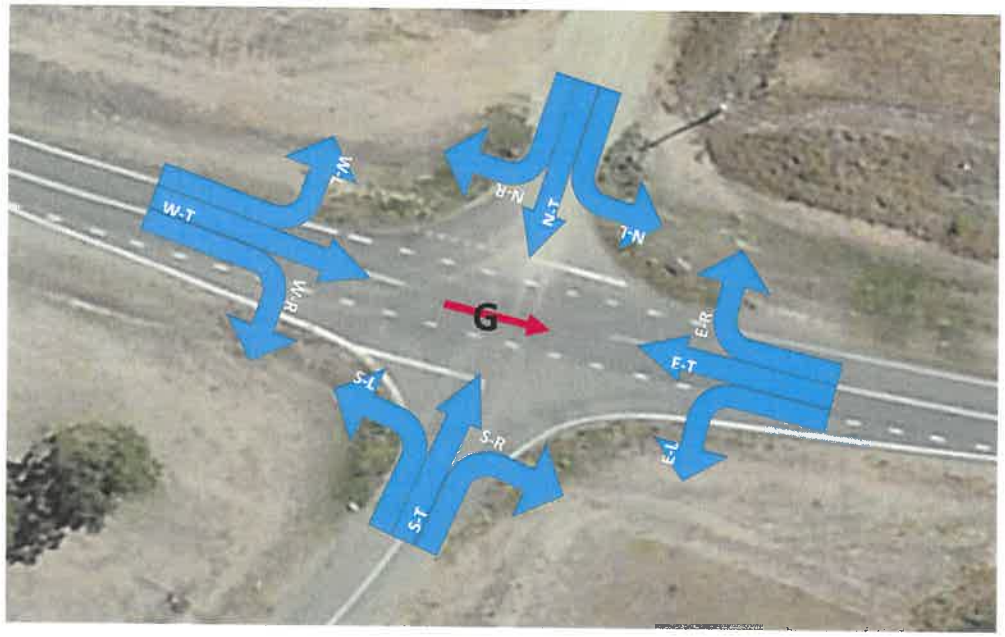

Shorts Road
N-L
N-T
N-R
-
-
-
E-L
E-T
E-R
SID 61826 AG - SID 60126 AG
SID 60126 G
-
N-R
N-T
N-L
Vehicle Parameters
Peak Vehicle Equation
W-L
W-T
W-R
Biloela Callide Road West
E-R
E-T
E-L
Biloela Callide Road East
S-L
S-R
S-T
W-L
W-T
W-R
-
SID 60126 AG
-
S-L
S-T
S-R
-
-
SID 61826 G - SID 60126 G
Linkes Road

Figure 2-4



East Approach = ID 61286
West Approach = ID 60126

Construction Dev traffic to be applied to 2024 year. Heavy vehicles are to be 100% W-L movements and 100% N-R movements. Light vehicles are to be 100% S-T in the AM and 100% N-T in the PM.

Operational Dev traffic to be applied to the 2034 year. Heavy vehicles are to be 100% W-L movements and 100% N-R movements. Light vehicles are to be 100% S-T in the AM and 100% N-T in the PM.

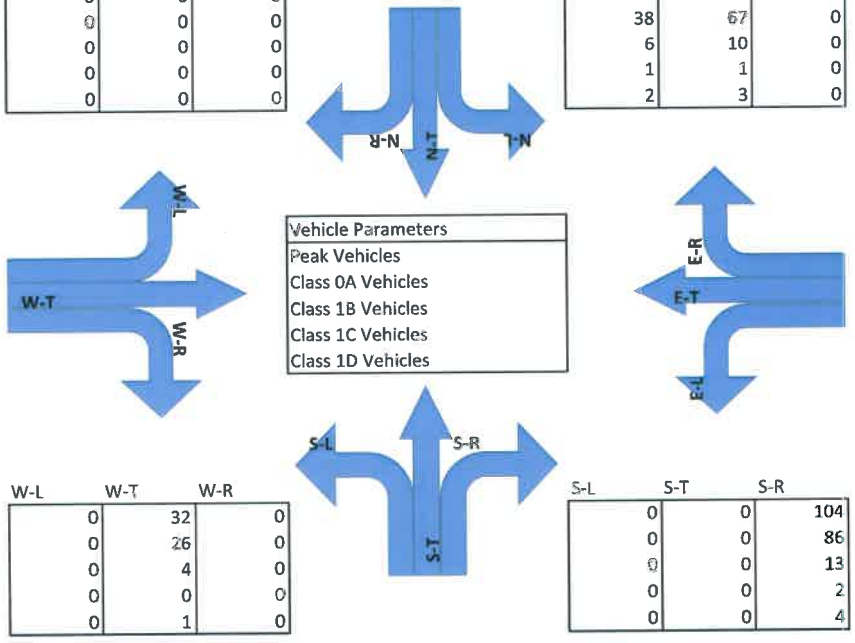
Site ID	Gazettal / AM/PM	Approach Direction	Volume 2024	Volume 2034	Year Recorded	Class 1A	Class 1B	Class 1C	Class 1D
60126	G / AM	W	81	121	2021	75.48	18.28	1.96	4.28
60126	G / PM 2	W	19	28	2021	75.48	18.28	1.96	4.28
60126	AG / AM	W	32	50	2021	85.69	8.87	1.46	3.98
60126	AG / PM 2	W	56	87	2021	85.69	8.87	1.46	3.98
61286	G / AM	E	185	230	2021	82.03	13.16	1.43	3.38
61286	G / PM 2	E	44	54	2021	82.03	13.16	1.43	3.38
61286	AG / AM	E	78	96	2021	86.68	9.34	1.17	2.81
61286	AG / PM 2	E	170	209	2021	86.68	9.34	1.17	2.81
						82.47	12.4125	1.505	3.6125

Approach ID	Approach Road	Turn Movement	Peak AM 2024 Traffic	Peak PM 2 2024 Traffic	Peak AM 2034	Peak PM 2 2034 Traffic
W-T	Biloela Callide Road	West Approach - Through	32	56	50	87
W-L	Biloela Callide Road	West Approach - Left	30	30	1	1
W-R	Biloela Callide Road	West Approach - Right	0	0	0	0
N-T	Shorts Road	North Approach - Through	0	26	0	7
N-L	Shorts Road	North Approach - Left	0	0	0	0
N-R	Shorts Road	North Approach - Right	30	30	1	1
E-T	Biloela Callide Road	East Approach - Through	81	19	121	28
E-L	Biloela Callide Road	East Approach - Left	46	114	46	122
E-R	Biloela Callide Road	East Approach - Right	0	0	0	0
S-T	Linkes Road	South Approach - Through	26	0	7	0
S-L	Linkes Road	South Approach - Left	0	0	0	0
S-R	Linkes Road	South Approach - Right	104	25	109	26

DEV PEAKS 2024-2034

N-L	N-T	N-R
0	0	0
0	0	0
0	0	0
0	0	0
0	0	0

E-L	E-T	E-R
46	81	0
38	67	0
6	10	0
1	1	0
2	3	0



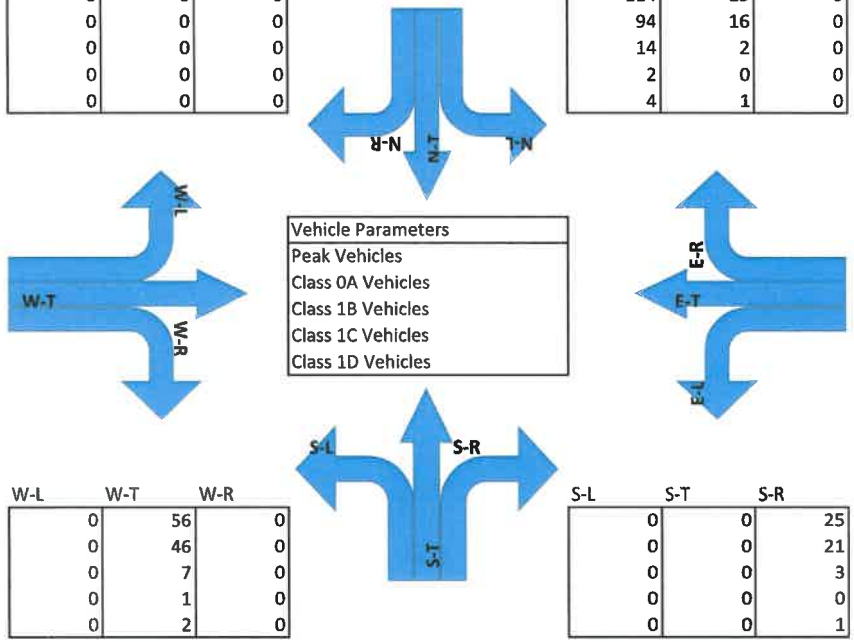
Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

W-L	W-T	W-R
0	32	0
0	26	0
0	4	0
0	0	0
0	1	0

S-L	S-T	S-R
0	0	104
0	0	86
0	0	13
0	0	2
0	0	4

N-L	N-T	N-R
0	0	0
0	0	0
0	0	0
0	0	0
0	0	0

E-L	E-T	E-R
114	19	0
94	16	0
14	2	0
2	0	0
4	1	0



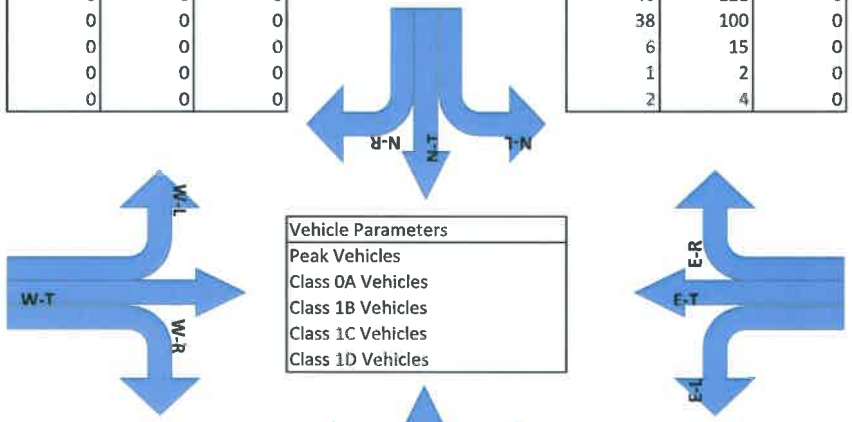
Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

W-L	W-T	W-R
0	56	0
0	46	0
0	7	0
0	1	0
0	2	0

S-L	S-T	S-R
0	0	25
0	0	21
0	0	3
0	0	0
0	0	1

N-L	N-T	N-R
0	0	0
0	0	0
0	0	0
0	0	0
0	0	0

E-L	E-T	E-R
46	121	0
38	100	0
6	15	0
1	2	0
2	4	0



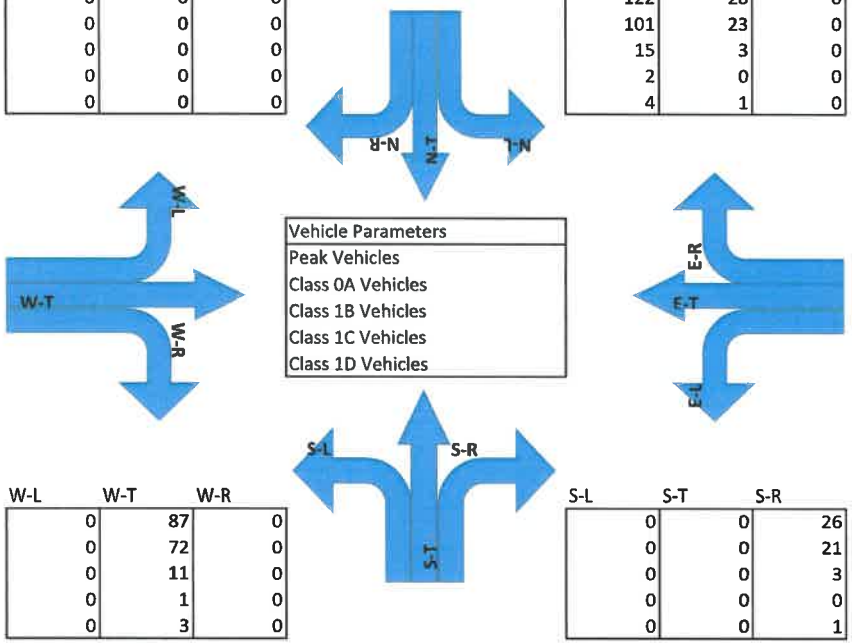
Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

W-L	W-T	W-R
0	50	0
0	41	0
0	6	0
0	1	0
0	2	0

S-L	S-T	S-R
0	0	109
0	0	90
0	0	14
0	0	2
0	0	4

N-L	N-T	N-R
0	0	0
0	0	0
0	0	0
0	0	0
0	0	0

E-L	E-T	E-R
122	28	0
101	23	0
15	3	0
2	0	0
4	1	0



Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

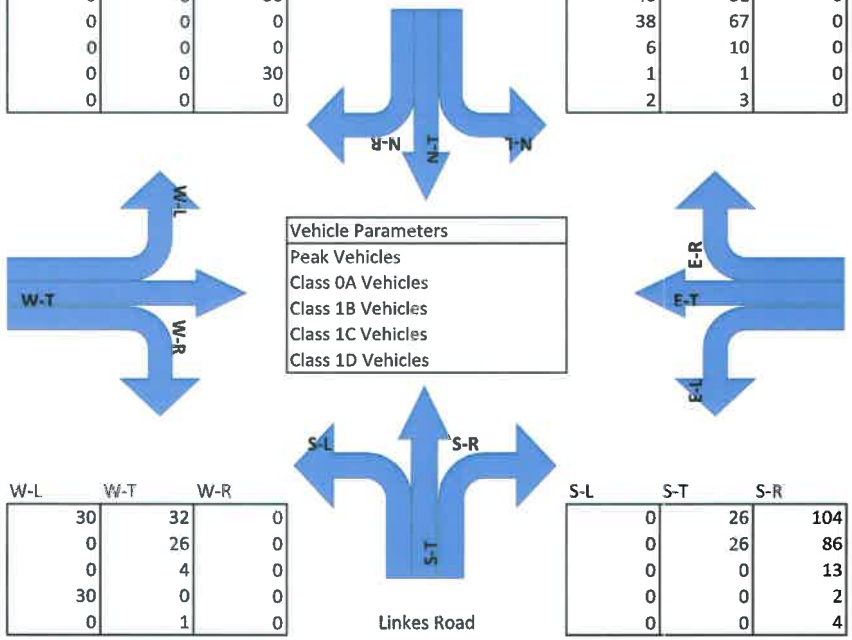
W-L	W-T	W-R
0	87	0
0	72	0
0	11	0
0	1	0
0	3	0

S-L	S-T	S-R
0	0	26
0	0	21
0	0	3
0	0	0
0	0	1

Shorts Road

N-L	N-T	N-R
0	0	30
0	0	0
0	0	0
0	0	30
0	0	0

E-L	E-T	E-R
46	81	0
38	67	0
6	10	0
1	1	0
2	3	0



Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

Biloela Callide Road West

Biloela Callide Road East

W-L	W-T	W-R
30	32	0
0	26	0
0	4	0
30	0	0
0	1	0

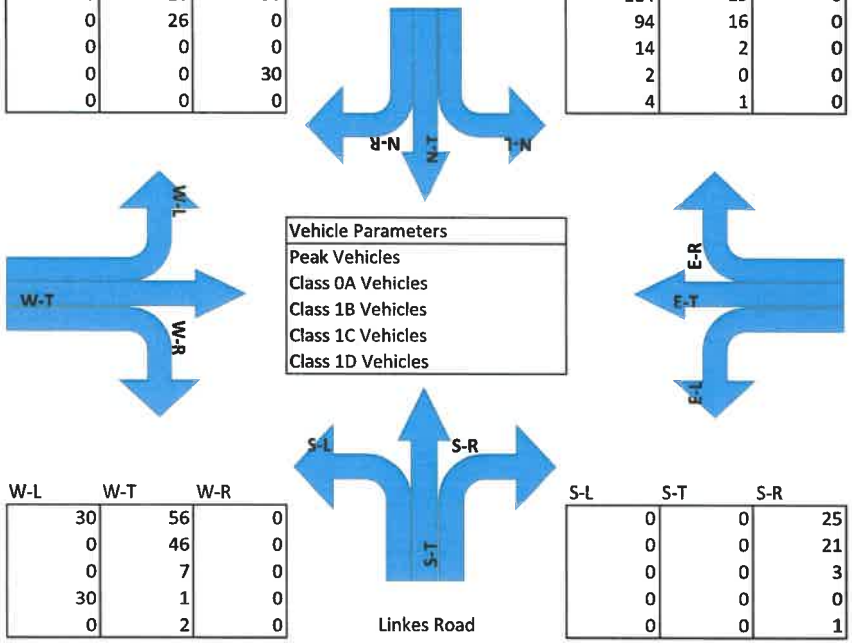
S-L	S-T	S-R
0	26	104
0	26	86
0	0	13
0	0	2
0	0	4

Linkes Road

N-L	N-T	N-R
0	26	30
0	26	0
0	0	0
0	0	30
0	0	0

Shorts Road

E-L	E-T	E-R
114	19	0
94	16	0
14	2	0
2	0	0
4	1	0



Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

Biloela Callide Road West

Biloela Callide Road East

W-L	W-T	W-R
30	56	0
0	46	0
0	7	0
30	1	0
0	2	0

Linkes Road

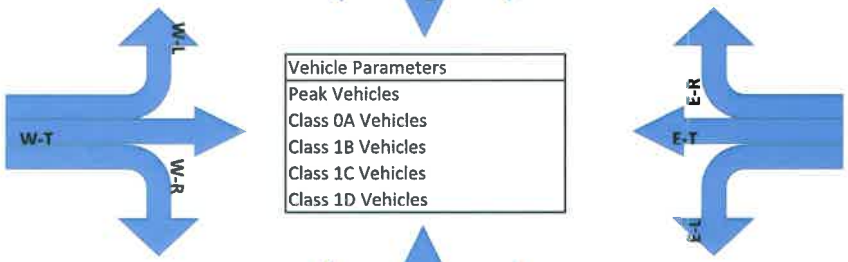
S-L	S-T	S-R
0	0	25
0	0	21
0	0	3
0	0	0
0	0	1

N-L	N-T	N-R
0	0	1
0	0	0
0	0	0
0	0	1
0	0	0

Shorts Road

E-L	E-T	E-R
46	121	0
38	100	0
6	15	0
1	2	0
2	4	0

Biloela Callide Road West



Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

Biloela Callide Road East

W-L	W-T	W-R
1	50	0
0	41	0
0	6	0
1	1	0
0	2	0

Linkes Road

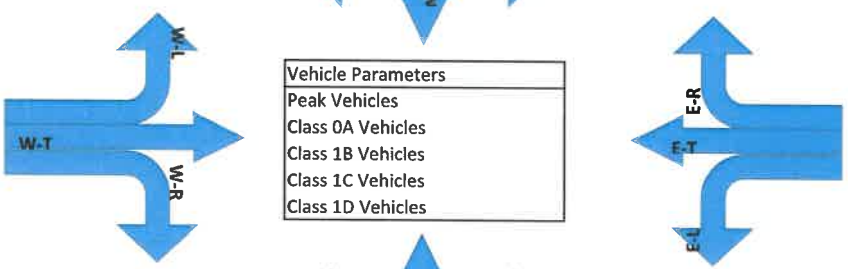
S-L	S-T	S-R
0	7	109
0	7	90
0	0	14
0	0	2
0	0	4

N-L	N-T	N-R
0	7	1
0	7	0
0	0	0
0	0	1
0	0	0

Shorts Road

E-L	E-T	E-R
122	28	0
101	23	0
15	3	0
2	0	0
4	1	0

Biloela Callide Road West



Vehicle Parameters
Peak Vehicles
Class 0A Vehicles
Class 1B Vehicles
Class 1C Vehicles
Class 1D Vehicles

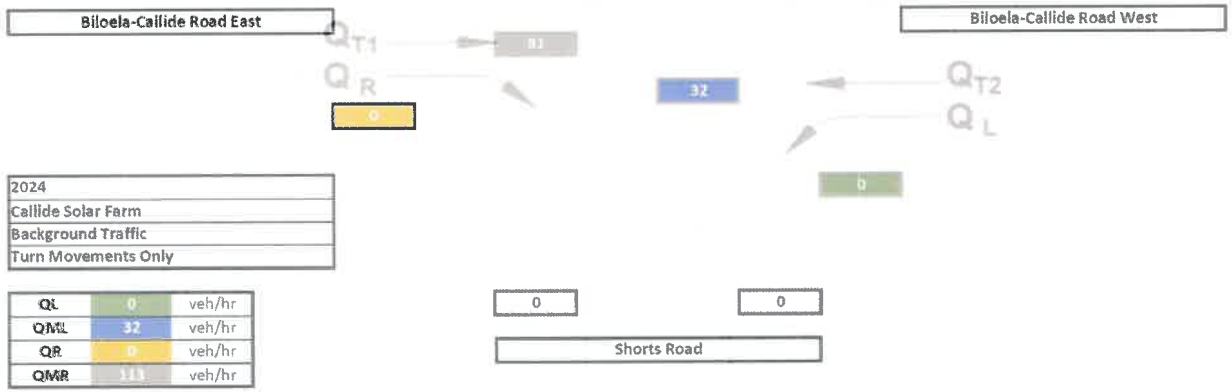
Biloela Callide Road East

W-L	W-T	W-R
1	87	0
0	72	0
0	11	0
1	1	0
0	3	0

Linkes Road

S-L	S-T	S-R
0	0	26
0	0	21
0	0	3
0	0	0
0	0	1

Figure 2.27: Calculation of the major road traffic volume Q_M



Road type	Turn type	Splitter island	Q_M (veh/h)
Two-lane two-way	Right	No	$= Q_{T1} + Q_{T2} + Q_L$
		Yes	$= Q_{T1} + Q_{T2}$
	Left	Yes or no	$= Q_{T2}$

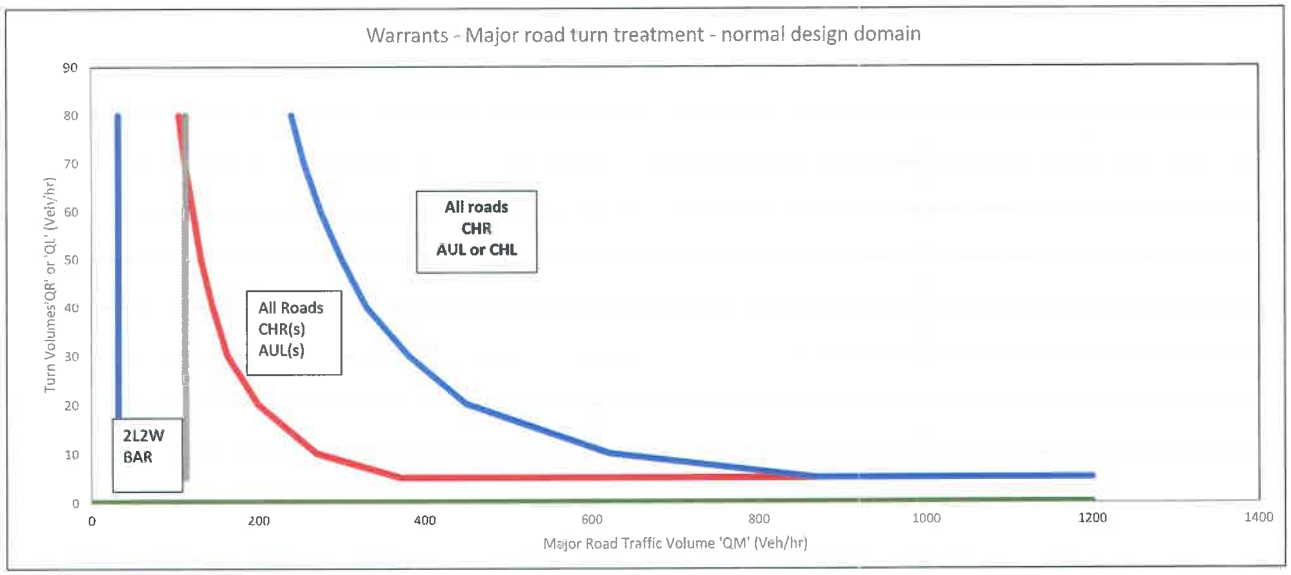
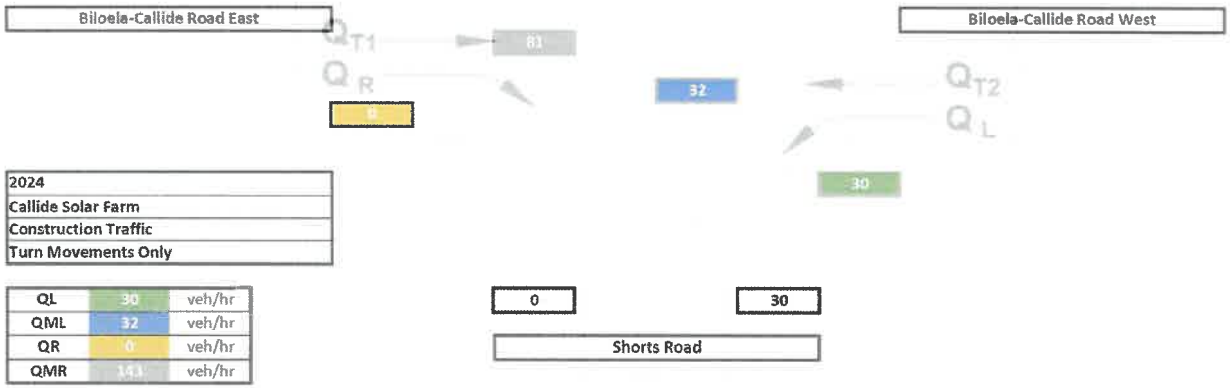


Fig 2.27 Background 2024 AM

Figure 2.27: Calculation of the major road traffic volume Q_M



Road type	Turn type	Splitter island	Q_M (veh/h)
Two-lane two-way	Right	No	$= Q_{T1} + Q_{T2} + Q_L$
		Yes	$= Q_{T1} + Q_{T2}$
	Left	Yes or no	$= Q_{T2}$

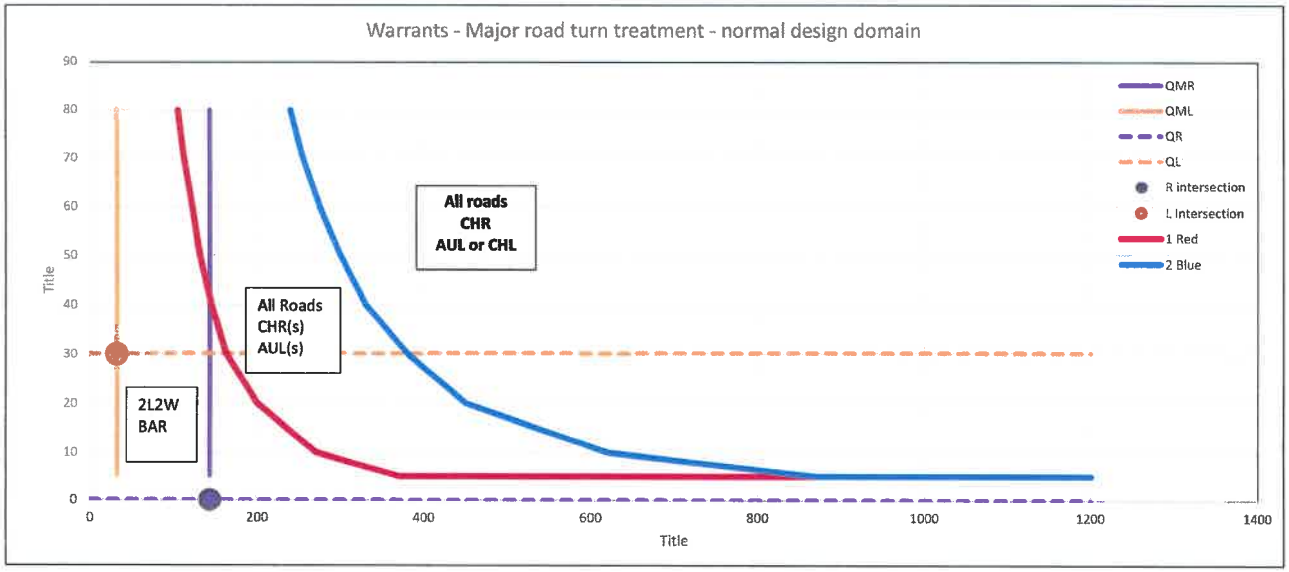
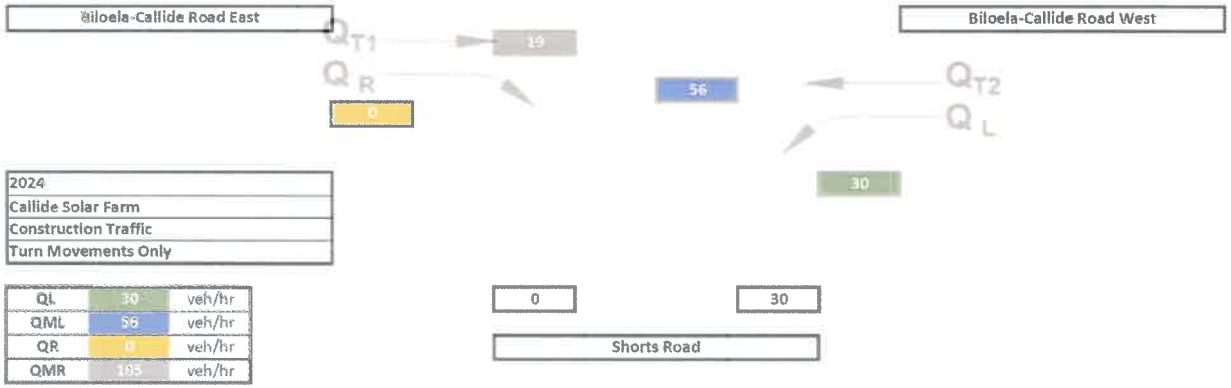


Fig 2.27 DEV 2024 AM

Figure 2.27: Calculation of the major road traffic volume Q_M



Road type	Turn type	Splitter island	Q_M (veh/h)
Two-lane two-way	Right	No	$= Q_{T1} + Q_{T2} + Q_L$
		Yes	$= Q_{T1} + Q_{T2}$
	Left	Yes or no	$= Q_{T2}$

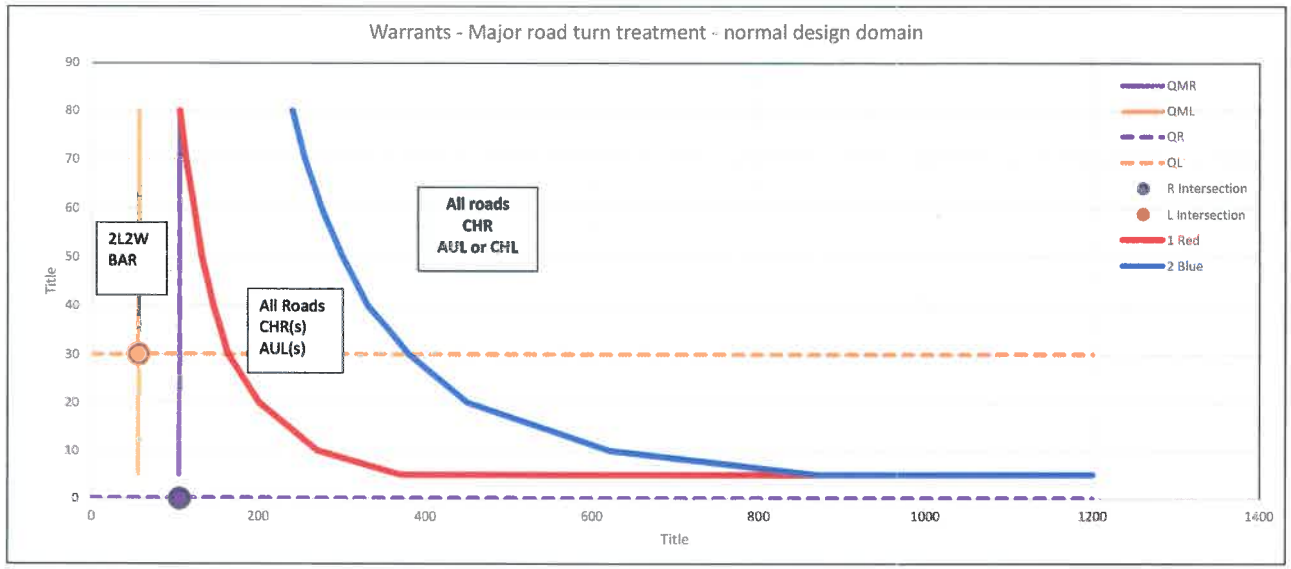
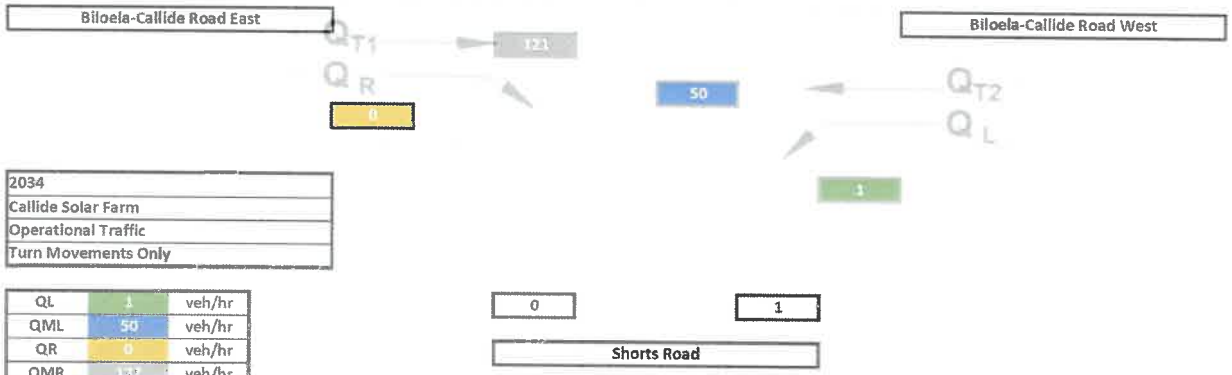


Fig 2.27 DEV 2024 PM

Figure 2.27: Calculation of the major road traffic volume Q_M



Road type	Turn type	Splitter island	Q_M (veh/h)
Two-lane two-way	Right	No	$= Q_{T1} + Q_{T2} + Q_L$
		Yes	$= Q_{T1} + Q_{T2}$
	Left	Yes or no	$= Q_{T2}$

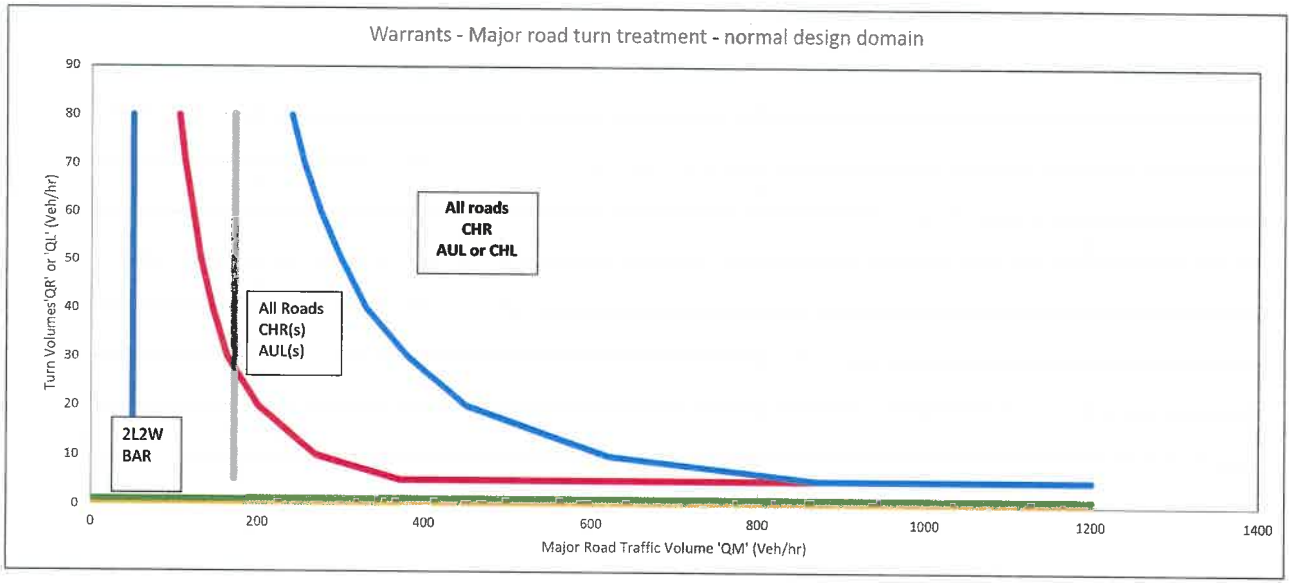
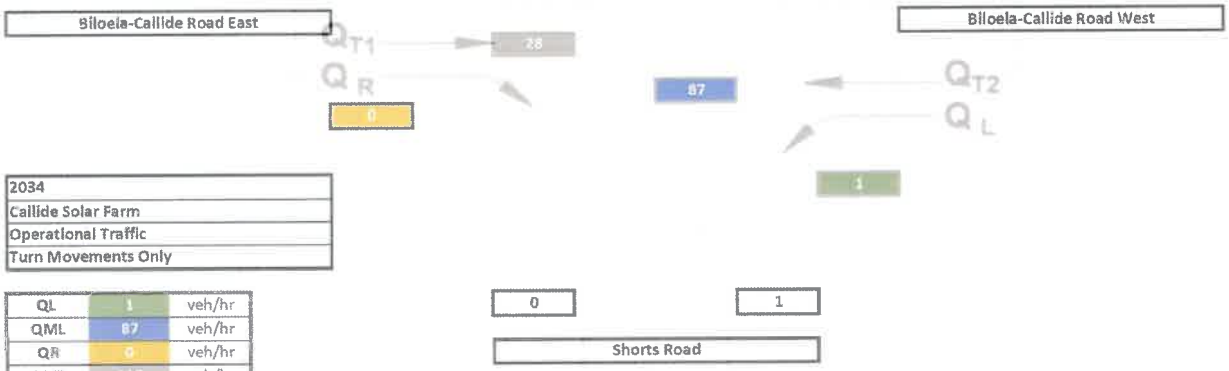


Fig 2.27 DEV 2034 AM

Figure 2.27: Calculation of the major road traffic volume Q_M



Road type	Turn type	Splitter island	Q_M (veh/h)
Two-lane two-way	Right	No	$= Q_{T1} + Q_{T2} + Q_L$
		Yes	$= Q_{T1} + Q_{T2}$
	Left	Yes or no	$= Q_{T2}$

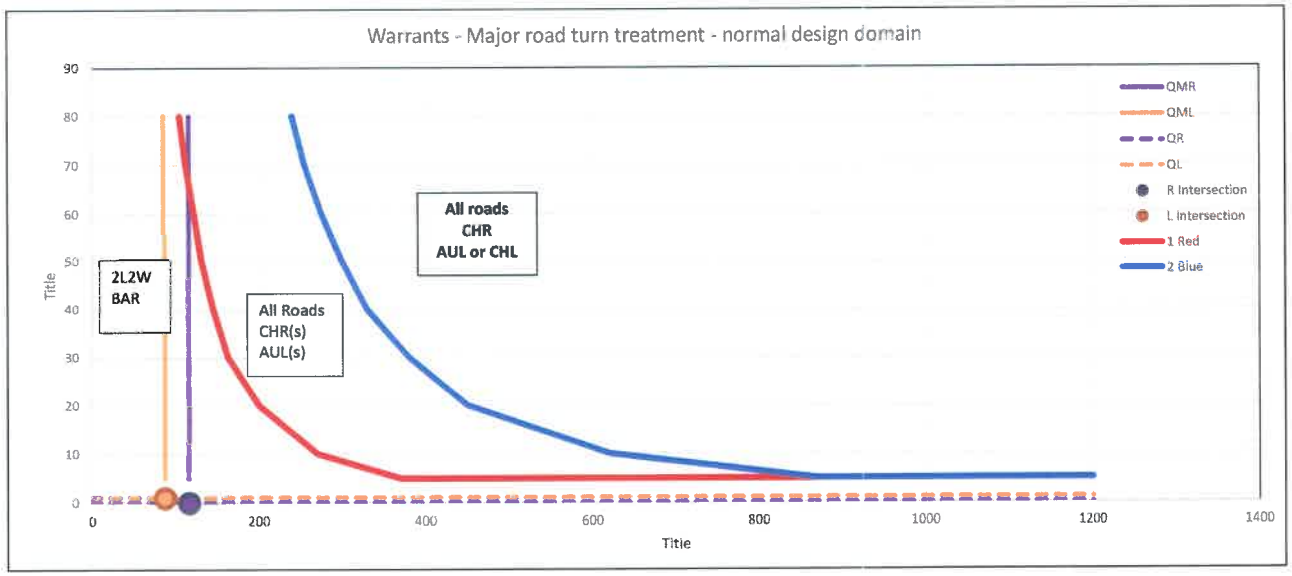


Fig 2.27 DEV 2034 PM

NORTHERN CONSULTING engineers

APPENDIX F

SIDRA Results Reports, Table and User Defined Traffic Data

USER REPORT FOR SITE

Project: 221116-MJ2370-Callide_Solar_Project

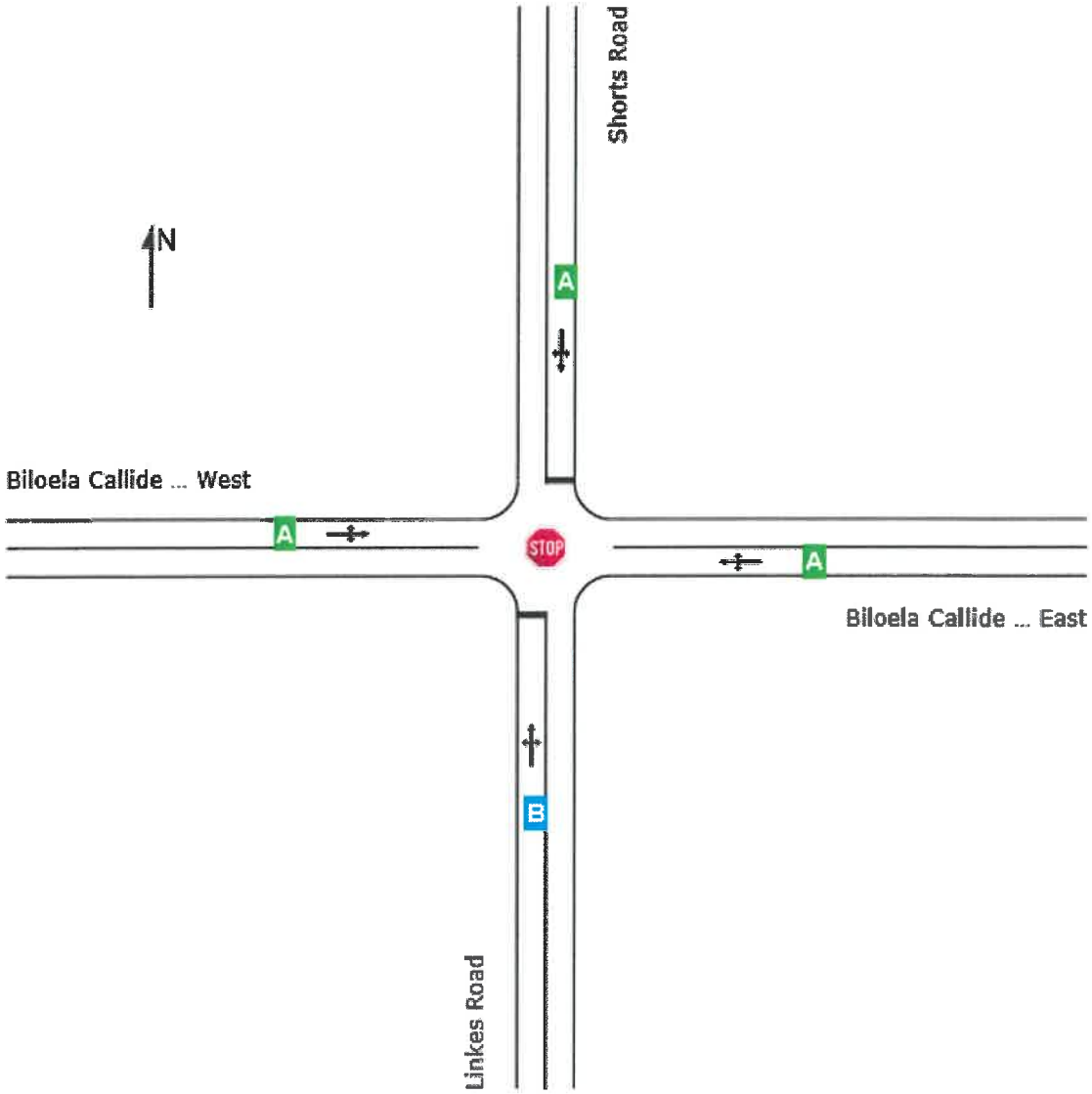
Output produced by SIDRA INTERSECTION Version: 9.1.1.200 Template: NCE Short

Site: 101 [Background 2024 AM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	A	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance												
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.161	10.0	LOS A	0.6	5.0	0.33	0.94	0.33	70.5
2	T1	All MCs	1	0.0	1	0.0	0.161	10.0	LOS A	0.6	5.0	0.33	0.94	0.33	70.5
3	R2	All MCs	111	18.1	111	18.1	0.161	12.0	LOS B	0.6	5.0	0.33	0.94	0.33	62.6
Approach			113	17.8	113	17.8	0.161	12.0	LOS B	0.6	5.0	0.33	0.94	0.33	62.7
East: Biloela Callide Road East															
4	L2	All MCs	46	13.6	46	13.6	0.086	8.3	LOS A	0.0	0.1	0.00	0.24	0.00	73.9
5	T1	All MCs	85	17.3	85	17.3	0.086	0.0	LOS A	0.0	0.1	0.00	0.24	0.00	92.6
6	R2	All MCs	1	0.0	1	0.0	0.086	7.4	LOS A	0.0	0.1	0.00	0.24	0.00	81.7
Approach			133	15.9	133	15.9	0.086	3.0	NA	0.0	0.1	0.00	0.24	0.00	85.0
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.003	9.6	LOS A	0.0	0.1	0.18	0.90	0.18	72.2
8	T1	All MCs	1	0.0	1	0.0	0.003	9.9	LOS A	0.0	0.1	0.18	0.90	0.18	72.1
9	R2	All MCs	1	0.0	1	0.0	0.003	9.6	LOS A	0.0	0.1	0.18	0.90	0.18	72.0
Approach			3	0.0	3	0.0	0.003	9.7	LOS A	0.0	0.1	0.18	0.90	0.18	72.1
West: Biloela Callide Road West															
10	L2	All MCs	1	0.0	1	0.0	0.021	7.8	LOS A	0.0	0.1	0.02	0.04	0.02	86.1
11	T1	All MCs	33	16.1	33	16.1	0.021	0.0	LOS A	0.0	0.1	0.02	0.04	0.02	98.2
12	R2	All MCs	1	0.0	1	0.0	0.021	8.1	LOS A	0.0	0.1	0.02	0.04	0.02	86.0
Approach			35	15.2	35	15.2	0.021	0.5	NA	0.0	0.1	0.02	0.04	0.02	97.4
All Vehicles			283	16.4	283	16.4	0.161	6.3	NA	0.6	5.0	0.14	0.50	0.14	75.4

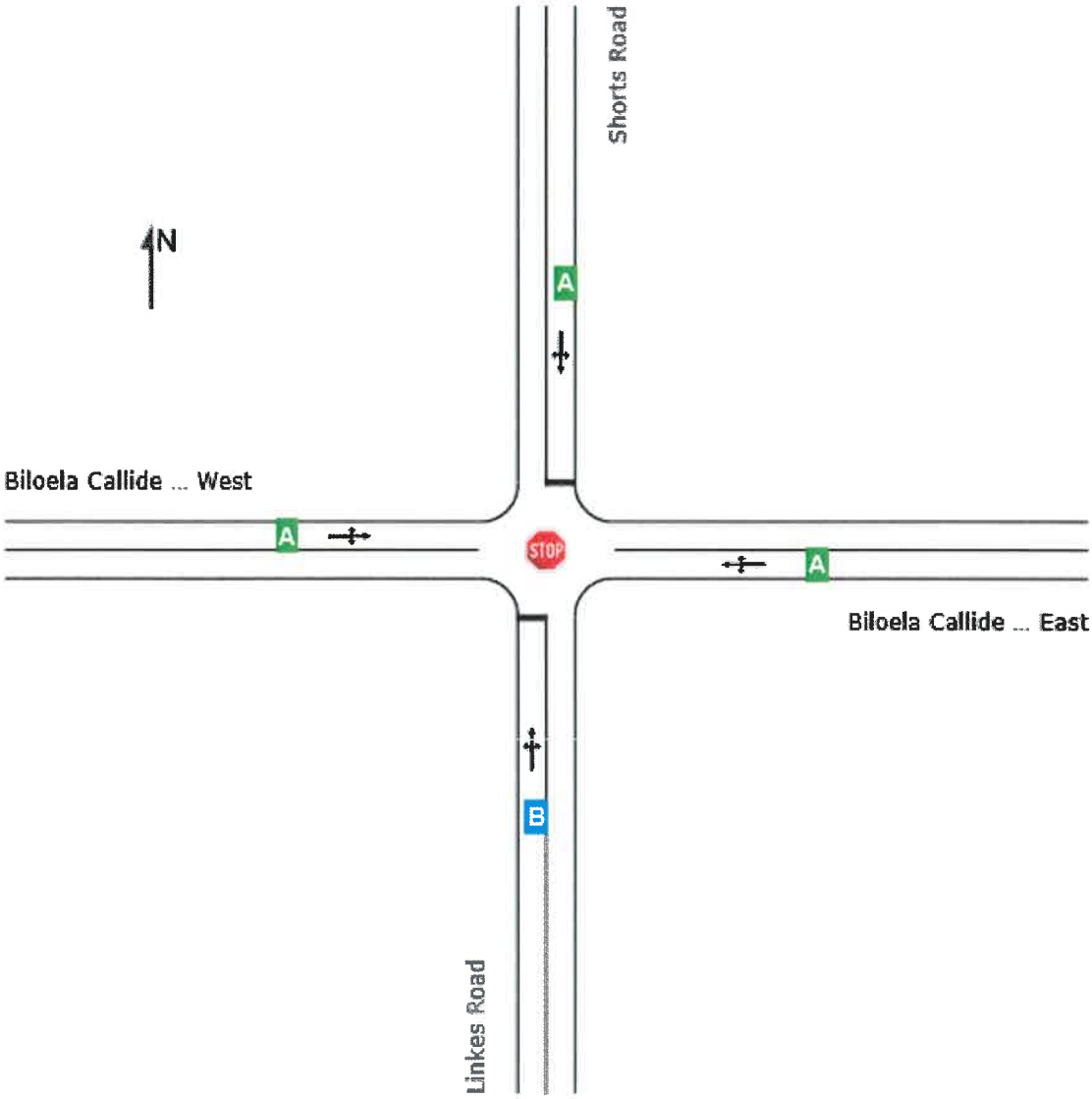
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Background 2024 PM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	A	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance

Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.038	9.6	LOS A	0.1	1.0	0.27	0.92	0.27	70.9
2	T1	All MCs	1	0.0	1	0.0	0.038	9.8	LOS A	0.1	1.0	0.27	0.92	0.27	70.9
3	R2	All MCs	26	16.0	26	16.0	0.038	11.5	LOS B	0.1	1.0	0.27	0.92	0.27	63.5
Approach			28	14.8	28	14.8	0.038	11.3	LOS B	0.1	1.0	0.27	0.92	0.27	64.0
East: Biloela Callide Road East															
4	L2	All MCs	120	17.5	120	17.5	0.094	8.4	LOS A	0.0	0.1	0.00	0.57	0.00	67.2
5	T1	All MCs	20	15.8	20	15.8	0.094	0.0	LOS A	0.0	0.1	0.00	0.57	0.00	84.5
6	R2	All MCs	1	0.0	1	0.0	0.094	7.4	LOS A	0.0	0.1	0.00	0.57	0.00	75.3
Approach			141	17.2	141	17.2	0.094	7.2	NA	0.0	0.1	0.00	0.57	0.00	69.2
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.003	9.8	LOS A	0.0	0.1	0.21	0.88	0.21	72.2
8	T1	All MCs	1	0.0	1	0.0	0.003	10.1	LOS B	0.0	0.1	0.21	0.88	0.21	72.2
9	R2	All MCs	1	0.0	1	0.0	0.003	9.3	LOS A	0.0	0.1	0.21	0.88	0.21	72.0
Approach			3	0.0	3	0.0	0.003	9.7	LOS A	0.0	0.1	0.21	0.88	0.21	72.2
West: Biloela Callide Road West															
10	L2	All MCs	1	0.0	1	0.0	0.039	7.8	LOS A	0.0	0.1	0.01	0.02	0.01	86.6
11	T1	All MCs	59	17.9	59	17.9	0.039	0.0	LOS A	0.0	0.1	0.01	0.02	0.01	99.0
12	R2	All MCs	1	0.0	1	0.0	0.039	7.8	LOS A	0.0	0.1	0.01	0.02	0.01	86.6
Approach			61	17.2	61	17.2	0.039	0.3	NA	0.0	0.1	0.01	0.02	0.01	98.5
All Vehicles			234	16.7	234	16.7	0.094	5.9	NA	0.1	1.0	0.04	0.48	0.04	74.3

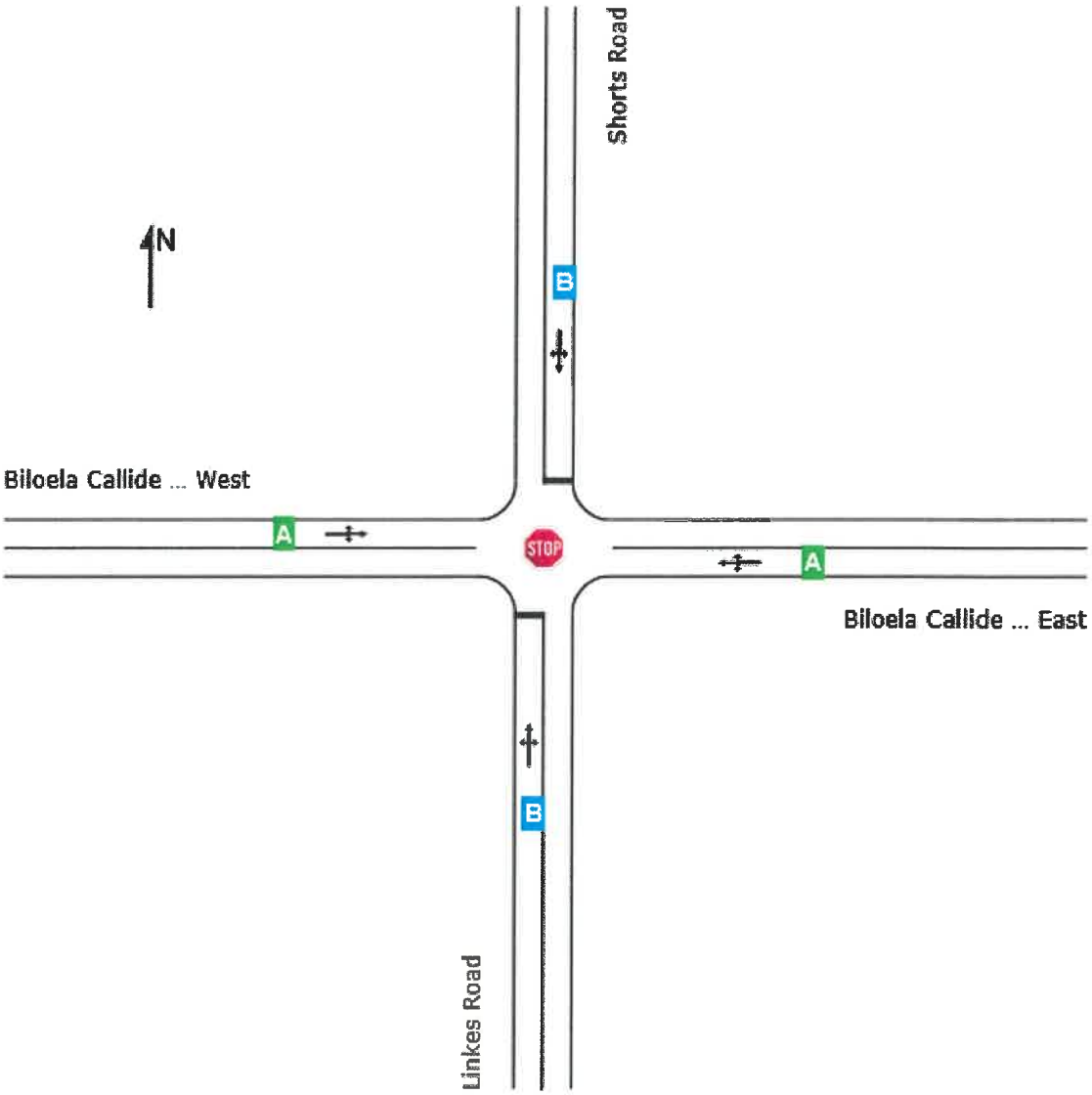
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Background 2034 AM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	B	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance

Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.191	10.2	LOS B	0.7	5.9	0.41	0.95	0.41	69.3
2	T1	All MCs	1	0.0	1	0.0	0.191	10.5	LOS B	0.7	5.9	0.41	0.95	0.41	69.3
3	R2	All MCs	116	18.2	116	18.2	0.191	13.1	LOS B	0.7	5.9	0.41	0.95	0.41	61.7
Approach			118	17.9	118	17.9	0.191	13.0	LOS B	0.7	5.9	0.41	0.95	0.41	61.8
East: Biloela Callide Road East															
4	L2	All MCs	49	19.1	49	19.1	0.115	8.5	LOS A	0.0	0.1	0.00	0.19	0.00	72.5
5	T1	All MCs	127	17.4	127	17.4	0.115	0.0	LOS A	0.0	0.1	0.00	0.19	0.00	94.3
6	R2	All MCs	1	0.0	1	0.0	0.115	7.4	LOS A	0.0	0.1	0.00	0.19	0.00	83.0
Approach			178	17.8	178	17.8	0.115	2.4	NA	0.0	0.1	0.00	0.19	0.00	87.0
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.003	9.7	LOS A	0.0	0.1	0.25	0.87	0.25	71.9
8	T1	All MCs	1	0.0	1	0.0	0.003	10.4	LOS B	0.0	0.1	0.25	0.87	0.25	71.9
9	R2	All MCs	1	0.0	1	0.0	0.003	10.1	LOS B	0.0	0.1	0.25	0.87	0.25	71.7
Approach			3	0.0	3	0.0	0.003	10.1	LOS B	0.0	0.1	0.25	0.87	0.25	71.8
West: Biloela Callide Road West															
10	L2	All MCs	1	0.0	1	0.0	0.035	7.8	LOS A	0.0	0.1	0.02	0.03	0.02	86.5
11	T1	All MCs	53	18.0	53	18.0	0.035	0.0	LOS A	0.0	0.1	0.02	0.03	0.02	98.8
12	R2	All MCs	1	0.0	1	0.0	0.035	8.2	LOS A	0.0	0.1	0.02	0.03	0.02	86.5
Approach			55	17.3	55	17.3	0.035	0.3	NA	0.0	0.1	0.02	0.03	0.02	98.3
All Vehicles			354	17.6	354	17.6	0.191	5.7	NA	0.7	5.9	0.14	0.43	0.14	77.7

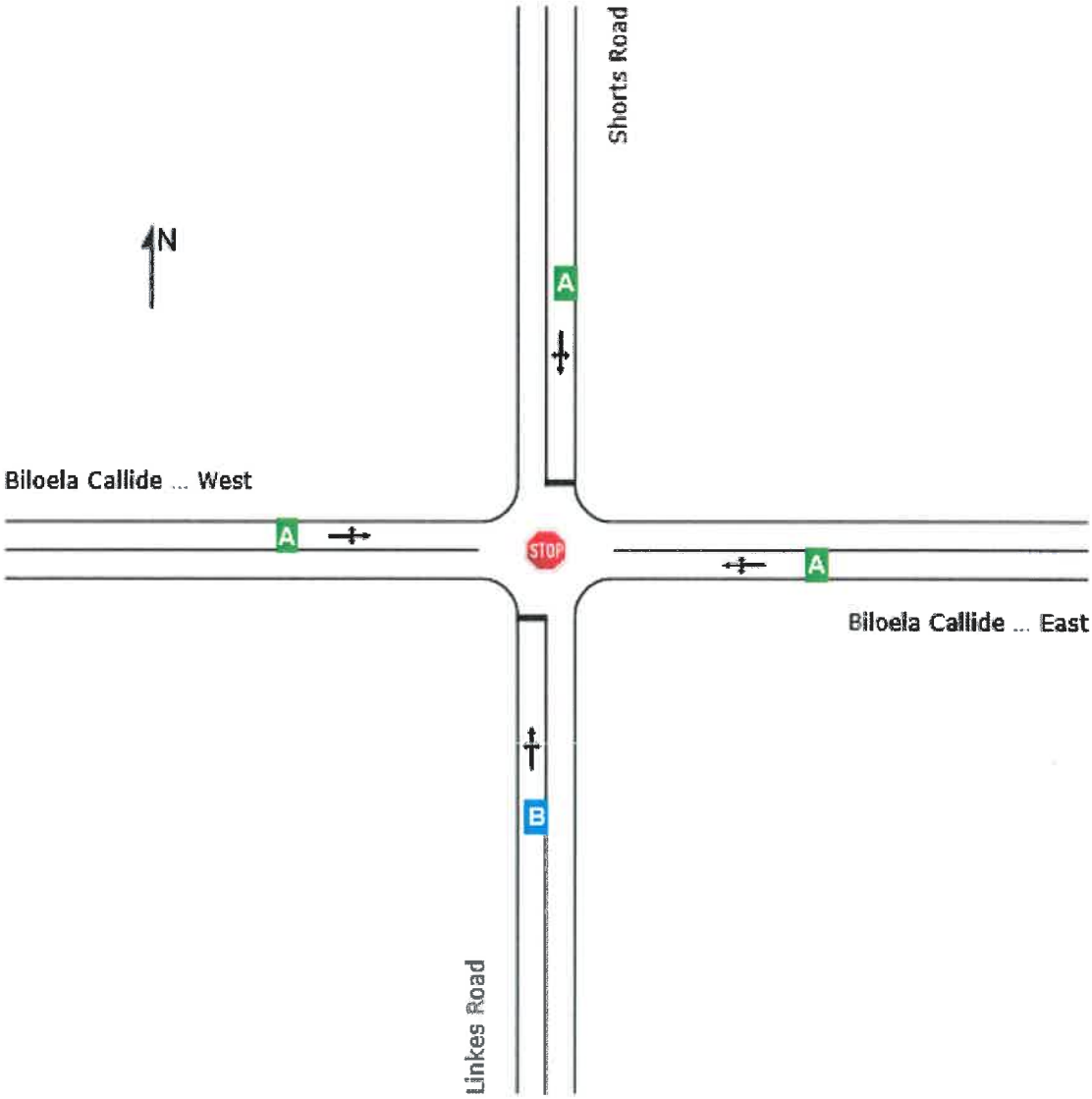
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Background 2034 PM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	A	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance												
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.041	9.6	LOS A	0.1	1.1	0.32	0.91	0.32	70.4
2	T1	All MCs	1	0.0	1	0.0	0.041	10.1	LOS B	0.1	1.1	0.32	0.91	0.32	70.4
3	R2	All MCs	26	16.0	26	16.0	0.041	12.0	LOS B	0.1	1.1	0.32	0.91	0.32	63.1
Approach			28	14.8	28	14.8	0.041	11.8	LOS B	0.1	1.1	0.32	0.91	0.32	63.6
East: Biloela Callide Road East															
4	L2	All MCs	128	17.2	128	17.2	0.104	8.4	LOS A	0.0	0.1	0.00	0.55	0.00	67.8
5	T1	All MCs	28	14.8	28	14.8	0.104	0.0	LOS A	0.0	0.1	0.00	0.55	0.00	85.1
6	R2	All MCs	1	0.0	1	0.0	0.104	7.5	LOS A	0.0	0.1	0.00	0.55	0.00	75.8
Approach			158	16.7	158	16.7	0.104	6.9	NA	0.0	0.1	0.00	0.55	0.00	70.4
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.003	9.9	LOS A	0.0	0.1	0.26	0.86	0.26	72.0
8	T1	All MCs	1	0.0	1	0.0	0.003	10.4	LOS B	0.0	0.1	0.26	0.86	0.26	72.0
9	R2	All MCs	1	0.0	1	0.0	0.003	9.6	LOS A	0.0	0.1	0.26	0.86	0.26	71.8
Approach			3	0.0	3	0.0	0.003	10.0	LOS A	0.0	0.1	0.26	0.86	0.26	72.0
West: Biloela Callide Road West															
10	L2	All MCs	1	0.0	1	0.0	0.059	7.8	LOS A	0.0	0.1	0.01	0.02	0.01	86.9
11	T1	All MCs	92	17.2	92	17.2	0.059	0.0	LOS A	0.0	0.1	0.01	0.02	0.01	99.3
12	R2	All MCs	1	0.0	1	0.0	0.059	7.8	LOS A	0.0	0.1	0.01	0.02	0.01	86.8
Approach			94	16.9	94	16.9	0.059	0.2	NA	0.0	0.1	0.01	0.02	0.01	99.0
All Vehicles			283	16.4	283	16.4	0.104	5.2	NA	0.1	1.1	0.04	0.41	0.04	76.9

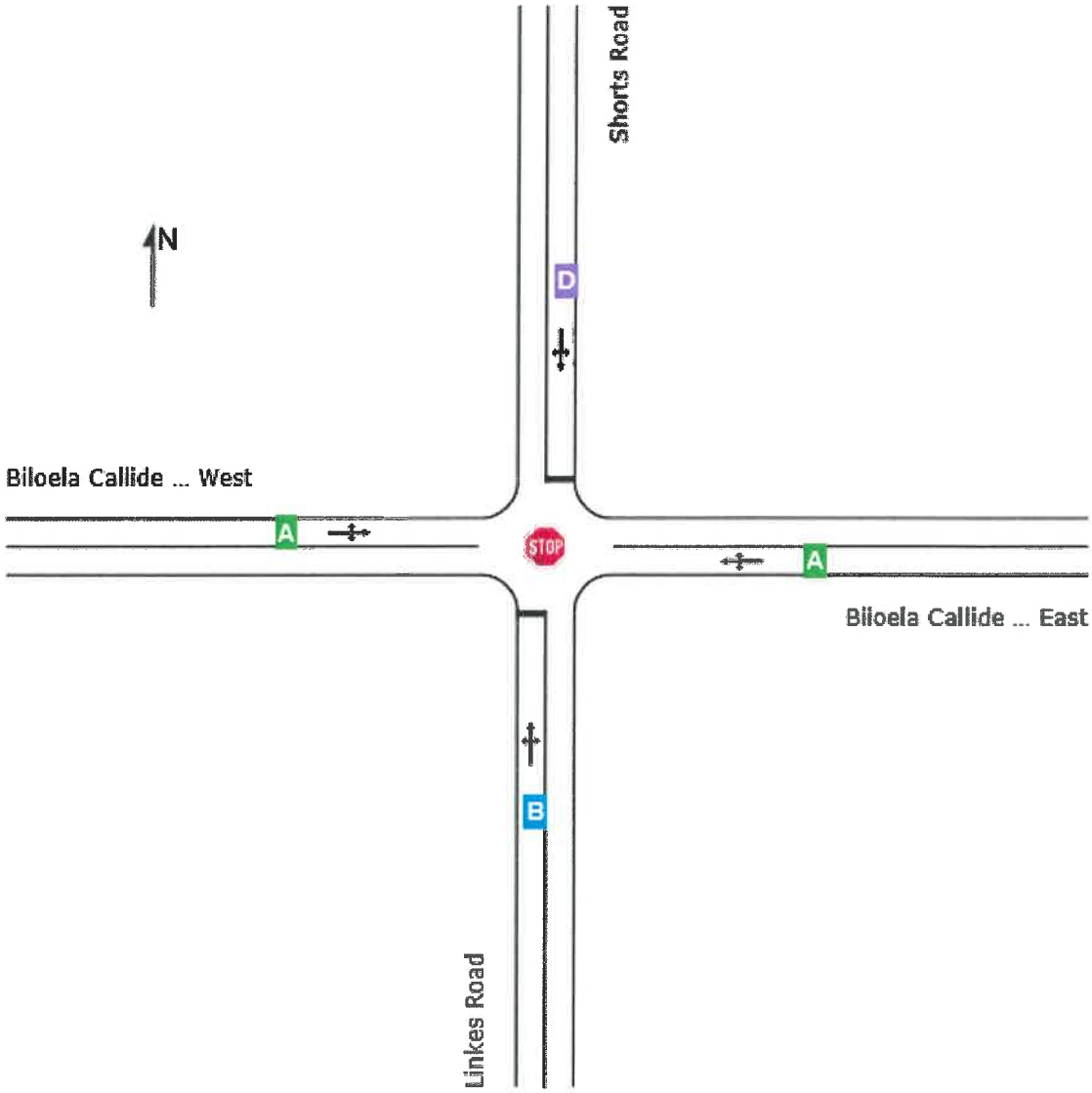
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Construction 2024 AM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	D	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance

Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.189	10.0	LOS A	0.7	5.9	0.35	0.93	0.35	70.5
2	T1	All MCs	27	0.0	27	0.0	0.189	10.5	LOS B	0.7	5.9	0.35	0.93	0.35	70.5
3	R2	All MCs	111	18.1	111	18.1	0.189	12.1	LOS B	0.7	5.9	0.35	0.93	0.35	62.6
Approach			139	14.4	139	14.4	0.189	11.7	LOS B	0.7	5.9	0.35	0.93	0.35	64.1
East: Biloela Callide Road East															
4	L2	All MCs	46	13.6	46	13.6	0.086	8.3	LOS A	0.0	0.1	0.00	0.24	0.00	73.9
5	T1	All MCs	85	17.3	85	17.3	0.086	0.0	LOS A	0.0	0.1	0.00	0.24	0.00	92.6
6	R2	All MCs	1	0.0	1	0.0	0.086	7.5	LOS A	0.0	0.1	0.00	0.24	0.00	81.7
Approach			133	15.9	133	15.9	0.086	3.0	NA	0.0	0.1	0.00	0.24	0.00	85.0
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.143	9.7	LOS A	0.5	11.9	0.51	1.04	0.51	56.5
8	T1	All MCs	1	0.0	1	0.0	0.143	10.3	LOS B	0.5	11.9	0.51	1.04	0.51	56.5
9	R2	All MCs	32	100.0	32	100.0	0.143	26.0	LOS D	0.5	11.9	0.51	1.04	0.51	38.0
Approach			34	93.8	34	93.8	0.143	25.0	LOS D	0.5	11.9	0.51	1.04	0.51	38.8
West: Biloela Callide Road West															
10	L2	All MCs	32	100.0	32	100.0	0.064	11.1	LOS B	0.0	0.1	0.01	0.34	0.01	41.4
11	T1	All MCs	33	16.1	33	16.1	0.064	0.0	LOS A	0.0	0.1	0.01	0.34	0.01	85.9
12	R2	All MCs	1	0.0	1	0.0	0.064	7.7	LOS A	0.0	0.1	0.01	0.34	0.01	76.4
Approach			65	56.5	65	56.5	0.064	5.5	NA	0.0	0.1	0.01	0.34	0.01	56.5
All Vehicles			371	29.5	371	29.5	0.189	8.7	NA	0.7	11.9	0.18	0.59	0.18	64.4

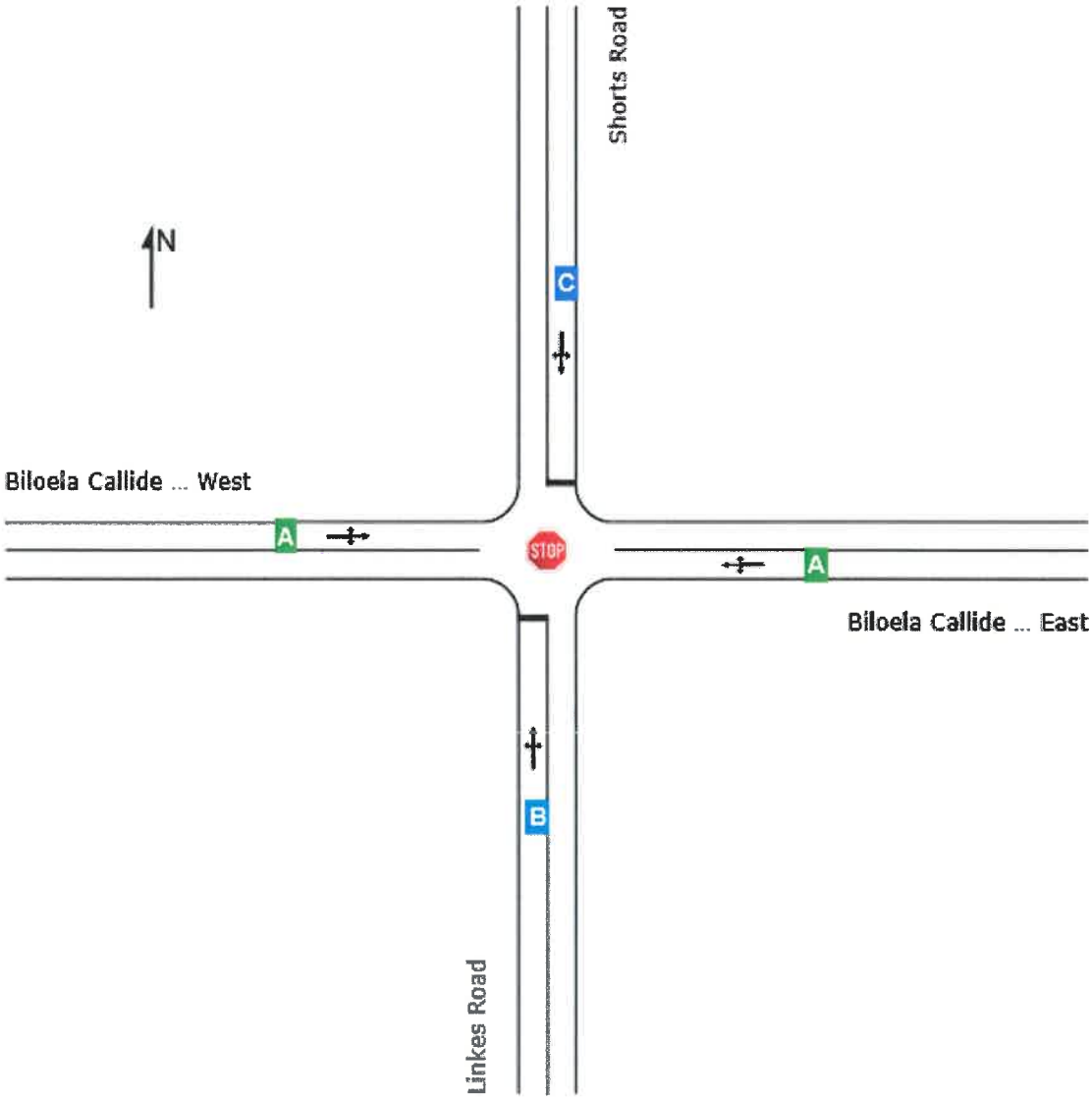
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Construction 2024 PM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	C	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance												
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.039	9.6	LOS A	0.1	1.1	0.29	0.92	0.29	70.7
2	T1	All MCs	1	0.0	1	0.0	0.039	10.3	LOS B	0.1	1.1	0.29	0.92	0.29	70.7
3	R2	All MCs	26	16.0	26	16.0	0.039	11.7	LOS B	0.1	1.1	0.29	0.92	0.29	63.3
Approach			28	14.8	28	14.8	0.039	11.6	LOS B	0.1	1.1	0.29	0.92	0.29	63.8
East: Biloela Callide Road East															
4	L2	All MCs	120	17.5	120	17.5	0.094	8.4	LOS A	0.0	0.1	0.01	0.57	0.01	67.2
5	T1	All MCs	20	15.8	20	15.8	0.094	0.0	LOS A	0.0	0.1	0.01	0.57	0.01	84.5
6	R2	All MCs	1	0.0	1	0.0	0.094	7.6	LOS A	0.0	0.1	0.01	0.57	0.01	75.3
Approach			141	17.2	141	17.2	0.094	7.2	NA	0.0	0.1	0.01	0.57	0.01	69.2
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.146	9.8	LOS A	0.5	8.7	0.42	0.99	0.42	64.1
8	T1	All MCs	27	0.0	27	0.0	0.146	10.6	LOS B	0.5	8.7	0.42	0.99	0.42	64.1
9	R2	All MCs	32	100.0	32	100.0	0.146	22.1	LOS C	0.5	8.7	0.42	0.99	0.42	41.2
Approach			60	52.6	60	52.6	0.146	16.6	LOS C	0.5	8.7	0.42	0.99	0.42	49.6
West: Biloela Callide Road West															
10	L2	All MCs	32	100.0	32	100.0	0.082	11.1	LOS B	0.0	0.1	0.01	0.24	0.01	42.0
11	T1	All MCs	59	17.9	59	17.9	0.082	0.0	LOS A	0.0	0.1	0.01	0.24	0.01	89.1
12	R2	All MCs	1	0.0	1	0.0	0.082	7.7	LOS A	0.0	0.1	0.01	0.24	0.01	79.0
Approach			92	46.0	92	46.0	0.082	3.9	NA	0.0	0.1	0.01	0.24	0.01	64.2
All Vehicles			321	31.8	321	31.8	0.146	8.4	NA	0.5	8.7	0.11	0.59	0.11	62.7

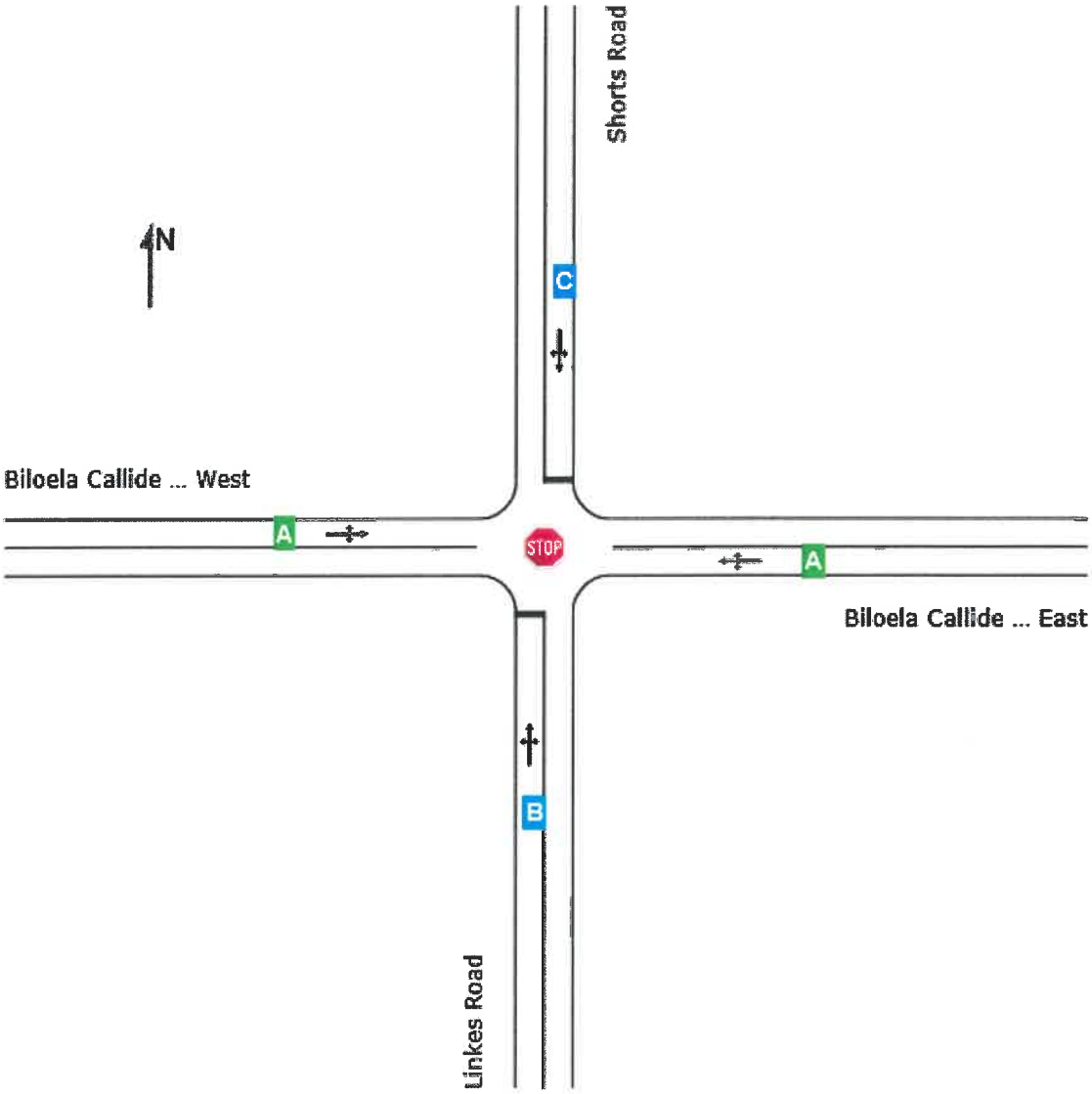
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Operation 2034 AM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	C	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance

Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.198	10.2	LOS B	0.7	6.1	0.42	0.95	0.42	69.3
2	T1	All MCs	7	0.0	7	0.0	0.198	10.5	LOS B	0.7	6.1	0.42	0.95	0.42	69.3
3	R2	All MCs	116	18.2	116	18.2	0.198	13.1	LOS B	0.7	6.1	0.42	0.95	0.42	61.7
Approach			124	16.9	124	16.9	0.198	12.9	LOS B	0.7	6.1	0.42	0.95	0.42	62.2
East: Biloela Callide Road East															
4	L2	All MCs	49	19.1	49	19.1	0.115	8.5	LOS A	0.0	0.1	0.00	0.19	0.00	72.5
5	T1	All MCs	127	17.4	127	17.4	0.115	0.0	LOS A	0.0	0.1	0.00	0.19	0.00	94.3
6	R2	All MCs	1	0.0	1	0.0	0.115	7.4	LOS A	0.0	0.1	0.00	0.19	0.00	83.0
Approach			178	17.8	178	17.8	0.115	2.4	NA	0.0	0.1	0.00	0.19	0.00	87.0
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.007	9.7	LOS A	0.0	0.3	0.37	0.87	0.37	65.2
8	T1	All MCs	1	0.0	1	0.0	0.007	10.4	LOS B	0.0	0.3	0.37	0.87	0.37	65.1
9	R2	All MCs	1	100.0	1	100.0	0.007	25.9	LOS D	0.0	0.3	0.37	0.87	0.37	41.7
Approach			3	33.3	3	33.3	0.007	15.3	LOS C	0.0	0.3	0.37	0.87	0.37	54.9
West: Biloela Callide Road West															
10	L2	All MCs	1	100.0	1	100.0	0.036	11.1	LOS B	0.0	0.1	0.01	0.03	0.01	42.3
11	T1	All MCs	53	18.0	53	18.0	0.036	0.0	LOS A	0.0	0.1	0.01	0.03	0.01	98.6
12	R2	All MCs	1	0.0	1	0.0	0.036	8.1	LOS A	0.0	0.1	0.01	0.03	0.01	86.3
Approach			55	19.2	55	19.2	0.036	0.4	NA	0.0	0.1	0.01	0.03	0.01	95.9
All Vehicles			360	17.8	360	17.8	0.198	5.8	NA	0.7	6.1	0.15	0.43	0.15	77.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

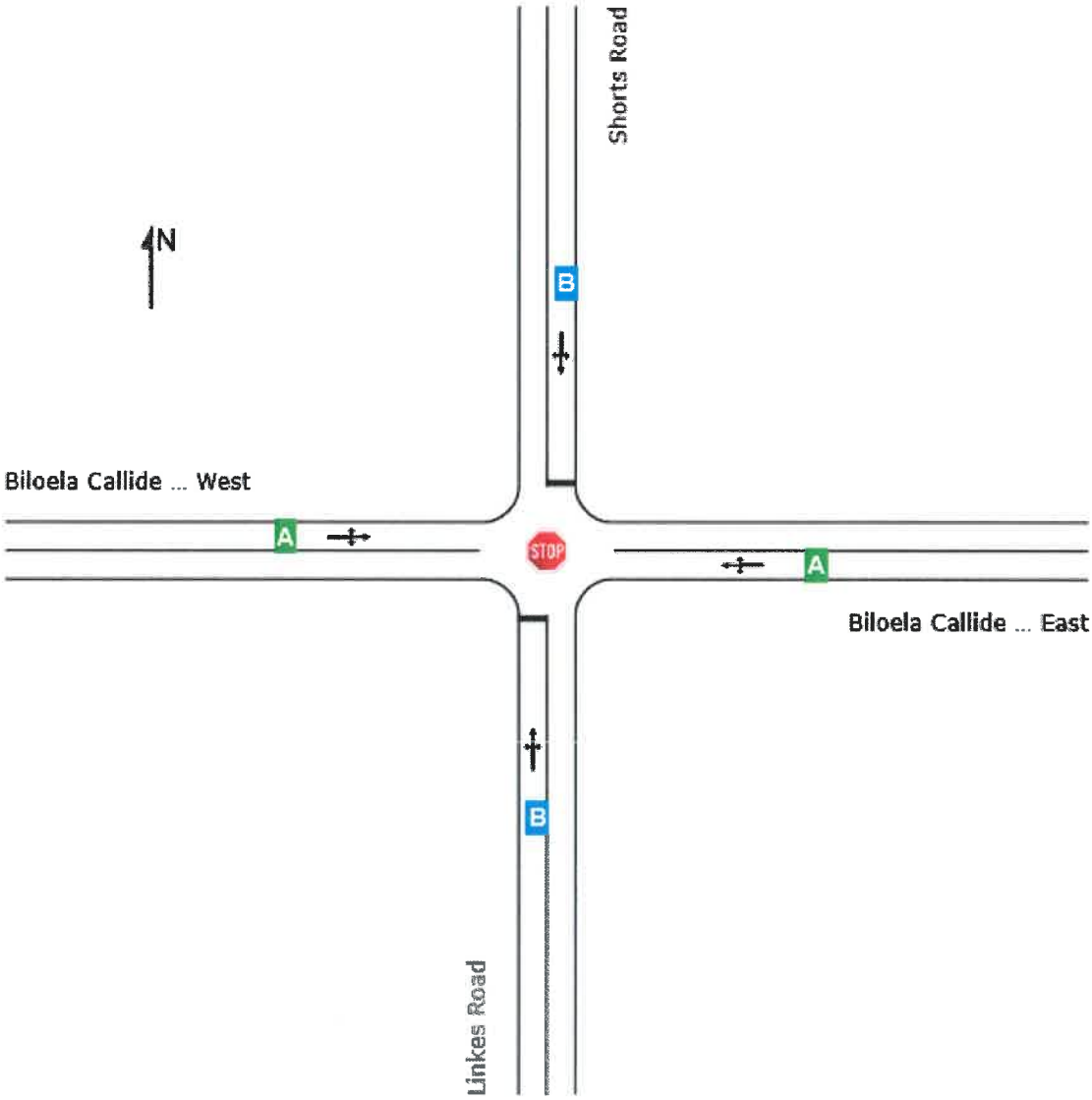
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: 101 [Operation 2034 PM Peak (Site Folder: General)]

Callide Solar Power Station
Site Category: Existing Design
Stop (Two-Way)

LOS Summary

	Approaches				Intersection
	South	East	North	West	
LOS	B	NA (TWSC)	B	NA (TWSC)	NA (TWSC)



Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Lane LOS values are based on average delay per lane.
Minor Road Approach LOS values are based on average delay for all lanes.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Vehicle Movement Performance												
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed

			[Total HV] veh/h	%	[Total HV] veh/h	%	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Linkes Road															
1	L2	All MCs	1	0.0	1	0.0	0.041	9.6	LOS A	0.1	1.1	0.32	0.91	0.32	70.3
2	T1	All MCs	1	0.0	1	0.0	0.041	10.1	LOS B	0.1	1.1	0.32	0.91	0.32	70.3
3	R2	All MCs	26	16.0	26	16.0	0.041	12.0	LOS B	0.1	1.1	0.32	0.91	0.32	63.0
Approach			28	14.8	28	14.8	0.041	11.9	LOS B	0.1	1.1	0.32	0.91	0.32	63.5
East: Biloela Callide Road East															
4	L2	All MCs	128	17.2	128	17.2	0.104	8.4	LOS A	0.0	0.1	0.00	0.55	0.00	67.8
5	T1	All MCs	28	14.8	28	14.8	0.104	0.0	LOS A	0.0	0.1	0.00	0.55	0.00	85.1
6	R2	All MCs	1	0.0	1	0.0	0.104	7.5	LOS A	0.0	0.1	0.00	0.55	0.00	75.8
Approach			158	16.7	158	16.7	0.104	6.9	NA	0.0	0.1	0.00	0.55	0.00	70.4
North: Shorts Road															
7	L2	All MCs	1	0.0	1	0.0	0.013	9.9	LOS A	0.0	0.4	0.36	0.90	0.36	69.5
8	T1	All MCs	7	0.0	7	0.0	0.013	10.5	LOS B	0.0	0.4	0.36	0.90	0.36	69.5
9	R2	All MCs	1	100.0	1	100.0	0.013	21.6	LOS C	0.0	0.4	0.36	0.90	0.36	43.4
Approach			9	11.1	9	11.1	0.013	11.7	LOS B	0.0	0.4	0.36	0.90	0.36	65.2
West: Biloela Callide Road West															
10	L2	All MCs	1	100.0	1	100.0	0.060	11.1	LOS B	0.0	0.1	0.01	0.02	0.01	42.7
11	T1	All MCs	92	17.2	92	17.2	0.060	0.0	LOS A	0.0	0.1	0.01	0.02	0.01	99.2
12	R2	All MCs	1	0.0	1	0.0	0.060	7.7	LOS A	0.0	0.1	0.01	0.02	0.01	86.8
Approach			94	18.0	94	18.0	0.060	0.2	NA	0.0	0.1	0.01	0.02	0.01	97.6
All Vehicles			289	16.7	289	16.7	0.104	5.4	NA	0.1	1.1	0.05	0.42	0.05	76.3

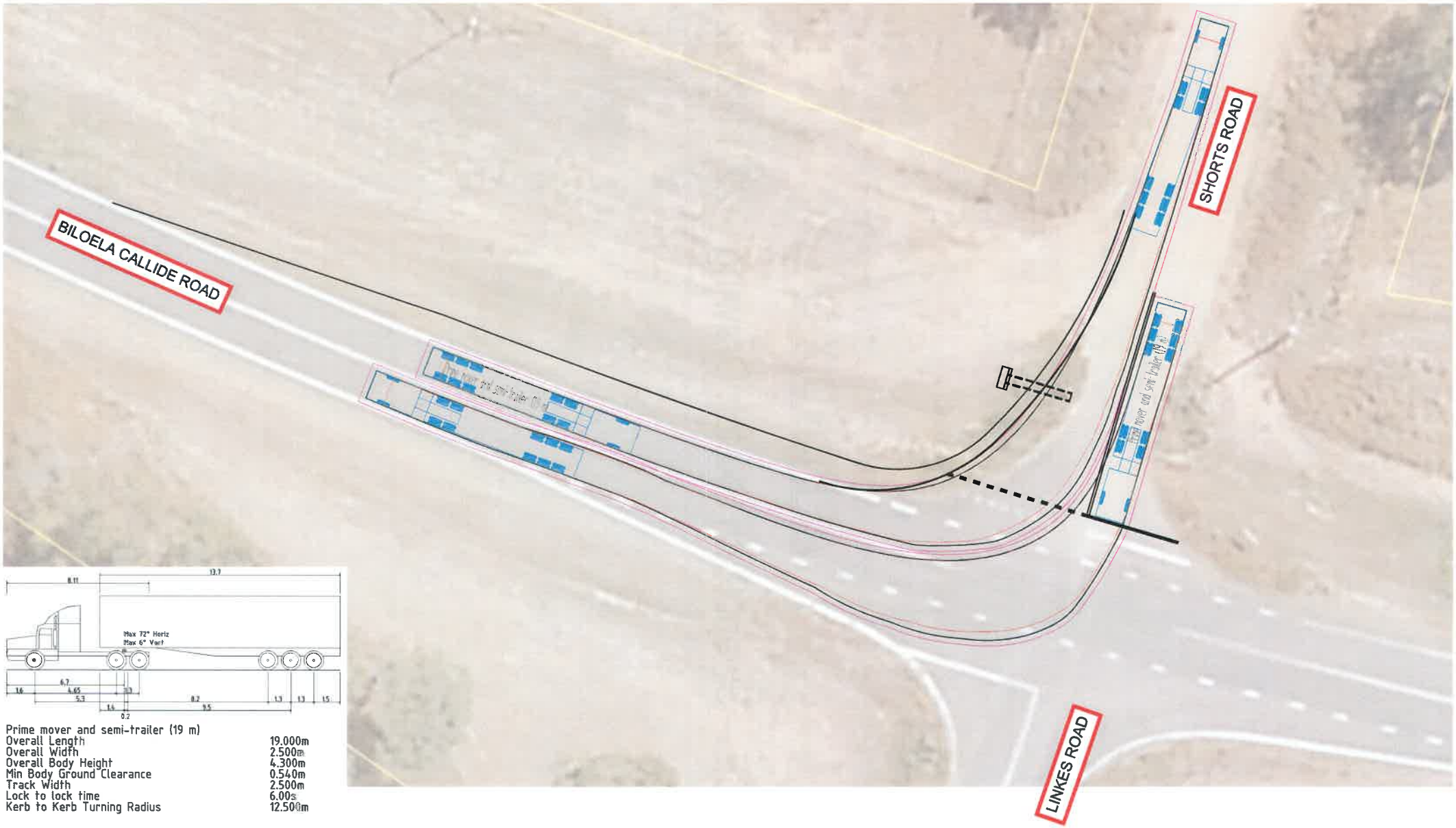
Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).
Two-Way Sign Control Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Critical Movement comparisons						
Time Status	Approach Leg	Movement Ref	Deg. Saturation	LOS	Delay (sec)	Back of Queue Dist (m)
Background 2024 AM	Biloeala Callide Road East	T1	0.086	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road East	T1	0.115	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road East	L2	0.086	A	8.3	0.1
Background 2034 AM	Biloeala Callide Road East	L2	0.115	A	8.5	0.1
Background 2024 AM	Biloeala Callide Road West	T1	0.021	A	0.0	0.1
Background 2034 AM	Biloeala Callide Road West	T1	0.035	A	0.0	0.1
Background 2024 AM	Biloeala Callide Road West	L2	0.021	A	7.8	0.1
Background 2034 AM	Biloeala Callide Road West	L2	0.035	A	7.8	0.1
Background 2024 AM	Shorts Road	T1	0.003	A	9.9	0.1
Background 2034 AM	Shorts Road	T1	0.003	B	10.4	0.1
Background 2024 AM	Shorts Road	R2	0.003	A	9.6	0.1
Background 2034 AM	Shorts Road	R2	0.003	B	10.1	0.1
Background 2024 AM	Linkes Road	T1	0.161	A	10.0	5.0
Background 2034 AM	Linkes Road	T1	0.191	B	10.5	5.9
Background 2024 AM	Linkes Road	R2	0.161	B	12.0	5.0
Background 2034 AM	Linkes Road	R2	0.191	B	13.1	5.9
Background 2024 PM	Biloeala Callide Road East	T1	0.094	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road East	T1	0.104	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road East	L2	0.094	A	8.4	0.1
Background 2034 PM	Biloeala Callide Road East	L2	0.104	A	8.4	0.1
Background 2024 PM	Biloeala Callide Road West	T1	0.039	A	0.0	0.1
Background 2034 PM	Biloeala Callide Road West	T1	0.059	A	0.0	0.1
Background 2024 PM	Biloeala Callide Road West	L2	0.039	A	7.8	0.1
Background 2034 PM	Biloeala Callide Road West	L2	0.059	A	7.8	0.1
Background 2024 PM	Shorts Road	T1	0.003	B	10.1	0.1
Background 2034 PM	Shorts Road	T1	0.003	B	10.4	0.1
Background 2024 PM	Shorts Road	R2	0.003	A	9.3	0.1
Background 2034 PM	Shorts Road	R2	0.003	A	9.6	0.1

NORTHERN CONSULTING engineers

APPENDIX G

NCE – Swept Path Sketch Drawing – 22/11/2022



Prime mover and semi-trailer (19 m)

Overall Length	19.000m
Overall Width	2.500m
Overall Body Height	4.300m
Min Body Ground Clearance	0.540m
Track Width	2.500m
Lock to lock time	6.00s
Kerb to Kerb Turning Radius	12.500m

NORTHERN CONSULTING engineers

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W: www.nceng.com.au

MJ2370/SK01

INTERSECTION UPGRADE LAYOUT

Scale 1:300 @ A3

NORTHERN CONSULTING engineers

APPENDIX H

NCE – Certification Statement and Authorisation

Appendix B: Traffic impact assessment certification

Certification of Traffic Impact Assessment Report

Registered Professional Engineer Queensland

for

Project title:	Callide Solar Power Station, Mount Murchison. 4715 Lot 154 on SP126053, Lot 2 on RP619032, Lot 28 on RN519, Lot 3 on RP608599 Traffic Impact Assessment (MJ2370-TIA)

As a professional engineer registered by the Board of Professional Engineers of Queensland pursuant to the *Professional Engineers Act 2002* as competent in my areas of nominated expertise, I understand and recognise:

- the significant role of engineering as a profession, and that
- the community has a legitimate expectation that my certification affixed to this engineering work can be trusted, and that
- I am responsible for ensuring its preparation has satisfied all necessary standards, conduct and contemporary practice.

As the responsible RPEQ, I certify:

(i) I am satisfied that all submitted components comprising this traffic impact assessment, listed in the following table, have been completed in accordance with the *Guide to Traffic Impact Assessment* published by the Queensland Department of Transport and Main Roads and using sound engineering principles, and

(ii) where specialised areas of work have not been under my direct supervision, I have reviewed the outcomes of the work and consider the work and its outcomes as suitable for the purposes of this traffic impact assessment, and that

(iii) the outcomes of this traffic impact assessment are a true reflection of results of assessment, and that

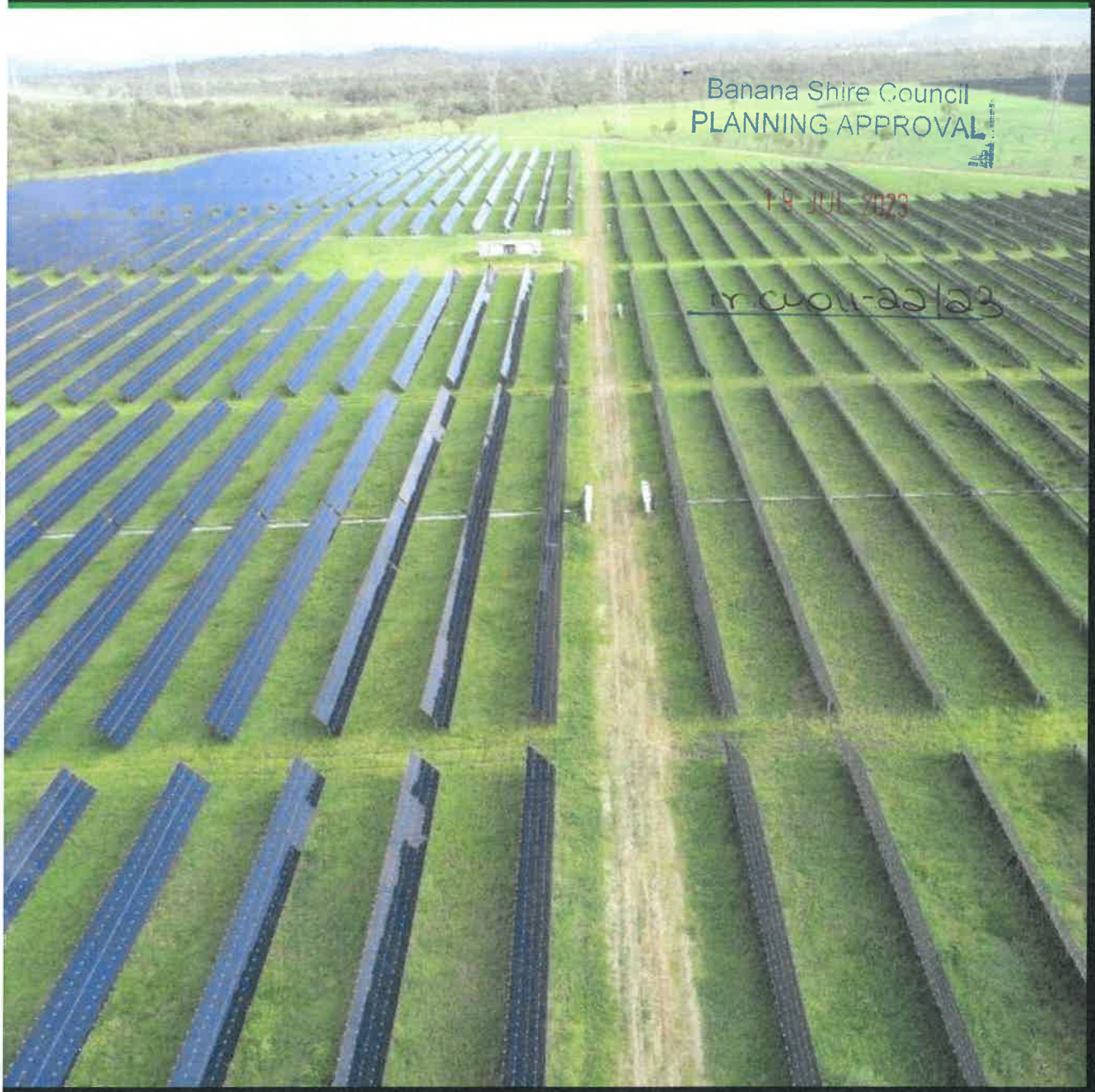
(iv) I believe the strategies recommended for mitigating impacts by this traffic impact assessment, embrace contemporary practice initiatives and will deliver the desired outcomes.

Name:	Derek Saw	RPEQ No:	7363
RPEQ competencies:	Civil		
Signature:		Date:	23rd Novermber 2022
Postal address:	50 Punari Street, Currajong. 4812		
Email:	derek.saw@nceng.com.au		

Traffic impact assessment components to which this certification applies	✓
1. Introduction	
Background	✓
Scope and study area	✓
Pre-lodgement meeting notes	
2. Existing Conditions	
Land use and zoning	✓
Adjacent land uses / approvals	✓
Surrounding road network details	✓
Traffic volumes	✓
Intersection and network performance	✓
Road safety issues	✓
Site access	✓
Public transport (if applicable)	✓
Active transport (if applicable)	✓
Parking (if applicable)	✓
Pavement (if applicable)	✓
Transport infrastructure (if applicable)	✓
3. Proposed Development Details	
Development site plan	✓
Operational details (including year of opening of each stage and any relevant catchment / market analysis)	✓
Proposed access and parking	✓
4. Development Traffic	
Traffic generation (by development stage if relevant and considering light and heavy vehicle trips)	✓
Trip distribution	✓
Development traffic volumes on the network	✓
5. Impact Assessment and Mitigation	
With and without development traffic volumes	✓
Construction traffic impact assessment and mitigation (if applicable)	✓
Road safety impact assessment and mitigation	✓
Access and frontage impact assessment and mitigation	✓
Intersection delay impact assessment and mitigation	✓
Road link capacity assessment and mitigation	
Pavement impact assessment and mitigation	
Transport infrastructure impact assessment and mitigation	✓
Other impacts assessment relevant to the specific development type / location (if applicable)	✓

Traffic impact assessment components to which this certification applies	✓
6. Conclusions and Recommendations	
Summary of impacts and mitigation measures proposed	✓
Certification statement and authorisation	✓
[change above and / or insert other component as needed]	

civil IQ



Surface Water and Flood Impact Assessment Report

Callide Solar Power Station

Reference No. EDF-002
Prepared for Edify Energy
9 November 2022

Document Control

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Project Name:	Callide Solar Power Station
Project Number:	EDF-002
Revision Number	A

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Revision No.	Date	Prepared By	Reviewed By	Approved For Issue By
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civil IQ Company Details

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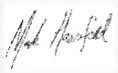
Signature	
Telephone	0434 940 509
Email	mark@civil-iq.com.au

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Executive summary

Edify Energy proposes to develop a solar power station at Mount Murchison in Central Queensland, associated within an investigation area of approximately 515.4 Ha, with an anticipated development footprint of approximately 466 Ha. This Surface Water and Flood Impact Assessment report is to address the Banana Shire Planning Scheme requirements relevant to stormwater and flooding.

The subject site is located within an agricultural area, with the CS Energy Callide Power Station located directly adjacent to the proposed eastern boundary, Biloela Callide Road and Callide Creek to the south, and Moura Rail System directly adjacent to the northern boundary. Shorts Road traverses through the centre of the site. The township of Biloela is located approximately 8 km south-west of the site.

The fall of the site is generally from north to south, with a level of 230 m Australian height datum (AHD) to the north and approximately 180 m AHD to the south. Several low points and gullies are located through the site with a stream order 1 watercourse located on the eastern side, directly adjacent to the Callide Power Station. There are several small farm dams located upstream and downstream within the vicinity of the site, and numerous contour bank features within the proposed project footprint.

The hydrologic (rainfall-runoff) modelling was undertaken using the Watershed Bounded Network Model (WBNM) and the hydraulic modelling was undertaken using Tuflow HPC SGS (2020-10-AD). Key results of the surface water and flood impact assessment for the 1% Annual Exceedance Probability (AEP) are:

- 1% AEP depths throughout are typically highest within the identified drainage/ waterway corridors and buffer zones. There are areas of breakouts from the main channels throughout the site, however, these are predominantly less than 0.5 m in depth as a maximum, with flows occurring as sheet flows.
- Due to the lack of sufficient terrain data in the northern section of the site, the watercourse buffer areas have been assessed and considered adequate for containment of higher channelised water depths, with shallow sheet flows perceived to typically occur in the northern areas, consistent with modelling observations throughout for the 1% AEP.
- The flood hazard mapping results within drainage lines and watercourses are typically contained within the nominated watercourse buffer zones throughout with associated hazards higher than H3. There are isolated occurrences where the flood hazard mapping marginally exceeds the H2 hazard category, however, this is largely associated with existing contour banks where concentrated sheet flows increase in depth locally. Localised modifications to landforms during future design stages are likely to resolve these localised hazards
- The solar arrays have been assessed to not have a significant impact on run off volumes, peaks, or times to peak, when associated with the provision of good vegetative ground cover throughout to replicate the existing scenario
- The laydown area footprint is approximately 5.9 Ha with an associated increase of approximately 1.2% of the proposed maximum development footprint of 466 Ha, resulting in an insignificant increase in 1% AEP flows relative to the proposed project extents and the greater regional Lake Callide catchment. A minimum freeboard of 300 mm for the peak 1% AEP storm event level is to be determined for habitable buildings and critical infrastructure during future project development. Diversion of external catchment flows for an approximate 60 Ha catchment is to be achieved through diversions channels/ bunding of the laydown area.
- A treatment train approach is required to be adopted, with tertiary implementation measures, such as bioretention basins or wetlands, within the laydown area development footprint to treat to the leading practice pollutant reduction targets prior to discharge to the receiving environment
- Erosion and Sediment Control measures are to be developed during the project development in accordance with the requirements of the Best Practice Erosion and Sediment Control Guidelines, IECA (2008), and in context of anticipated construction methods for the extent of works

The following key limitations are identified for the project to be addressed during the next project stages:

- Sufficient LiDAR data was not available for the upper extents of the Project area. This data is required to confirm extents of hazard flows in the upper catchment to confirm proposed extents of the solar array layout
- The Moura Rail system upstream of the Project site, and associated culverts crossings, were not within the limits of the hydraulic model. Inclusion of these would provide for accurate assessment of flow distribution. It is noted that inclusion of these features may actually bypass flow along the rail line south-west away from the Project site.
- Project development of site infrastructure within the laydown area

1 Introduction

Edify Energy has engaged Civil IQ to prepare a Surface Water and Flood Impact Assessment report to accompany Edify Energy's overarching planning report for submission to Banana Shire Council (BSC). The planning report will support the Development Application (DA) for a Development Permit for a Material Change of Use – Public Facility – Other (Solar PV Power Station, with battery energy storage system) for proposed development of the Callide Solar Power Station project.

1.1 Background

Edify Energy proposes to develop a solar power station at Mount Murchison in Central Queensland, associated within an investigation area of approximately 515.4 Ha, with an anticipated development footprint of approximately 466 Ha. The proposed development is on multiple land parcels described as Lot 28 on RN519, Lot 3 on RP608599, Lot 2 on RP619032 and Lot 154 on SP126053, with the proposed development footprint shown in Figure 1.

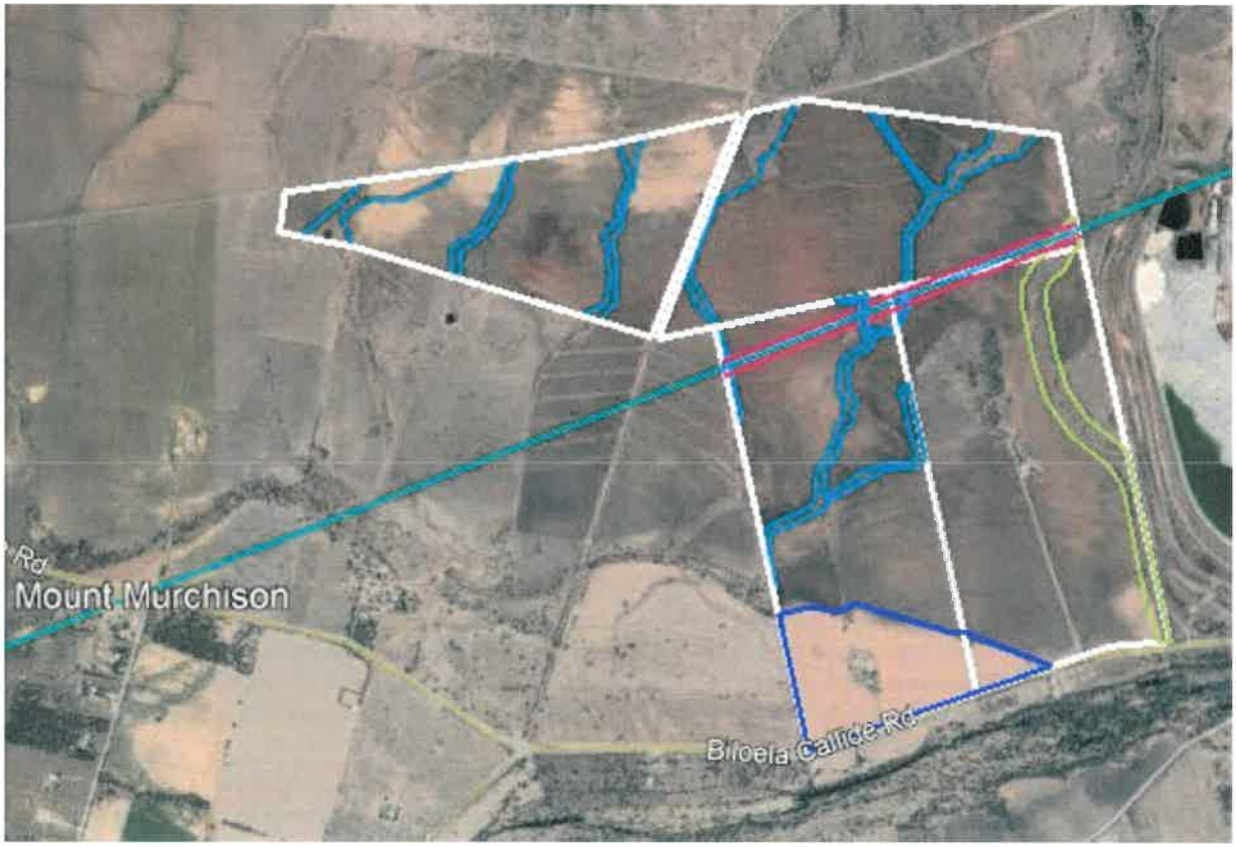


Figure 1: Site location

1.2 Purpose and objectives

The purpose of the Surface Water and Flood Impact Assessment report is to address Part 6, Section 6.3, Table 6.3.2 of the Banana Shire Planning Scheme. All relevant requirements have been documented in Appendix A to address how the Performance Outcomes have been addressed in the report, with specific references provided.

1.3 Scope of works

The scope of works for the project is to develop a stormwater management plan, hydrological model and 2d hydraulic flood model to meet assessment requirements as summarised below:

- Delineate external and internal catchments associated with the project site
- Develop a hydrologic model for input into the hydraulic models

- Build a flood model for the site
- Assess the 1% Annual Exceedance Probability (AEP) design event to Australian Rainfall and Runoff (ARR) 2019 requirements
- Assess impacts of the proposed development footprint inclusive of solar arrays, internal access roads and site laydown and infrastructure areas, as shown by the proposed site layout provided in Appendix B
- Develop mapping of key flood modelling results for the 1% AEP including flooding depths and hazard mapping

1.4 Site description

The subject site is located within an agricultural area, with the CS Energy Callide Power Station located directly adjacent to the proposed eastern boundary, Biloela Callide Road and Callide Creek to the south, and Moura Rail System directly adjacent to the northern boundary. Shorts Road traverses through the centre of the site. The township of Biloela is located approximately 8 km south-west of the site.

The fall of the site is generally from north to south, with a level of 230 m Australian height datum (AHD) to the north and approximately 180 m AHD to the south. Several low points and gullies are located through the site with a stream order 1 watercourse located on the eastern side, directly adjacent to the Callide Power Station. There are several small farm dams located upstream and downstream within the vicinity of the site, and numerous contour bank features within the proposed project footprint.

2 Data

2.1 Topographic data

Queensland Government topographic data was sourced for the project consisting of the following:

- Biloela Thangool 2011 1m DEM tif data
- SRTM derived 1 second digital surface model version 3
- QTopo online topographic maps data

The 1m DEM data sourced does not cover the footprint of the entire site, as demonstrated in Figure 2.

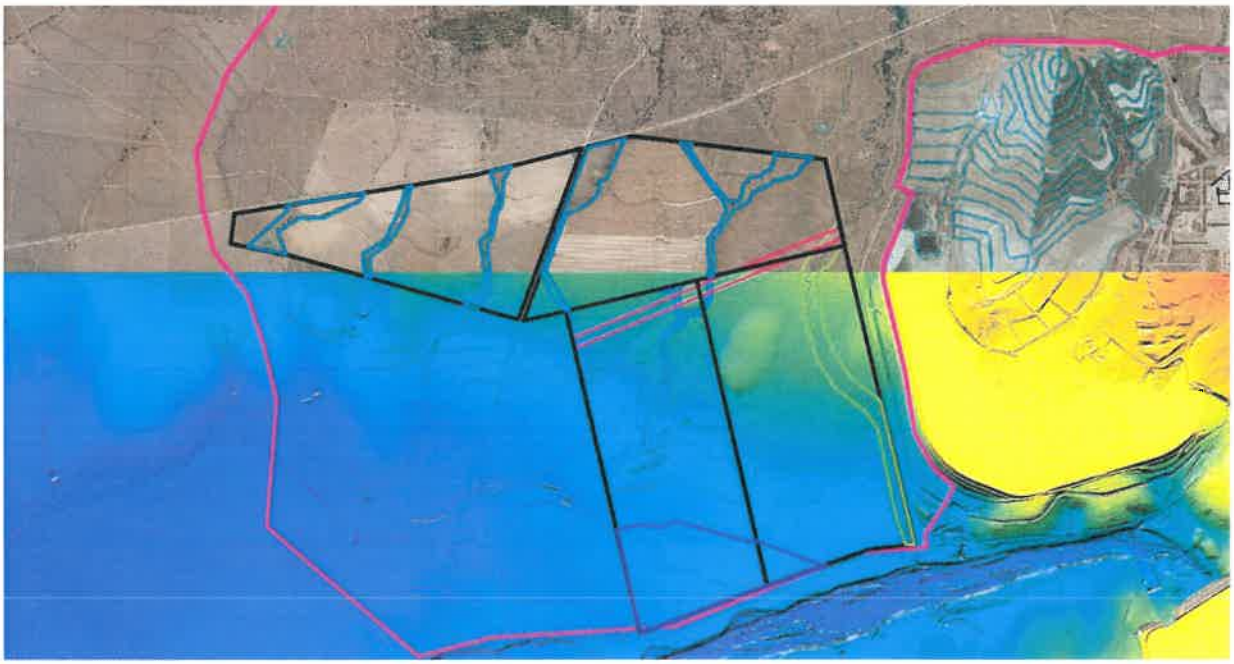


Figure 2: LiDAR data extents relative to the site footprint

2.2 Design rainfall data

Intensity Frequency Durations (IFD) data was sourced from the Bureau of Meteorology (BoM) for site latitude and longitude (-24.3375S, 150.5875E), as provided in Table 1.

Table 1: BoM IFD data rainfall depths – centroid gauge IFD06 (refer to Figure 3)

Duration	Annual Exceedance Probability (AEP) – Design rainfall intensities (mm/hr)						
	63.20%	50%	20%	10%	5%	2%	1%
1 min	2.4	2.66	3.49	4.04	4.58	5.28	5.82
2 min	4.05	4.51	5.98	7.03	8.04	9.35	10.4
3 min	5.71	6.35	8.39	9.82	11.2	13	14.4
4 min	7.26	8.07	10.6	12.4	14.1	16.3	18
5 min	8.7	9.66	12.7	14.7	16.7	19.3	21.3
10 min	14.4	15.9	20.8	24	27.2	31.3	34.4
15 min	18.3	20.3	26.6	30.7	34.7	40	44
20 min	21.3	23.7	30.9	35.8	40.5	46.7	51.4
25 min	23.7	26.3	34.4	39.8	45.2	52.2	57.5
30 min	25.6	28.4	37.2	43.2	49	56.7	62.6

Duration	Annual Exceedance Probability (AEP) – Design rainfall intensities (mm/hr)						
	63.20%	50%	20%	10%	5%	2%	1%
45 min	29.7	33	43.5	50.6	57.7	67	74.2
1 hour	32.6	36.2	47.8	55.8	63.7	74.2	82.4
1.5 hour	36.5	40.5	53.6	62.8	72	84.2	93.9
2 hour	39.1	43.5	57.6	67.7	77.7	91.2	102
3 hour	42.9	47.7	63.3	74.4	85.7	101	113
4.5 hour	46.7	52	69.2	81.5	94.1	112	126
6 hour	49.7	55.3	73.8	87	101	120	135
9 hour	54.3	60.5	81	95.8	111	133	151
12 hour	57.9	64.5	86.8	103	120	145	164
18 hour	63.5	71	96.4	115	135	164	187
24 hour	67.9	76.1	104	126	148	181	207
30 hour	71.6	80.3	111	134	160	196	225
36 hour	74.7	83.9	117	142	170	209	241
48 hour	79.7	89.8	126	155	187	231	268
72 hour	86.8	98.3	140	174	212	265	309
96 hour	91.7	104	149	186	228	286	335
120 hour	95.1	108	155	194	237	298	350
144 hour	97.6	111	159	197	242	304	357
168 hour	99.5	113	161	198	242	305	357

2.3 Drainage structures

Drainage structure information was not available for the Moura Railway System at the northern boundary of the site for this assessment. Therefore, conservatively, drainage structure information has not been incorporated into the hydraulic model.

3 Flood impact assessment and flood modelling

3.1 Hydrologic assessment

The hydrologic (rainfall-runoff) modelling was undertaken using the Watershed Bounded Network Model (WBNM) WBNM calculates flood runoff from rainfall hyetographs, allowing hydrographs to be calculated. Figure 3 presents the layout of the WBNM hydrologic model. It is noted that the full spatial rainfall gradient was with the red points indicating the IFD gauge locations.

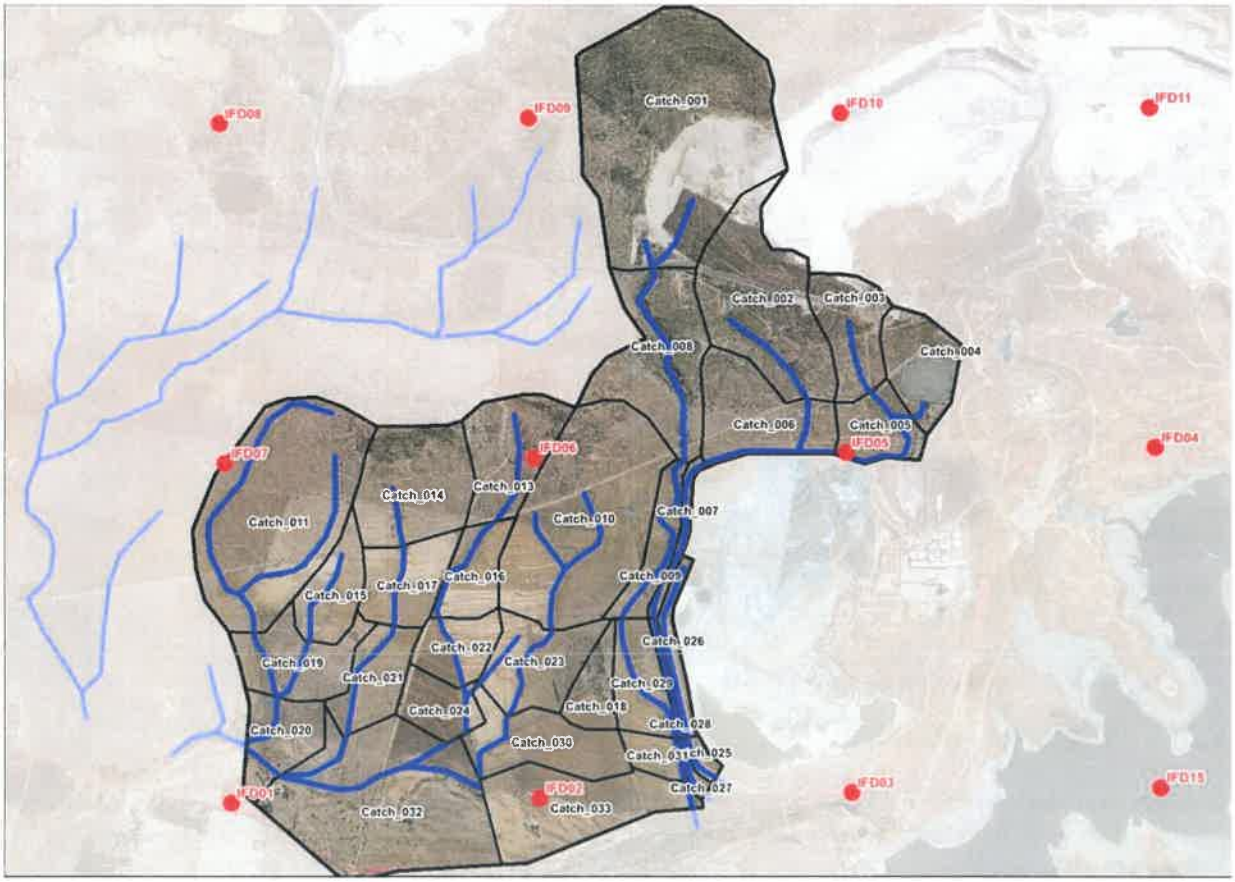


Figure 3: WBNM model layout showing design rainfall gauge locations

3.1.1 Catchments

The area of assessment incorporates the site and relevant catchments to the Mount Isa Rail line. Sub-catchments were delineated based on Department of Natural Resources and Mines (DNRM) 1 m Aerial Laser Survey (ALS) (conducted 2011) and Geoscience Australia (GA) 5 m DEM-H elevation data, as illustrated on Figure 1 in Appendix B and summarised in Table 2.

Table 2: Sub-catchment summary

ID	Area (ha)	ID	Area (ha)
Catch_001	267.5	Catch_021	39.3
Catch_002	116.7	Catch_022	32.2
Catch_003	48.2	Catch_023	55.7
Catch_004	46.1	Catch_024	28.4
Catch_005	35.1	Catch_029	37.0
Catch_006	69.4	Catch_030	59.4

ID	Area (ha)	ID	Area (ha)
Catch_008	95.2	Catch_031	15.3
Catch_009	30.4	Catch_007	11.1
Catch_010	177.3	Catch_026	16.9
Catch_011	185.7	Catch_025	6.1
Catch_013	63.2	Catch_028	5.6
Catch_014	83.1	Catch_027	4.2
Catch_015	36.6	Catch_033	87.7
Catch_016	34.4	Catch_032	181.5
Catch_017	45.0	Catch_020	40.8
Catch_018	23.9	**TOTAL**	**2031.3**
Catch_019	52.3		

Current best practices were adopted to estimate design storm runoff for the existing catchment conditions, including the following model parameters:

- 0% fraction impervious
- 1.3 catchment lag factor
- BoM 2016 design rainfall depths and intensities, refer to Table 1.
- ARR, Geosciences Australia, (2019) temporal patterns
- Initial storm loss of 10 mm
- Continuing storm loss of 1.5 mm/hr

The 1% AEP, 1 to 12 hour durations and temporal ensembles were simulated in the hydrological model. Adopting the default WBNM catchment lag factor of 1.6, and default ARR Data Hub initial loss and median pre-burst losses the WBNM peak flow estimates were approximately 40% lower than Regional Flood Frequency Estimate (RFFE) calculations. Noting the significant difference between estimates and lack of additional information to support a robust model calibration process, the WBNM catchment lag parameter was reduced to 1.3 and a constant storm initial loss of 10 mm was adopted such that WBNM peak flow estimates were higher and thus closer to RFFE estimates, producing conservative outcomes.

3.1.2 Verification

Model calibration to historical events could not be performed as there are no pluvio-rainfall stations, historic stream gauges, or anecdotal records of historic storms within the model area. Therefore, results were validated to a RFFE as presented in Table 3

In context of the RFFE assessment, the catchment shape for the RFFE verification is not regular due to the large Callide Power Station site which diverts flow around its perimeter and retains significant flows on site. Nonetheless, the RFFE is a meaningful order of magnitude check to confirm that the WBNM estimates are reasonable. For the RFFE assessment, the total WBNM model was passed through catchment 020 and therefore the peak WBNM flows presented in Table 3 are for the 20.3 km^2 catchment

Table 3: Design storm validation (catchment 020 – 20.3 km^2 upstream area)

AEP (%)	Peak Flow (m^3/s)	
	WBNM (median storm)	RFFE
5	144	131
2	176	190
1	204	243

The above results indicate a reasonable WBNM model validation to meet the assessment objectives.

3.1.3 Critical duration

Peak flows from the WBNM model were reviewed at the upstream limits of the TUFLOW model to determine the critical durations required for the hydraulic assessment. The results of the WBNM modelling for the 1% AEP are summarised in Table 4. The TUFLOW model accordingly enveloped the results of the 60-minute duration with storm ID 5105 and the 90-minute duration storm with ID 5329.

Table 4: WBNM critical duration assessment

Catchment	Duration and Storm ID	1% AEP Peak Flow
Catchment 7	90-minute, Storm ID 5329	48.9 m^3/s
Catchment 9	90-minute, Storm ID 5329	47.4 m^3/s
Catchment 11	60-minute, Storm ID 5105	31.8 m^3/s
Catchment 15	60-minute, Storm ID 5105	8.1 m^3/s

3.2 Hydraulic assessment

The hydraulic modelling was undertaken using Tuflow HPC SGS (2020-10-AD). Tuflow is a hydraulic and hydrodynamic modelling software package.

3.2.1 Model setup

The Tuflow model was developed to incorporate the following parameters into the model:

- Topography is based on DNRM ALS data with a 2 m model grid
- Natural contour banks applied as 'thin' break lines
- Inflow polygons with the appropriate local/ total hydrographs applied
- Normal depth applied to downstream model boundary
- Global Manning's n for light vegetation/ grass / field – 0.05

3.2.2 Limitations

The following key limitations are identified for the TUFLOW assessment to be resolved during the next project stages:

- LiDAR data was not available for the upper limits of the Project area. Desirably this would be obtained to confirm extents of hazard flows in the upper catchment to confirm proposed extents solar array layout
- The Moura Rail system upstream of the Project site, and associated culverts crossings, were not within the limits of the hydraulic model. Inclusion of these would provide for accurate assessment of flow distribution. It is noted that inclusion of these features may actually bypass flow along the rail line SW away from the Project site.
- Downstream boundary conditions were nominal low flow conditions. The 1% AEP Callide Creek flood extents should be identified from separate Council commissioned study to confirm that development footprint of the Solar Arrays is outside the limits of this regional flooding

3.2.3 Hydraulic modelling results

The hydraulic modelling results are discussed in this section with reference to the results mapping provided in Appendix D.

3.2.3.1 Proposed infrastructure

The infrastructure inputs and parameters for the proposed development footprint are summarised below:

- PV solar panels array blocks are proposed to be supported on piled foundations. There is no increase in imperviousness or concentration of flows associated with the rain shadow of the solar panel arrays. Stormwater sheet flows evenly from the downslope of the panels and discharges directly to the existing in-situ ground conditions, and is mobilised for infiltration and surface flows, replicating the existing catchment scenario and characteristics. Reference to a research paper documenting the basis for hydrological impacts is discussed in detail in Section 4.2.1.

- Unsealed access roads are proposed through the site and are to be constructed at-grade to maintain sheet flow characteristics of the existing site. Surface flows over roads discharge directly to adjacent pervious catchments, avoiding concentration of flows throughout. Therefore, the effective impervious area is not increased.
- The development area of the of the site laydown area is approximately 5.9 Ha, as referenced to the layout in Appendix B, and equates to an approximate 1.2% increase in imperviousness relative to the proposed maximum development footprint area of 466 Ha. In context of the relative scale of the laydown area, this is not considered significant to the development of a hydraulic model relative to the existing scenario.

3.2.3.2 Water depth and flood extents results

Water depth and flood extents results for the 1% AEP storm event are provided in Appendix D. The results show that water depths throughout are typically highest within the identified drainage/ waterway corridors and buffer zones. There are areas of breakouts from the main channels throughout the site, however, these are predominantly less than 0.5 m in depth as a maximum, with flows occurring as sheet flows.

The mapping shows areas of breakout flows concentrate against the contour banks throughout, where localised depths increase in locations. The contour banks are a distinct feature of the existing terrain and generally assist with flow management on the site and should be retained in coordination with the solar arrays layouts as part of future project development and design. In consideration of the solar array's layouts, the location and extent of the existing contour banks shall be retained or amended as required to allow for suitable foundation and freeboard above the existing levels. Lower height contour banks are also recommended, and additional or interim contour banks throughout to reduce localised flows depths as required, and to replicate the existing flow regime

The water level at the southern extent of the site is associated with localised increased depths due to boundary conditions associated with the hydraulic model. This area is associated with a proposed bushfire and access buffer width of 10 m.

From the review of the hydraulic modelling results, and in context of the lack of sufficient terrain data in the northern section of the site, the watercourse buffer areas are considered adequate for containment of higher channelised water depths, with shallow sheet flows perceived to typically occur in the northern areas, consistent with modelling observations throughout. Additional LiDAR data or similar survey is required in the northern section of the site to validate this assessment during future project stages, as discussed in Section 2.1.

3.2.3.3 Flood hazard results

Flood hazard results for the 1% AEP storm event are provided in Appendix D.

To understand the impacts of flood hazard, reference is made to Section 7.2.7 of ARR 2019. Figure 6.7.9 guidance on indexing of flood hazard, relative to the depth and velocity of flooding and stormwater flows. An extract of Section 7.2.7 of this guideline has been provided in Appendix E for reference. Figure 6.7.9 show various hazard classifications from H1 to H6 and limiting classification combinations required for depth and velocity.

H1 and H2 are the lowest hazard categories and are considered the benchmark for the development footprint for assessment relative to a 1% AEP event. The range of characteristics are defined as below:

- H1 – generally safe for vehicles, people and buildings
- H2 – unsafe for small vehicles. In a 1% AEP event, access to site by any means typically would not occur
- Classification limit d*v <= 0.3 m^2/s to <= 0.6 m^2/s
- Limiting still water depth = 0.3 m – 0.5m
- Limiting velocity = 2.0 m/s

The flood hazard mapping results within drainage lines and watercourses are typically contained within the nominated watercourse buffer zones throughout with associated hazards higher than H3. There are isolated occurrences where the flood hazard mapping exceeds the H2 hazard category, however, this is largely associated with existing contour banks where concentrated sheet flows increase in depth locally. Localised modifications to landforms during future design stages are likely to resolve these localised hazards, as discussed in Section 3.2.3.2. Also as discussed in Section 3.2.3.2, additional LiDAR data is required in the northern section of the site to validate this assessment during future project stages.

4 Surface water assessment

The impacts of flooding for the 1% AEP have been documented in Section 3 of the report. The surface water assessment is associated with development of the site in context of the following three elements:

- Stormwater quantity – management of stormwater flows from the proposed site to minimise all impacts to the receiving environment
- Stormwater quality – management of increased pollutant loads primarily associated with mobilisation of nutrients and suspended solids within the development footprint from increased impervious surfaces
- Erosion and sediment control – management of soil and water throughout the construction phase of the project in coordination with construction staging, removal of vegetation and exposure of underlying soils, and coordination with proposed stormwater quantity and quality infrastructure measures

4.1 Stormwater management approach

The approach for the management of surface water is associated with:

- The solar facility development – consisting predominantly of solar panels arrays and unsealed access roads for the majority of the site in reference to the proposed layout shown in Appendix B
- The laydown areas and associated infrastructure is located within a total area of approximately 5.9 Ha, as shown by the footprint within the yellow border in Figure 4.

The surface water assessment for these areas is fundamentally different, therefore, are addressed and discussed separately within this Section.

Figure 4: laydown development footprint area of ~5.9 Ha

4.2 Solar arrays development

4.2.1 Stormwater quantity

The intent for the stormwater quantity assessment for the developed solar facility site is to maintain existing surface sheet flows over predominantly existing vegetated surfaces. The solar panel arrays and unsealed access roads insignificantly impact the effective impervious areas of surfaces for the project.

There is an overall negligible hydrologic impact associated with the solar arrays in the 1% AEP event upon discharge to the downstream receiving environment, in reference to the research paper entitled Hydrologic Response of Solar Farms (Cook et al, 2013) provided in Appendix C. This paper states that the solar arrays do not have a significant impact on run off volumes, peaks, or times to peak, when associated with the provision of good vegetative ground cover throughout to replicate the existing scenario. Additionally, the kinetic energy of flow from the solar panels has increased potential to cause erosion at the base of the panels, reiterating the importance for effective grassing/ vegetation to be adopted at this interface, for the life of the project.

The development footprint of the solar panel arrays is associated with a low hazard category below H2 throughout, and d*V ratio <0.6, during the 1% AEP event. The site layout in Appendix B shows that concentrated flow is associated with Two Mile Creek and adjacent localised gullies, the development footprint contains only low hazard sheet flow.

At-grade unsealed access roads, and causeway crossings of creeks, will require ongoing maintenance over the life of the project, and grassing/ vegetation is be maintained for the footprint of the solar arrays to replicate the existing scenario as close as possible over the operational phase of the project.

4.2.2 Stormwater quality

No stormwater quality treatment measures are associated with the solar farm development area. The effective impervious area of the solar panel arrays and the at-grade access roads is insignificant and does not impact surface flows and infiltration compared to the existing scenario.

4.2.3 Erosion and sediment control

Erosion and Sediment Control measures are to be developed during the project development in accordance with the requirements of the Best Practice Erosion and Sediment Control Guidelines, IECA (2008) to meet the following objectives:

- Management of overland flow through the site
- Implementation of vegetation regrowth
- Implementation of soil erosion protection measures
- Minimising disturbance

Erosion and sediment control measures for the solar farm development area are associated with the following infrastructure and construction works:

- At-grade unsealed access tracks and causeways at watercourse/ gully crossings
- Piled foundations for solar panel arrays
- Installation of underground medium voltage network
- Installation of security fencing
- Integration of the works with existing contour bank landforms

Development of erosion control measures for the project are to be developed in consideration of the following:

- Early planning, design and implementation of an ESCP that identifies soil and water management issues and objectives, and is maintained, supervised and monitored for effectiveness.
- Site construction works can be staged, and construction works developed to minimise areas of exposure to achieve no greater than 2500 m^2 of exposed soils or achieve a soil loss rate less than 75 t/ Ha/ year. This will require implementation of Type 3 controls, typically consisting of sediment fence treatment
- Allowance for sedimentation basins within the exposure footprint of the unsealed access roads for localised containment of sediment laden water
- Placement of rock check dams or coir fibre logs along access roads to slow velocities and dissipate energy for potential long run lengths dependent on staging
- Construction of diversion drains on the upstream side of unsealed access roads, within the proposed access road footprint, to capture clean water flows and discharge downstream to stabilised landforms
- Stockpiles, where required for access road construction predominantly, are to have sediment fences to control any potential sediment laden flows
- Undertake construction activities during the dry season, where possible, to reduce likelihood of rainfall mobilising sediments during stormwater runoff

- If construction during the wet season must occur, isolation of workspace to reduce inflow and runoff to be completed prior to works commencing.

A detailed ESCP will be required prior to the commencement of construction once detailed design for the site has been undertaken.

4.2.4 Earthworks

Earthworks for the solar panel array development area are anticipated to be minor with no significant filling proposed. Civil works for the development area include:

- Construction of 200 mm depth at-grade gravel access tracks and causeways, requiring excavation to required depth
- Installation of solar array panels on pier footings, requiring localised excavation of piers to required foundation level
- Excavation for electrical cabling, requiring trenching to required depths
- Replication and/ or reconfiguration of contour banks and existing landform features
- Revegetation/ grassing with natural specifies to replicate the existing scenario

4.3 Laydown areas and associated infrastructure

4.3.1 Stormwater quantity

The stormwater quantity management measures for the laydown area footprint are provided in Figure 5, and is impacted by the following catchments:

- External upstream catchment north of laydown areas – 60 Ha
- Laydown area footprint combined catchment area – 5.9 Ha

Figure 5: Proposed diversion bunds/ channels for the laydown area and associated infrastructure

For the northern portion of the laydown area, the existing watercourse intercepts the external catchment flows. There are break out flows for the 1% AEP storm event from the watercourse however, as shown in the flood depth and extents figures in Appendix D. A diversion bund or channel will need to be designed to intercept and divert these flows back to the watercourse and downstream.

For the southern portion of the laydown area, two diversion channels and bunds will be required to capture and convey the external 1% AEP flows away from the area and discharge to the downstream watercourse, in coordination

with other works in the vicinity including solar arrays, fencing and the transmission line easement (shown as the green corridor in Figure 5).

A minimum freeboard of 300 mm for the peak 1% AEP storm event level is to be determined for habitable buildings and critical infrastructure during future project development.

As discussed in Section 3.2.3.1, the laydown area footprint is associated with an increase of approximately 1.2% of the proposed maximum footprint of 466 Ha, resulting in an insignificant increase in 1% AEP flows relative to the proposed project extents and the greater regional Lake Callide catchment as shown in Figure 6. In context of this relative comparison of local and regional catchment areas, the increase in imperviousness associated with the laydown area is insignificant. Further development of the layout of the laydown area, and actual impervious extents within the 5.9 Ha footprint needs to be developed during future project development and stages.

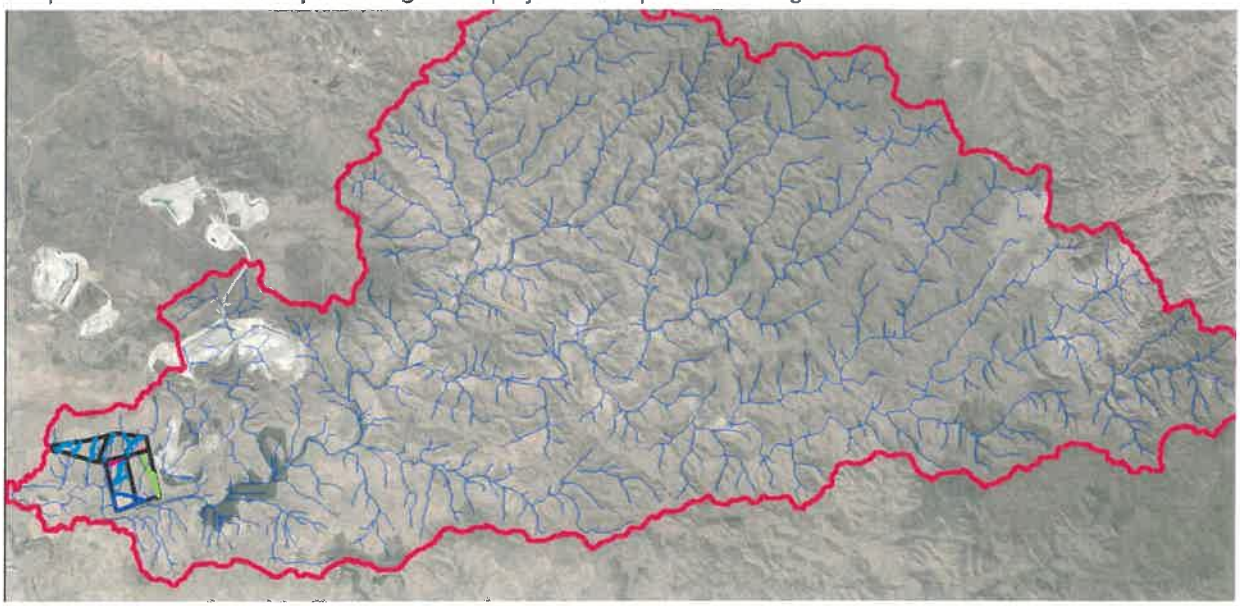


Figure 6: Relative comparison of proposed total development site footprint and Lake Callide catchment

4.3.2 Stormwater quality

The overarching design objectives for water sensitive urban design (WSUD) is referenced to the State Planning Policy (SPP), 2017, Appendix B, Table B, for Western Queensland, for mean annual pollutant load reductions for the laydown development footprint, are:

- >= 85% reduction in total suspended solids load
- >= 60% reduction in total phosphorus load
- >= 45% reduction on total nitrogen load
- >= 90% reduction in gross pollutant load

To meet the required pollution reduction targets, a treatment train approach is required to be adopted, with tertiary implementation measures, such as bioretention basins or wetlands, proposed within the laydown area development footprint to treat to the required levels prior to discharge to the receiving environment. An end of the line sizing for tertiary treatment measures has been conservatively sized based on a 5.9 Ha of development area. Preliminary sizing requirements for two tertiary treatment options are:

- Bioretention basin – 1.5% of effective impervious area (EIA)
- Constructed wetland – 4% of EIA

A summary of the concept sizing of each is summarised in Table 5.

Table 5: Tertiary treatment measures concept sizing

Laydown development area (m^2)	Bioretention Basin Sizing (m^2)	Constructed Wetland Sizing (m^2)
59,000	885	2,360

Incorporation of a bioretention basin provides the most efficient use of area, however, it has potential outletting issues on flat sites, and ongoing operation and maintenance requirements. A wetland requires significantly more

surface area to achieve required tertiary treatment levels. A treatment train approach incorporating the whole laydown development area is required to be developed at future design stages. The use of leading practice proprietary products may also be an option, including tertiary level treatment of WSUD pollutants.

4.3.3 Erosion and sediment control

Erosion and Sediment Control objectives are as previously stated in Section 4.2.3 associated with the development of the approximate 5.9 Ha laydown footprint area. Although construction works and staging will be associated with this area, Type 1 erosion and sediment control implementation measures will be a requirement, associated with an annual soil loss rate greater than 150 t/Ha/yr. Type 1 treatment measures are essentially appropriately sized sediment basins to the requirements of IECA (2008). The design and implementation of erosion control measures for the site is to be undertaken in consideration of the following:

- Early construction and stabilisation of the external catchment diversion channel/ bunds to divert external flows away from the laydown development footprint construction works
- Early construction and stabilisation of the proposed sedimentation basin and associated dirty water connectivity through swales, channels, pipes etc
- Construct the sediment basin to sizing determined from a detailed ESCP to be developed during future design stages. The sedimentation basin should be coordinated with WSUD requirements and location of the treatment train and tertiary treatment measures.
- Stockpiles, where required, for management of imported and excavated topsoil and material
- Implementation of stabilised site access at entry and exit locations to manage sediment removal from construction plant before leaving the construction site
- Undertake construction activities during the dry season, where possible, to reduce likelihood of rainfall mobilising sediments in runoff
- If construction works occur during the wet season, isolation of workspaces is required to be completed to reduce inflow and runoff prior to works commencing.

A detailed ESCP will be required prior to the commencement of construction once detailed design for the site has been undertaken.

4.3.4 Earthworks

Potential earthworks and civil works for the laydown development area include:

- Cut and fill of earthworks to form building pad levels for buildings and as required for associated infrastructure, to be developed during future project development
- Construction of 1% AEP diversion channels/ bunds, and excavation for drainage works requiring excavation to formation and invert levels
- Construction of water quality treatment infrastructure with excavation to required design depths
- Construction of car parking, pavements, road crossing accesses etc, with excavation to foundation and sub-grade levels throughout
- Installation of underground services with excavation to designed depths and inverts

4.4 Lawful Point of Discharge

Section 3.9 of the Queensland Urban Drainage Manual (QUDM), Fourth Edition (IPWEA Queensland, 2017) describes that the lawful point of discharge must comply with all laws (Federal, State, Local and common law), and is the developer's responsibility that it is compliant to all laws in carrying out stormwater and development works including not causing a nuisance. However, Section 3.9.1 of QUDM also states that the term 'lawful point of discharge' has no prescribed legal meaning, but states a process for determination of the lawful point of discharge as:

- Whether the proposed development will alter the site's stormwater discharge characteristics in a manner that may substantially damage a third-party property:
  - If not, then no further steps are required to obtain tenure for a lawful point of discharge

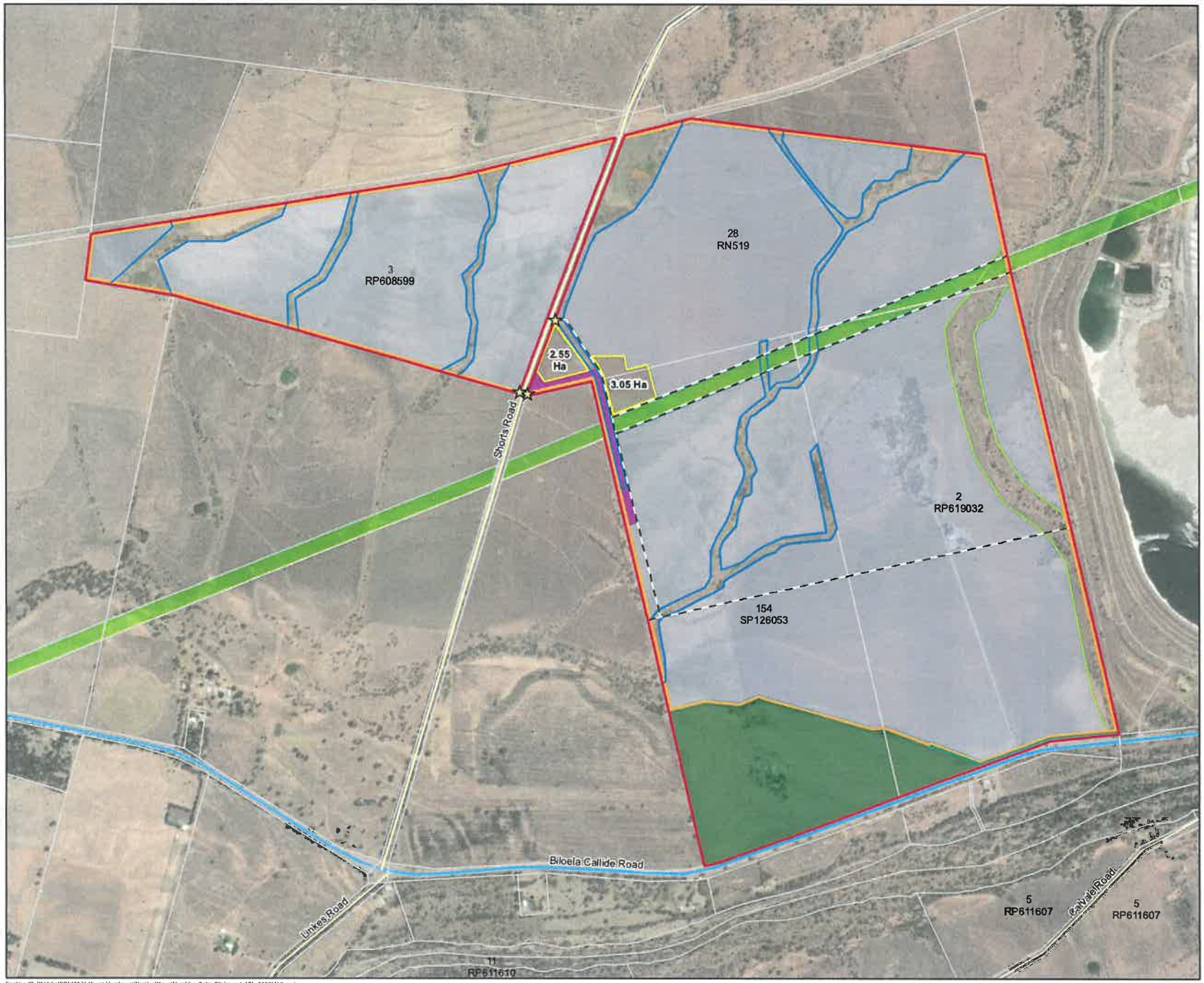
Therefore, from the outcomes of the flood modelling and assessment undertaken for the 1% AEP, the proposed development discharges to existing watercourses and drainage features on the site and has insignificant impacts on

the flow regime of the site, and regional and local catchments. Therefore, the proposed site discharges are lawful point of discharge.

Appendix A Assessment of Performance Outcomes

Performance outcomes	Acceptable outcomes	Response
Division 7 - Development Standards Code - Table 6.7.1 - October 2006		
Stormwater Drainage		
PO12 Stormwater drainage is designed and constructed to provide adequate capacity for existing and anticipated development at predicted design flows and velocity.	**AO12.1** Stormwater drainage, including inter-allotment drainage is in accordance with the CMDG.	Stormwater drainage associated with the solar arrays has adequate capacity for existing and anticipated development as documented in Section 3.2.3.2 and 3.2.3.3 of the report for flood modelling results, and Section 4.2.1, in accordance with the intent of the CDMG. Stormwater drainage associated with the laydowns and associated infrastructure is documented in Section 4.3.1 of the report, in accordance with the intent of the CDMG.
PO17 On-site and off-site erosion and sedimentation is minimised, whether drainage is via formed drainage systems or runoff from the site.	**AO13.1(RAD)** Erosion and Sediment control measures are designed and constructed in accordance with the CMDG.	Management of on-site and off-site erosion potential is documented in Section 4.2.3 in reference to the solar arrays, and Section 4.3.3 in reference to the laydown areas and associated infrastructure. The requirements are in accordance with IECA (2008) and to meet the intent of the CDMG.
Division 9 - Development Design Code - Table 6.8.1		
Stormwater Drainage		
PO16 Stormwater drainage (including interallotment drainage) is designed and constructed to: (1) provide a design capacity able to accommodate existing and anticipated development and flows; (2) ensure that inundation of private and public buildings located in flood prone areas occurs only on rare occasions and that, in such events, surface flow routes convey floodwaters below the prescribed velocity/depth limits; . (3) provide convenience and safety for pedestrians and traffic in frequent stormwater flows by controlling those flows within prescribed limits; (4) maximise the retention of incident rainfall and runoff within each catchment consistent with the planned use and the characteristics of that catchment.	**AO16.1** The design of stormwater drainage shall be in accordance with the CMDG. **AO16.2** Development applications are to comply with the "Flood Level Plan for Determination of Floor Levels" for the Town of Banana in particular, Note: The floor height of buildings will be governed by this plan. **AO16.3** Water Sensitive Urban Design Principles will be based on demonstrated best practice. **AO16.4** Stormwater quality is to meet the requirements of Environmental Protection (Water) Policy 2009. Note: Water quality objectives and environmental values for Queensland waters are contained within Schedule 1 of the Environmental Protection (Water) Policy 2009. Water quality objectives are locally specific and vary between and within river catchments	**AO16.1** The design capacity of the solar arrays is documented in Section 4.2.1 of the report, with the laydown areas and associated infrastructure documented in Section 4.3.1 of the report, to meet the intent of the CDMG **AO16.2** A minimum freeboard for buidlings and critical infrastructure has been specified as a minimum of 300 mm above the peak 1% AEP water level **AO16.3** WSUD principles have been demonstrated in Section 4.2.2 for the solar arrays and Section 4.3.2 for the laydown areas and associated infrastructure. The approach is based on leading practice WSUD guidance **AO16.4** The requirements for development reference the State Planning Policy 2017 requirements for pollution reduction targets, which in turn references the requirements of the Environmental Protection (Water) Policy 2009, and more recent revisions, as documented in Section 4.2.2 and Section 4.3.2 of the report.
PO17 Sub-surface drainage is provided to drain water from structures to ensure their satisfactory performance.	**AO17.1(RAD)** The design of sub-surface drainage is to be in accordance with the CMDG.	Sub-surface drainage is to be associated with future structures development footprint works at the detailed design stage upon development of detailed layouts.
PO18 On-site and off-site erosion and sedimentation is minimised, whether drainage is via formed drainage systems or runoff from the site. Where development involves works, sediment fences, earth berms and temporary drainage are provided and located to prevent sediment being transported to adjoining properties, roads and/or drainage systems.	**AO18.1(RAD)** Erosion and sediment control measures are employed during works to prevent run-off in accordance with the Soil Erosion and Sediment Control Guidelines for Queensland, the Queensland Urban Drainage Manual (QUDM) and the CMDG.	Management of on-site and off-site erosion potential is documented in Section 4.2.3 in reference to the solar arrays, and Section 4.3.3 in reference to the laydown areas and associated infrastructure. The requirements are in accordance with IECA (2008) and to meet the intent of the CDMG, and the Queensland Urban Drainage Manual

Appendix B Proposed Development Layout



LEGEND

- Proposed / Existing Access
- New Farm Fences
- State Controlled Road
- Local Road
- Cadastre
- Site Boundary
- Solar Array Disturbance Footprint
- Vegetation Buffer (7m wide)
- Bushfire/Access Buffer (10m wide) and Security Fence
- Livestock Laneway / Yard
- Optional BESS Location / Site Office / Laydown Area / Substation
- Drainage Line
- Watercourse Buffer (approx. 100m wide)
- Transmission Line Easement
- Flood Area

N

0 250 500 750 1,000

Metres

APPROX SCALE 14,000 @ A3

GDA 2020 MGA Zone 56

Disclaimer: While all reasonable care has been taken to ensure the information contained on this map is up to date and accurate, no guarantee is given that the information portrayed is free from error or omission. Please verify the accuracy of all information prior to use.

DATA SOURCES

Imagery from Esri basemap layer.

Cadastral data © Department of Resources 2022

PR151484-1

Preliminary Site Layout

Callide Solar Power Station

Tracking ID: W:\Jobs\PR151976 Mount Murchison\Working\Mount Murchison\Solar-Site-Layout_A3L_20221117.mxd

Compiled: 17 November 2022

Appendix C Hydrologic Response of Solar Farms Paper

Hydrologic Response of Solar Farms

Lauren M. Cook, S.M.ASCE[1]; and Richard H. McCuen, M.ASCE[2]

Abstract: Because of the benefits of solar energy, the number of solar farms is increasing; however, their hydrologic impacts have not been studied. The goal of this study was to determine the hydrologic effects of solar farms and examine whether or not storm-water management is needed to control runoff volumes and rates. A model of a solar farm was used to simulate runoff for two conditions: the pre- and postpaneled conditions. Using sensitivity analyses, modeling showed that the solar panels themselves did not have a significant effect on the runoff volumes, peaks, or times to peak. However, if the ground cover under the panels is gravel or bare ground, owing to design decisions or lack of maintenance, the peak discharge may increase significantly with storm-water management needed. In addition, the kinetic energy of the flow that drains from the panels was found to be greater than that of the rainfall, which could cause erosion at the base of the panels. Thus, it is recommended that the grass beneath the panels be well maintained or that a buffer strip be placed after the most downgradient row of panels. This study, along with design recommendations, can be used as a guide for the future design of solar farms. **DOI: 10.1061/(ASCE)HE.1943-5584.0000530.**

CE Database subject headings: Hydrology; Land use; Solar power; Floods; Surface water; Runoff; Stormwater management.

Author keywords: Hydrology; Land use change; Solar energy; Flooding; Surface water runoff; Storm-water management.

Introduction

Storm-water management practices are generally implemented to reverse the effects of land-cover changes that cause increases in volumes and rates of runoff. This is a concern posed for new types of land-cover change such as the solar farm. Solar energy is a renewable energy source that is expected to increase in importance in the near future. Because solar farms require considerable land, it is necessary to understand the design of solar farms and their potential effect on erosion rates and storm runoff, especially the impact on offsite properties and receiving streams. These farms can vary in size from 8 ha (20 acres) in residential areas to 250 ha (600 acres) in areas where land is abundant.

The solar panels are impervious to rain water; however, they are mounted on metal rods and placed over pervious land. In some cases, the area below the panel is paved or covered with gravel. Service roads are generally located between rows of panels. Although some panels are stationary, others are designed to move so that the angle of the panel varies with the angle of the sun. The angle can range, depending on the latitude, from 22° during the summer months to 74° during the winter months. In addition, the angle and direction can also change throughout the day. The issue posed is whether or not these rows of impervious panels will change the runoff characteristics of the site, specifically increase runoff volumes or peak discharge rates. If the increases are hydrologically significant, storm-water management facilities may be needed. Additionally, it is possible that the velocity of water draining from the edge of the panels is sufficient to cause erosion of the soil below the panels, especially where the maintenance roadways are bare ground.

The outcome of this study provides guidance for assessing the hydrologic effects of solar farms, which is important to those who plan, design, and install arrays of solar panels. Those who design solar farms may need to provide for storm-water management. This study investigated the hydrologic effects of solar farms, assessed whether or not storm-water management might be needed, and if the velocity of the runoff from the panels could be sufficient to cause erosion of the soil below the panels.

Model Development

Solar farms are generally designed to maximize the amount of energy produced per unit of land area, while still allowing space for maintenance. The hydrologic response of solar farms is not usually considered in design. Typically, the panels will be arrayed in long rows with separations between the rows to allow for maintenance vehicles. To model a typical layout, a unit width of one panel was assumed, with the length of the downgradient strip depending on the size of the farm. For example, a solar farm with 30 rows of 200 panels each could be modeled as a strip of 30 panels with space between the panels for maintenance vehicles. Rainwater that drains from the upper panel onto the ground will flow over the land under the 29 panels on the downgradient strip. Depending on the land cover, infiltration losses would be expected as the runoff flows to the bottom of the slope.

To determine the effects that the solar panels have on runoff characteristics, a model of a solar farm was developed. Runoff in the form of sheet flow without the addition of the solar panels served as the prepaneled condition. The paneled condition assumed a downgradient series of cells with one solar panel per ground cell. Each cell was separated into three sections: wet, dry, and spacer.

The dry section is that portion directly underneath the solar panel, unexposed directly to the rainfall. As the angle of the panel from the horizontal increases, more of the rain will fall directly onto

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the ground; this section of the cell is referred to as the wet section. The spacer section is the area between the rows of panels used by maintenance vehicles. Fig. 1 is an image of two solar panels and the spacer section allotted for maintenance vehicles. Fig. 2 is a schematic of the wet, dry, and spacer sections with their respective dimensions. In Fig. 1, tracks from the vehicles are visible on what is modeled within as the spacer section. When the solar panel is horizontal, then the length longitudinal to the direction that runoff will occur is the length of the dry and wet sections combined. Runoff from a dry section drains onto the downgradient spacer section. Runoff from the spacer section flows to the wet section of the next downgradient cell. Water that drains from a solar panel falls directly onto the spacer section of that cell.

The length of the spacer section is constant. During a storm event, the loss rate was assumed constant for the 24-h storm because a wet antecedent condition was assumed. The lengths of the wet and dry sections changed depending on the angle of the solar panel. The total length of the wet and dry sections was set equal to the length of one horizontal solar panel, which was assumed to be 3.5 m. When a solar panel is horizontal, the dry section length would equal 3.5 m and the wet section length would be zero. In the paneled condition, the dry section does not receive direct rainfall because the rain first falls onto the solar panel then drains onto the spacer section. However, the dry section does infiltrate some of the runoff that comes from the upgradient wet section. The wet section was modeled similar to the spacer section with rain falling directly onto the section and assuming a constant loss rate.

For the presolar panel condition, the spacer and wet sections are modeled the same as in the paneled condition; however, the cell does not include a dry section. In the prepaneled condition, rain falls directly onto the entire cell. When modeling the prepaneled condition, all cells receive rainfall at the same rate and are subject to losses. All other conditions were assumed to remain the same such that the prepaneled and paneled conditions can be compared.

Rainfall was modeled after an natural resources conservation service (NRCS) Type II Storm (McCuen 2005) because it is an accurate representation of actual storms of varying characteristics that are imbedded in intensity-duration-frequency (IDF) curves. For each duration of interest, a dimensionless hyetograph was developed using a time increment of 12 s over the duration of the storm (see Fig. 3). The depth of rainfall that corresponds to each storm magnitude was then multiplied by the dimensionless hyetograph. For a 2-h storm duration, depths of 40.6, 76.2, and 101.6 mm were used for the 2-, 25-, and 100-year events. The 2- and 6-h duration hyetographs were developed using the center portion of the 24-h storm, with the rainfall depths established with the Baltimore IDF curve. The corresponding depths for a 6-h duration were 53.3, 106.7, and 132.1 mm, respectively. These magnitudes were chosen to give a range of storm conditions.

During each time increment, the depth of rain is multiplied by the cell area to determine the volume of rain added to each section of each cell. This volume becomes the storage in each cell. Depending on the soil group, a constant volume of losses was subtracted from the storage. The runoff velocity from a solar panel was calculated using Manning's equation, with the hydraulic radius for sheet flow assumed to equal the depth of the storage on the panel (Bedient and Huber 2002). Similar assumptions were made to compute the velocities in each section of the surface sections.

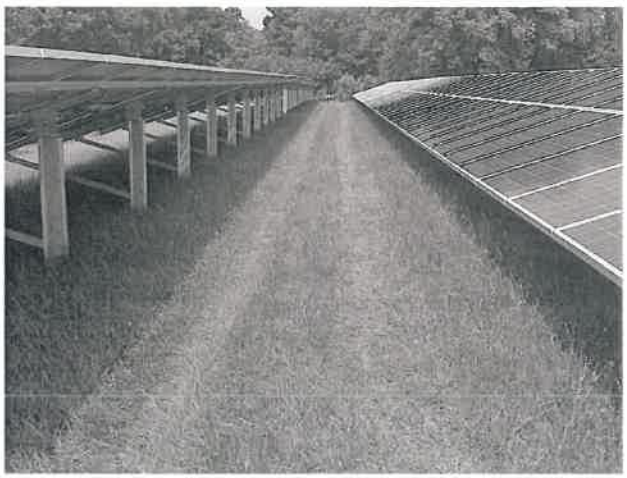

Fig. 1. Maintenance or "spacer" section between two rows of solar panels (photo by John E. Showler, reprinted with permission)

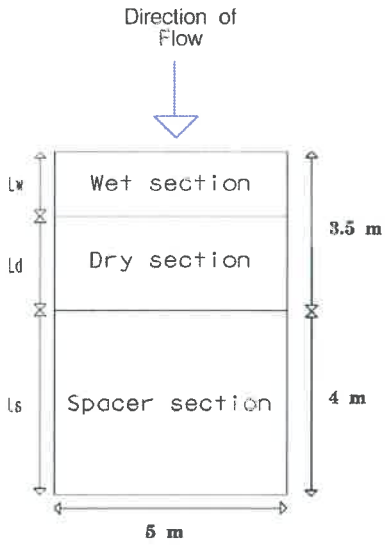

Fig. 2. Wet, dry, and spacer sections of a single cell with lengths *Lw*, *Ls*, and *Ld* with the solar panel covering the dry section

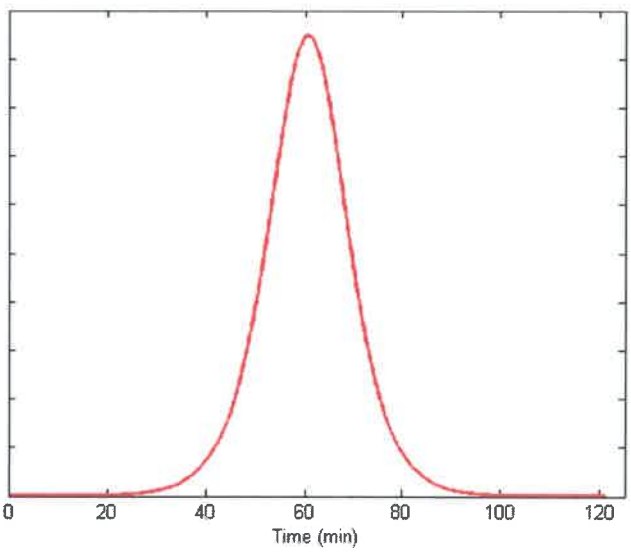

Fig. 3. Dimensionless hyetograph of 2-h Type II storm

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Runoff from one section to the next and then to the next downgradient cell was routed using the continuity of mass. The routing coefficient depended on the depth of flow in storage and the velocity of runoff. Flow was routed from the wet section to the dry section to the spacer section, with flow from the spacer section draining to the wet section of the next cell. Flow from the most downgradient cell was assumed to be the outflow. Discharge rates and volumes from the most downgradient cell were used for comparisons between the prepaneled and paneled conditions.

Alternative Model Scenarios

To assess the effects of the different variables, a section of 30 cells, each with a solar panel, was assumed for the base model. Each cell was separated individually into wet, dry, and spacer sections. The area had a total ground length of 225 m with a ground slope of 1% and width of 5 m, which was the width of an average solar panel. The roughness coefficient (Engman 1986) for the silicon solar panel was assumed to be that of glass, 0.01. Roughness coefficients of 0.15 for grass and 0.02 for bare ground were also assumed. Loss rates of 0.5715 cm/h (0.225 in./h) and 0.254 cm/h (0.1 in./h) for B and C soils, respectively, were assumed.

The prepaneled condition using the 2-h, 25-year rainfall was assumed for the base condition, with each cell assumed to have a good grass cover condition. All other analyses were made assuming a paneled condition. For most scenarios, the runoff volumes and peak discharge rates from the paneled model were not significantly greater than those for the prepaneled condition. Over a total length of 225 m with 30 solar panels, the runoff increased by 0.26 m^3, which was a difference of only 0.35%. The slight increase in runoff volume reflects the slightly higher velocities for the paneled condition. The peak discharge increased by 0.0013 m^3, a change of only 0.31%. The time to peak was delayed by one time increment, i.e., 12 s. Inclusion of the panels did not have a significant hydrologic impact.

Storm Magnitude

The effect of storm magnitude was investigated by changing the magnitude from a 25-year storm to a 2-year storm. For the 2-year storm, the rainfall and runoff volumes decreased by approximately 50%. However, the runoff from the paneled watershed condition increased compared to the prepaneled condition by approximately the same volume as for the 25-year analysis, 0.26 m^3. This increase represents only a 0.78% increase in volume. The peak discharge and the time to peak did not change significantly. These results reflect runoff from a good grass cover condition and indicated that the general conclusion of very minimal impacts was the same for different storm magnitudes.

Ground Slope

The effect of the downgradient ground slope of the solar farm was also examined. The angle of the solar panels would influence the velocity of flows from the panels. As the ground slope was increased, the velocity of flow over the ground surface would be closer to that on the panels. This could cause an overall increase in discharge rates. The ground slope was changed from 1 to 5%, with all other conditions remaining the same as the base conditions.

With the steeper incline, the volume of losses decreased from that for the 1% slope, which is to be expected because the faster velocity of the runoff would provide less opportunity for infiltration. However, between the prepaneled and paneled conditions, the increase in runoff volume was less than 1%. The peak discharge and the time to peak did not change. Therefore, the greater ground slope did not significantly influence the response of the solar farm.

Soil Type

The effect of soil type on the runoff was also examined. The soil group was changed from B soil to C soil by varying the loss rate. As expected, owing to the higher loss rate for the C soil, the depths of runoff increased by approximately 7.5% with the C soil when compared with the volume for B soils. However, the runoff volume for the C soil condition only increased by 0.17% from the prepaneled condition to the paneled condition. In comparison with the B soil, a difference of 0.35% in volume resulted between the two conditions. Therefore, the soil group influenced the actual volumes and rates, but not the relative effect of the paneled condition when compared to the prepaneled condition.

Panel Angle

Because runoff velocities increase with slope, the effect of the angle of the solar panel on the hydrologic response was examined. Analyses were made for angles of 30° and 70° to test an average range from winter to summer. The hydrologic response for these angles was compared to that of the base condition angle of 45°. The other site conditions remained the same. The analyses showed that the angle of the panel had only a slight effect on runoff volumes and discharge rates. The lower angle of 30° was associated with an increased runoff volume, whereas the runoff volume decreased for the steeper angle of 70° when compared with the base condition of 45°. However, the differences (~0.5%) were very slight. Nevertheless, these results indicate that, when the solar panel was closer to horizontal, i.e., at a lower angle, a larger difference in runoff volume occurred between the prepaneled and paneled conditions. These differences in the response result are from differences in loss rates.

The peak discharge was also lower at the lower angle. At an angle of 30°, the peak discharge was slightly lower than at the higher angle of 70°. For the 2-h storm duration, the time to peak of the 30° angle was 2 min delayed from the time to peak of when the panel was positioned at a 70° angle, which reflects the longer travel times across the solar panels.

Storm Duration

To assess the effect of storm duration, analyses were made for 6-h storms, testing magnitudes for 2-, 25-, and 100-year return periods, with the results compared with those for the 2-h rainfall events. The longer storm duration was tested to determine whether a longer duration storm would produce a different ratio of increase in runoff between the prepaneled and paneled conditions. When compared to runoff volumes from the 2-h storm, those for the 6-h storm were 34% greater in both the paneled and prepaneled cases. However, when comparing the prepaneled to the paneled condition, the increase in the runoff volume with the 6-h storm was less than 1% regardless of the return period. The peak discharge and the time-to-peak did not differ significantly between the two conditions. The trends in the hydrologic response of the solar farm did not vary with storm duration.

Ground Cover

The ground cover under the panels was assumed to be a native grass that received little maintenance. For some solar farms, the area beneath the panel is covered in gravel or partially paved because the panels prevent the grass from receiving sunlight. Depending on the

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volume of traffic, the spacer cell could be grass, patches of grass, or bare ground. Thus, it was necessary to determine whether or not these alternative ground-cover conditions would affect the runoff characteristics. This was accomplished by changing the Manning's n for the ground beneath the panels. The value of n under the panels, i.e., the dry section, was set to 0.015 for gravel, with the value for the spacer or maintenance section set to 0.02, i.e., bare ground. These can be compared to the base condition of a native grass ($n = 0.15$). A good cover should promote losses and delay the runoff.

For the smoother surfaces, the velocity of the runoff increased and the losses decreased, which resulted in increasing runoff volumes. This occurred both when the ground cover under the panels was changed to gravel and when the cover in the spacer section was changed to bare ground. Owing to the higher velocities of the flow, runoff rates from the cells increased significantly such that it was necessary to reduce the computational time increment. Fig. 4(a) shows the hydrograph from a 30-panel area with a time increment of 12 s. With a time increment of 12 s, the water in each cell is discharged at the end of every time increment, which results in no attenuation of the flow; thus, the undulations shown in Fig. 4(a) result. The time increment was reduced to 3 s for the 2-h storm, which resulted in watershed smoothing and a rational hydrograph shape [Fig. 4(b)]. The results showed that the storm runoff

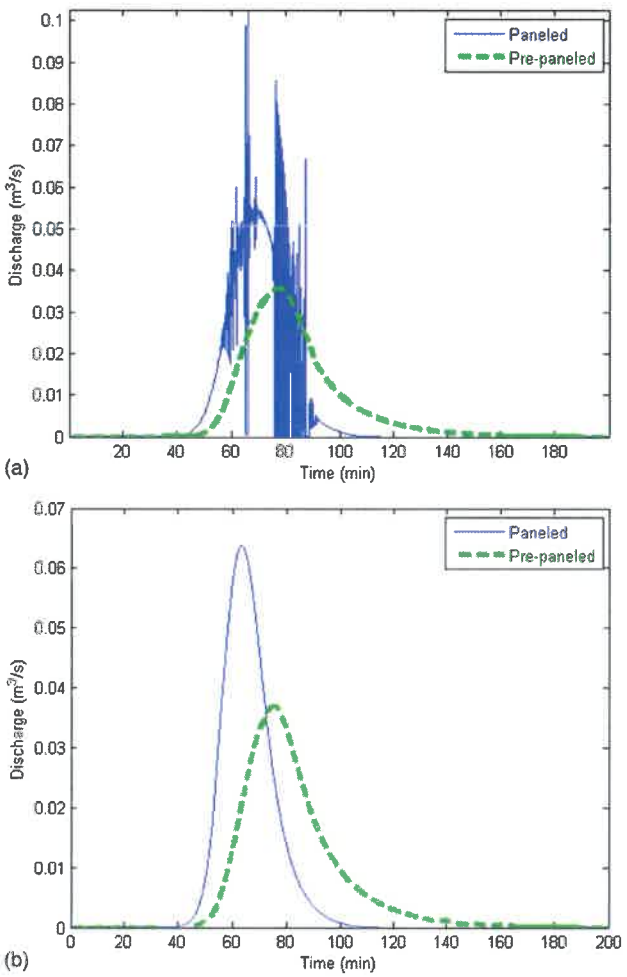

Fig. 4. Hydrograph with time increment of (a) 12 s; (b) 3 s with Manning's n for bare ground

increased by 7% from the grass-covered scenario to the scenario with gravel under the panel. The peak discharge increased by 73% for the gravel ground cover when compared with the grass cover without the panels. The time to peak was 10 min less with the gravel than with the grass, which reflects the effect of differences in surface roughness and the resulting velocities.

If maintenance vehicles used the spacer section regularly and the grass cover was not adequately maintained, the soil in the spacer section would be compacted and potentially the runoff volumes and rates would increase. Grass that is not maintained has the potential to become patchy and turn to bare ground. The grass under the panel may not get enough sunlight and die. Fig. 1 shows the result of the maintenance trucks frequently driving in the spacer section, which diminished the grass cover.

The effect of the lack of solar farm maintenance on runoff characteristics was modeled by changing the Manning's n to a value of 0.02 for bare ground. In this scenario, the roughness coefficient for the ground under the panels, i.e., the dry section, as well as in the spacer cell was changed from grass covered to bare ground ($n = 0.02$).The effects were nearly identical to that of the gravel. The runoff volume increased by 7% from the grass-covered to the bare-ground condition. The peak discharge increased by 72% when compared with the grass-covered condition. The runoff for the bare-ground condition also resulted in an earlier time to peak by approximately 10 min. Two other conditions were also modeled, showing similar results. In the first scenario, gravel was placed directly under the panel, and healthy grass was placed in the spacer section, which mimics a possible design decision. Under these conditions, the peak discharge increased by 42%, and the volume of runoff increased by 4%, which suggests that storm-water management would be necessary if gravel is placed anywhere.

Fig. 5 shows two solar panels from a solar farm in New Jersey. The bare ground between the panels can cause increased runoff rates and reductions in time of concentration, both of which could necessitate storm-water management. The final condition modeled involved the assumption of healthy grass beneath the panels and bare ground in the spacer section, which would simulate the condition of unmaintained grass resulting from vehicles that drive over the spacer section. Because the spacer section is 53% of the cell, the change in land cover to bare ground would reduce losses and decrease runoff travel times, which would cause runoff to amass as it

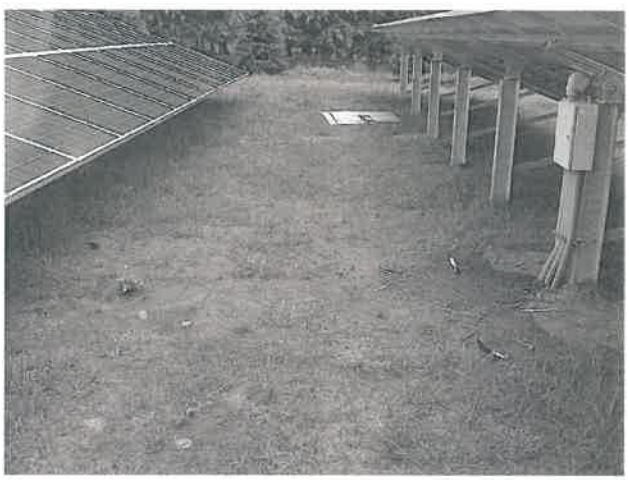

Fig. 5. Site showing the initiation of bare ground below the panels, which increases the potential for erosion (photo by John Showler, reprinted with permission)

moves downgradient. With the spacer section as bare ground, the peak discharge increased by 100%, which reflected the increases in volume and decrease in timing. These results illustrate the need for maintenance of the grass below and between the panels.

Design Suggestions

With well-maintained grass underneath the panels, the solar panels themselves do not have much effect on total volumes of the runoff or peak discharge rates. Although the panels are impervious, the rainwater that drains from the panels appears as runoff over the downgradient cells. Some of the runoff infiltrates. If the grass cover of a solar farm is not maintained, it can deteriorate either because of a lack of sunlight or maintenance vehicle traffic. In this case, the runoff characteristics can change significantly with both runoff rates and volumes increasing by significant amounts. In addition, if gravel or pavement is placed underneath the panels, this can also contribute to a significant increase in the hydrologic response.

If bare ground is foreseen to be a problem or gravel is to be placed under the panels to prevent erosion, it is necessary to counteract the excess runoff using some form of storm-water management. A simple practice that can be implemented is a buffer strip (Dabney et al. 2006) at the downgradient end of the solar farm. The buffer strip length must be sufficient to return the runoff characteristics with the panels to those of runoff experienced before the gravel and panels were installed. Alternatively, a detention basin can be installed.

A buffer strip was modeled along with the panels. For approximately every 200 m of panels, or 29 cells, the buffer must be 5 cells long (or 35 m) to reduce the runoff volume to that which occurred before the panels were added. Even if a gravel base is not placed under the panels, the inclusion of a buffer strip may be a good practice when grass maintenance is not a top funding priority. Fig. 6 shows the peak discharge from the graveled surface versus the length of the buffer needed to keep the discharge to prepaneled peak rate.

Water draining from a solar panel can increase the potential for erosion of the spacer section. If the spacer section is bare ground, the high kinetic energy of water draining from the panel can cause soil detachment and transport (Garde and Raju 1977; Beuselinck et al. 2002). The amount and risk of erosion was modeled using the velocity of water coming off a solar panel compared with the velocity and intensity of the rainwater. The velocity of panel runoff was calculated using Manning's equation, and the velocity of falling rainwater was calculated using the following:

$$V_t = 120\, d_r^{0.35} \tag{1}$$

where d_r = diameter of a raindrop, assumed to be 1 mm. The relationship between kinetic energy and rainfall intensity is

$$K_e = 916 + 330 \log_{10} i \tag{2}$$

where i = rainfall intensity (in./h) and K_e = kinetic energy (ft-tons per ac-in. of rain) of rain falling onto the wet section and the panel, as well as the water flowing off of the end of the panel (Wischmeier and Smith 1978). The kinetic energy (Salles et al. 2002) of the rainfall was greater than that coming off the panel, but the area under the panel (i.e., the product of the length, width, and cosine of the panel angle) is greater than the area under the edge of the panel where the water drains from the panel onto the ground. Thus, dividing the kinetic energy by the respective areas gives a more accurate representation of the kinetic energy experienced by the soil. The energy of the water draining from the panel onto the ground can be nearly 10 times greater than the rain itself falling onto the ground area. If the solar panel runoff falls onto an unsealed soil, considerable detachment can result (Motha et al. 2004). Thus, because of the increased kinetic energy, it is possible that the soil is much more prone to erosion with the panels than without. Where panels are installed, methods of erosion control should be included in the design.

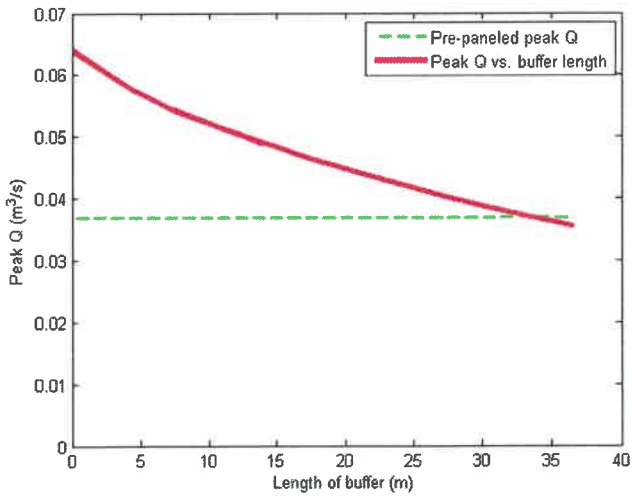

Fig. 6. Peak discharge over gravel compared with buffer length

Conclusions

Solar farms are the energy generators of the future; thus, it is important to determine the environmental and hydrologic effects of these farms, both existing and proposed. A model was created to simulate storm-water runoff over a land surface without panels and then with solar panels added. Various sensitivity analyses were conducted including changing the storm duration and volume, soil type, ground slope, panel angle, and ground cover to determine the effect that each of these factors would have on the volumes and peak discharge rates of the runoff.

The addition of solar panels over a grassy field does not have much of an effect on the volume of runoff, the peak discharge, nor the time to peak. With each analysis, the runoff volume increased slightly but not enough to require storm-water management facilities. However, when the land-cover type was changed under the panels, the hydrologic response changed significantly. When gravel or pavement was placed under the panels, with the spacer section left as patchy grass or bare ground, the volume of the runoff increased significantly and the peak discharge increased by approximately 100%. This was also the result when the entire cell was assumed to be bare ground.

The potential for erosion of the soil at the base of the solar panels was also studied. It was determined that the kinetic energy of the water draining from the solar panel could be as much as 10 times greater than that of rainfall. Thus, because the energy of the water draining from the panels is much higher, it is very possible that soil below the base of the solar panel could erode owing to the concentrated flow of water off the panel, especially if there is bare ground in the spacer section of the cell. If necessary, erosion control methods should be used.

Bare ground beneath the panels and in the spacer section is a realistic possibility (see Figs. 1 and 5). Thus, a good, well-maintained grass cover beneath the panels and in the spacer section is highly recommended. If gravel, pavement, or bare ground is

deemed unavoidable below the panels or in the spacer section, it may necessary to add a buffer section to control the excess runoff volume and ensure adequate losses. If these simple measures are taken, solar farms will not have an adverse hydrologic impact from excess runoff or contribute eroded soil particles to receiving streams and waterways.

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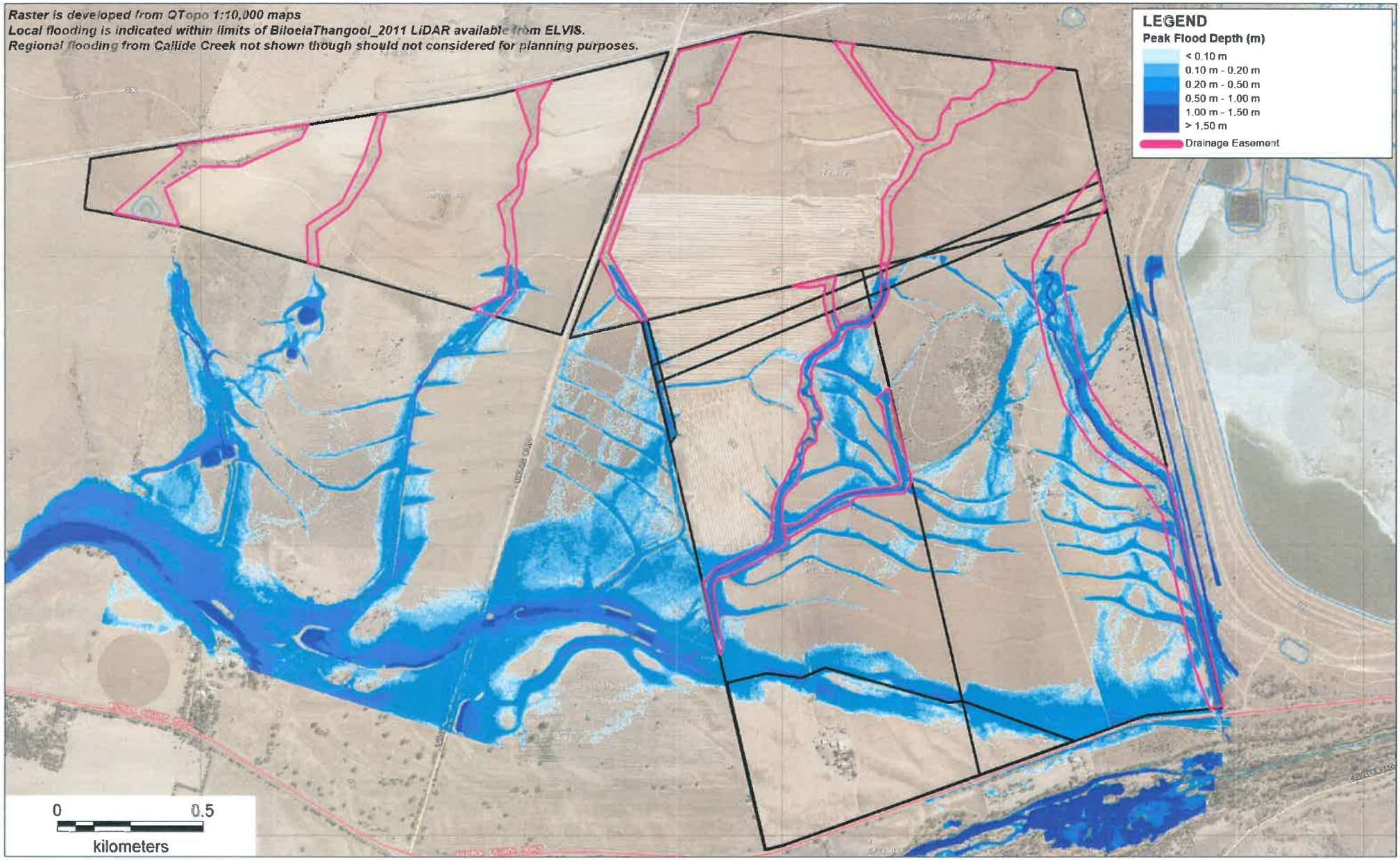
Appendix D Hydraulic modelling



COORDINATE SYSTEM Datum: GDA94 Projection: MGA Zone 56	NORTH	FIGURE NO D-1	civil IQ
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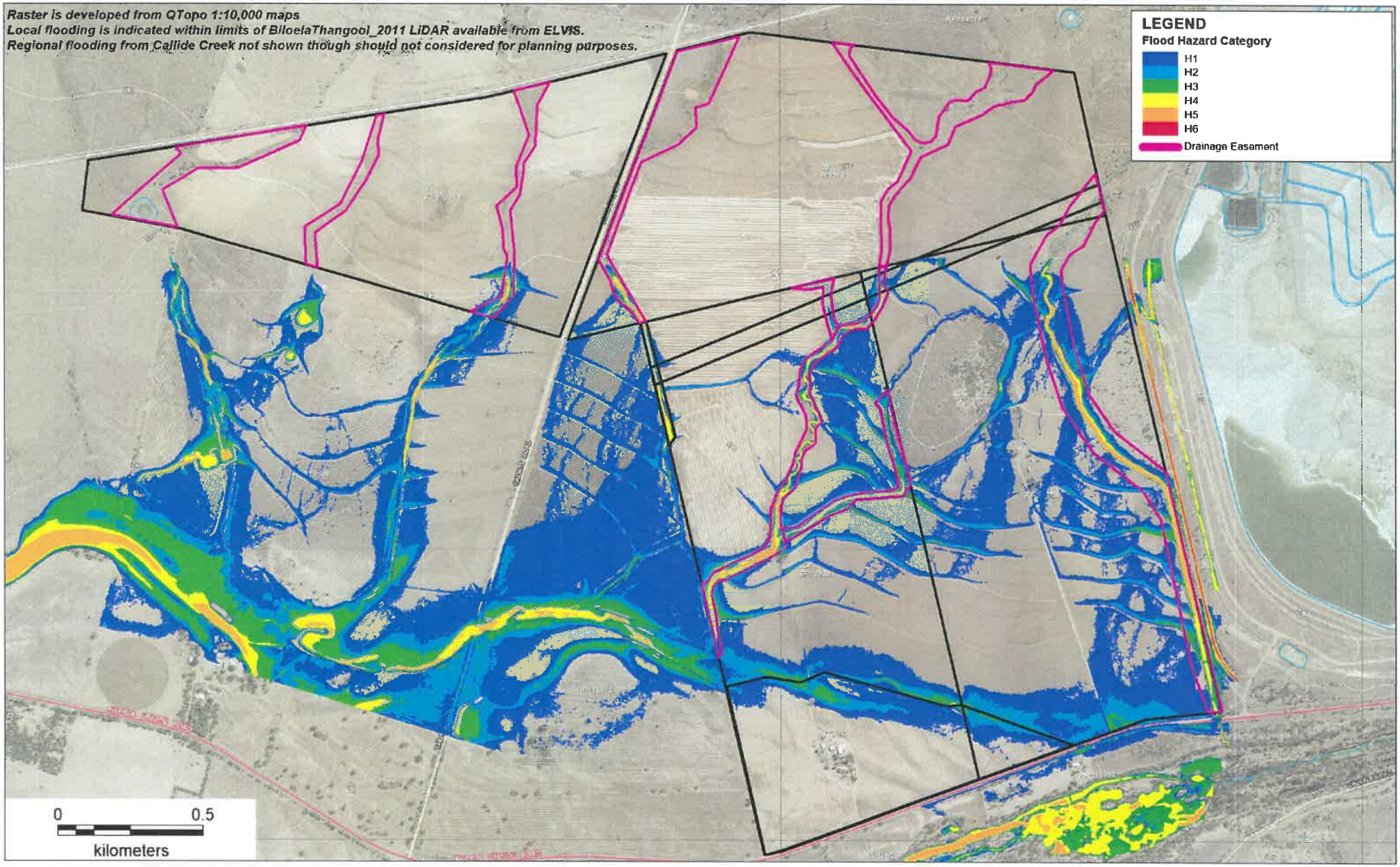

COORDINATE SYSTEM Datum: GDA94 Projection: MGA Zone 56	NORTH	FIGURE NO D-2	civil IQ
PAGE SIZE A3		DATE 6/11/2022	
PROJECT TITLE Callide Solar Power Station		TITLE **Flood Hazard Category ARR2019 100 yr ARI (1% AEP)**	

Appendix E ARR 2019 flood hazard guidance extract

vehicles or building thresholds separately. In many instances, this will suit the requirements of specific analyses. For example, if the required assessment is to determine whether a road evacuation route is trafficable for a given flood event, then the vehicle stability threshold curves should be applied. Likewise, if the assessment is to determine which buildings would be suitable for shelter in place during a PMF event, then the building stability thresholds for flood hazard should be used in the analysis.

7.2.7. General Flood Hazard Curves

When dealing with specific floodplain management or emergency management analysis there may be a clear need to use specific thresholds as described above. However, particularly in a preliminary assessment of risks or as part of a constraints analysis such as might be applied as part of a strategic floodplain management assessment, there is also an acknowledged need for a combined set of hazard vulnerability curves, which can be used as a general classification of flood hazard on a floodplain. A suggested set of curves based on the referenced thresholds presented above is provided in Figure 6.7.9.

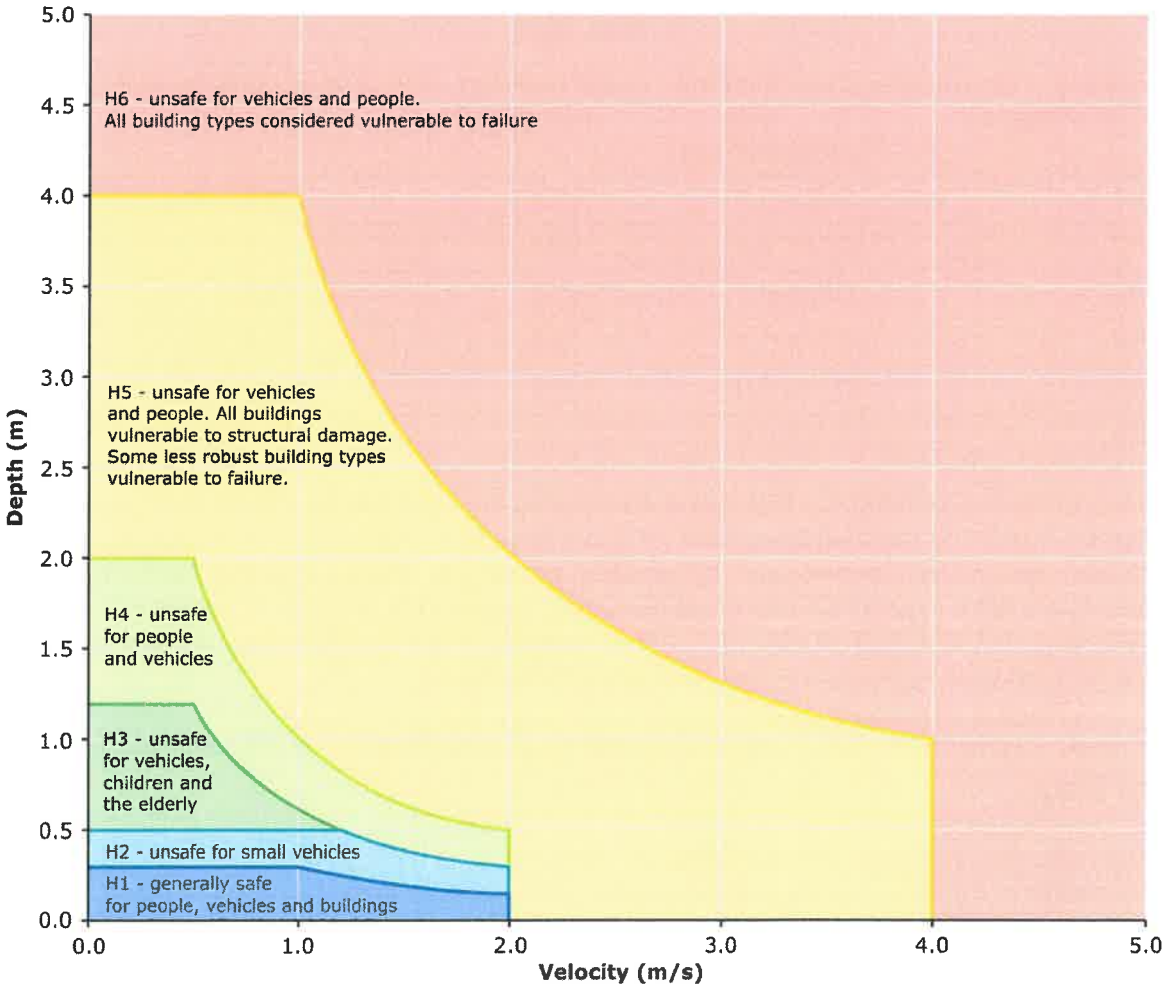

Figure 6.7.9. Combined Flood Hazard Curves (Smith et al., 2014)

The combined flood hazard curves presented in Figure 6.7.9 set hazard thresholds that relate to the vulnerability of the community when interacting with floodwaters. The combined curves are divided into hazard classifications that relate to specific vulnerability thresholds as described in Table 6.7.3. Table 6.7.4 provides the limits for the classifications in Table 6.7.3

Table 6.7.3. Combined Hazard Curves - Vulnerability Thresholds (Smith et al., 2014)

Hazard Vulnerability Classification	Description
H1	Generally safe for vehicles, people and buildings.
H2	Unsafe for small vehicles.
H3	Unsafe for vehicles, children and the elderly.
H4	Unsafe for vehicles and people.
H5	Unsafe for vehicles and people. All buildings vulnerable to structural damage. Some less robust buildings subject to failure.
H6	Unsafe for vehicles and people. All building types considered vulnerable to failure.

Table 6.7.4. Combined Hazard Curves - Vulnerability Thresholds Classification Limits (Smith et al., 2014)

Hazard Vulnerability Classification	Classification Limit (D and V in combination)	Limiting Still Water Depth (D)	Limiting Velocity (V)
H1	D*V ≤ 0.3	0.3	2.0
H2	D*V ≤ 0.6	0.5	2.0
H3	D*V ≤ 0.6	1.2	2.0
H4	D*V ≤ 1.0	2.0	2.0
H5	D*V ≤ 4.0	4.0	4.0
H6	D*V > 4.0	-	-

Importantly, the vulnerability thresholds identified in the flood hazard curves described above can be applied to the best description of flood behaviour available for a subject site. In this regard, the hazard curves can be applied equally to flood behaviour estimates from measured data, simpler 1D numerical modelling approaches, through to complex 2D model estimates with the level of accuracy and uncertainty of the flood hazard estimate linked to the method used to derive the flood behaviour estimate.

7.2.8. Isolation, Effective Warning Time, Rate of Rise and Time of Day

The effective warning time available to respond to a flood event, the rate of rise of floodwaters, the time of day a flood occurs, and isolation from safety by floodwaters and impassable terrain are all factors that may increase the potential for people to be exposed to hazardous flood situations. These factors are important considerations that influence the vulnerability of communities to flooding and are important considerations in managing flood risk.

7.2.8.1. Isolation

As outlined in AEM Handbook 7 (AEMI, 2014), flooding can isolate parts of the landscape and cut-off evacuation routes to flood-free land. This can result in dangerous situations,